

A TREATISE
ON
LAND
AND
LEVELLING,
ILLUSTRATED BY
COPIOUS FIELD NOTES, PLANS, AND DIAGRAMS,
IN FOUR PARTS,
BY THE
CHAIN, THEODOLITE, CIRCUMFERENTOR, AND SPIRIT LEVEL,
With Drawings,
EXPLAINING THEIR USE, AND EXHIBITING THEIR ADJUSTMENTS,
TOGETHER WITH INTRODUCTORY CHAPTERS
ON GEOMETRY, LOGARITHMS, MENSURATION, AND TRIGONOMETRY,
AND AN APPENDIX OF
TABLES OF LOGARITHMS, SINES, COSINES, TANGENTS, &c., TO SIX PLACES,
AND
A TRAVERSE TABLE
TO ANY DISTANCE, AND TO THREE MINUTES OF BEARING.

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TO

LIEUT. COL. SIR R. BONNYCASTLE, R.E.,

COMMANDING OFFICER OF ENGINEERS,
UPPER CANADA,

AS AN HUMBLE TOKEN OF RESPECT FOR HIS VIRTUES,
OF ADMIRATION FOR HIS TALENTS,

AND

AS A SINCERE, THOUGH INADEQUATE, OFFERING OF ACKNOWLEDGMENT

OF THE MANY
PROFESSIONAL SERVICES RECEIVED AT HIS HANDS,
WHILE EMPLOYED UNDER HIM IN THE FIELD,

THIS WORK

IS, WITH EVERY WISH FOR HIS HEALTH AND HAPPINESS,
RESPECTFULLY INSCRIBED BY HIS SINCERE FRIEND,)

AND OBLIGED SERVANT,

THE AUTHOR.

IT was from the want of any sufficient treatise, that I could put into my pupils' hands, on the subject of LAND SURVEYING and LEVELLING, and the inconvenience I experienced in consequence, while engaged in my professional duties, as Lecturer at King's College, London, that I was induced to compile the following pages.

I found many parts of the subject ably discussed, scattered indifferently among several authors, but none sufficiently consecutive, or in detail, to suit my purpose.

Most of them were too elementary—confined to Chain Surveying—and sometimes not referring, even in that, to the modern system of “tyeing in,” as it is called, or triangulating.

Others, on the contrary, were either purely military, or soared so highly into the depths of analytical calculations, that, though of invaluable assistance to the professional man of science, were, from the omission altogether of the more humble details of operation, both of the Chain and Theodolite, unfitted for the use of the civil surveyor;

and not one of them contained any information on the subject of the Circumferentor, or seemed sufficiently illustrated with plans and field notes.

From the direction also, that education had now taken towards the arts and sciences of life, *in most public and private schools*, I was induced to think that the present would be found a desirable *school book*, among the higher forms, as a full and corrected treatise of the theory and practice of Surveying. At the same time I trusted, that, by supplying the previous deficiencies, I should be furnishing the Surveyor with a complete *vade mecum* of reference.

To fulfill both these objects, I have introduced preliminary chapters on the most useful Problems and Theorems—on the Nature and Use of Logarithms—on Mensuration of Planes—and sufficient of Trigonometry as to enable the reader to understand fully any of the subsequent Trigonometrical Problems.

That portion, which refers to Surveying, has been, for facility of reference, divided into three Parts—by the Chain, Theodolite, and Circumferentor—containing copious field notes, plans, and diagrams, of each kind, together with drawings of the Theodolite and Circumferentor, and ample descriptions of their uses and adjustments.

In the Third Part, on Surveying, a special treatise has been introduced on the uses of the Circumferentor, in its application to the surveying of new countries. And the various methods, in actual operation in America—in the survey, by the needle, of extensive tracts of country—with the nature and application of the Traverse Table, to that purpose, are fully entered into and explained. The method of using the needle in this country, holding, as it does, but a subordinate part in surveying here, bears about an equal proportion to the same instrument in its perfection and accuracy in

new Countries, as the bubble of the Theodolite does to the same principle, when it is perfected in the Spirit Level.

In that part of the book that treats upon Levelling, in addition to the mere description of the nature and object of LEVELLING generally, and the use and adjustments of the *Instrument* and the reading of the Staff, have been introduced Field Notes of three miles of levels, taken on a line of railway, with the several Cross Sections. The method of computing the lowering and raising of the approaches have been given; and Drawings of the sections and cross sections, conformably to the regulations required for plans, that are intended for parliamentary deposit, have been added for illustration.

The several methods, whether correct or incorrect, adopted in practice, for calculating the CUTTINGS AND EMBANKMENTS, have been carefully investigated and examined, and corrections given for those that are wrong; and practical examples have been annexed, fully worked out, by the *Prismoidal Formula*—by Bidder's tables—and checked by a new Formula, which is specially adapted for cuttings of *any* length and height, for which I am indebted to Professor Moseley, of King's College.

And, lastly, there is an Appendix of tables of Logarithms, Sines, and Cosines; and a Traverse Table of Latitudes and Departures to any distance, and to three minutes of bearing.

I have now to acknowledge my obligation to the works of several authors, to whom I am indebted for much valuable assistance: among others, to Hall, Bridges, Keith, Hutton, Gummere, &c., &c.

Having intended these pages, as much for those who may have no opportunity of obtaining the assistance of a master, as for those who have, I have, in every case, annexed examples for practice;—to some of them have been given answers; to others, when the same result could be obtained by two or three methods, as additional practice for the Student, the answers have been omitted.

Before concluding, I have only to appeal to the kind indulgence of my readers for any mistakes that may have crept into the work, assuring them, that, as I shall have full opportunity of checking every example among my own Pupils, those mistakes shall be duly corrected in the next Edition.

30, *Queen Square, Bloomsbury,*

March 2, 1842.

P.S.—On further consideration, I have given, at the end of the work, the *corrected and omitted Answers to all the Examples.*

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AND

ONE HUNDRED AND FORTY

WOOD CUTS.

INTRODUCTION.

GEOMETRY.

GEOMETRICAL DEFINITIONS.

A point has neither length, breadth, nor thickness.

A line has length.

A plane, or superficies, has length and breadth.

A solid has all three—length, breadth, and thickness.

An angle is the inclination of two straight lines meeting in a point.

A right angle is the inclination of one straight line to another, when, if either be produced, the second angle will be equal to the former.

An obtuse angle is greater: an acute angle, less : than a right angle.

Parallel lines never meet.

A parallelogram is a four parallel sided figure.

A rectangle is a right angle parallelogram.

A square is an equal sided rectangle.

A rhombus is equilateral, but not equiangular.

A rhomboid is neither equiangular nor equilateral.

A circle is a plane figure bounded by its circumference, every part of which is equidistant from the centre. The radius is this distance from the centre.

An arc is a portion of this circumference.

The chord of an arc is the straight line, joining the extremities of the arc.

A segment is the space, included between the chord and the arc.

The sector of a circle is the space, included between the two sides, subtending the angle, and the arc; therefore the sector of a right angle is a quadrant.

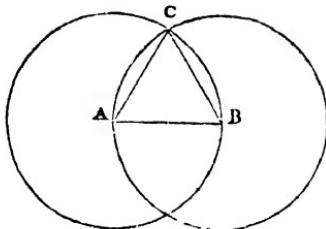
GEOMETRICAL PROBLEMS.

1. To describe an equilateral triangle upon a given line.

Let AB be the given line.

From A and B, with the radius AB, describe two circles intersecting in C; join AC and BC.

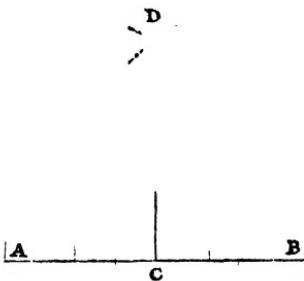
ABC is the equilateral triangle required.



2. From a point, within a given line, to erect a perpendicular.

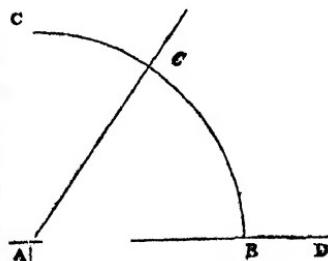
Let AB be the given line, and C be the given point.

From C, as a centre, take any distance, CA, and make CB=CA. Then from A, and B, as centres, at the distance AB, describe arcs intersecting at D; join the point of intersection at D with the point C. CD will be perpendicular to AB.



At a given point, in a given line, to construct a right angle, or an angle of any number of degrees, by means of a scale of chords; let AD be the given line, and A the given point.

Take off with your compasses, on any scale of chords, AB, the chord of 60 degrees (*radius*); and from the given point A, with that distance, describe a circle, intersecting the given line at B; then, from the point of intersection, with the distance of the chord of 90 degrees, BC, or of the chord of any other angle that may be required, BC, on the same scale, describe another

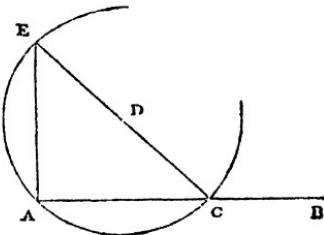


circle, intersecting the former. Join the points of intersection with the given point, and the lines will be perpendicular to, or making the required angle, with the given line. A protractor, which is a semi-circle graduated into 180 degrees, and numbering, both from right to left, and from left to right, may be used, but not so accurately, for the same purpose.

3. From a point at the end of a given straight line to erect a perpendicular.

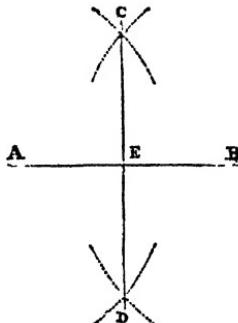
Let AB be the given straight line, and A the point at the end. Take any point D and from D as centre, at the distance DA, describe the circle EAC, and join CD, and produce it to E. Join EA.

EA shall be the perpendicular required.



4. To bisect a given straight line.

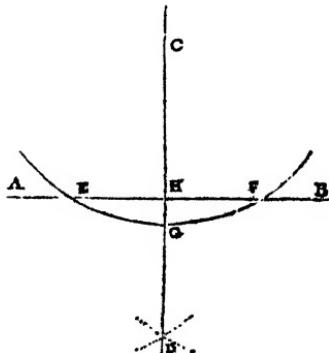
Let AB be the given straight line; it is required to bisect it. Upon AB describe the equilateral triangles, ACB and ADB; join the vertices C and D by the line CD, intersecting AB at E. E shall be the point of bisection.



5. To let fall a perpendicular upon a given straight line, from a given point above it.

Let AB be the given straight line, and C the given point. From C take any distance CG, and describe the circle EGF, intersecting AB in E and F; bisect EF in H, and join CH.

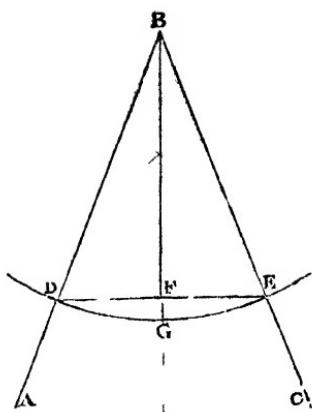
CH is the perpendicular required.



6. To bisect a given angle.

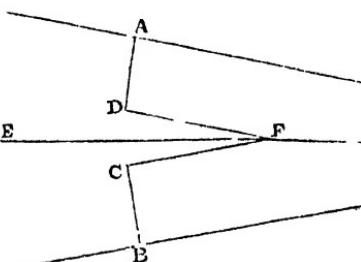
Let ABC be the given angle; take any point D in AB , and from B , as centre, at the distance BD , describe the circle DGE ; join DE , and bisect it in F ; join BF .

BF shall bisect the angle ABC .



7. To bisect a given angle, when the inclination of the two sides can only be obtained, and not the vertex of the angle, included between them.

Let A and B be the two sides, such, that they cannot be produced. Take any points, A and B , and draw the equal perpendiculars AD and BC ; through D and C draw DF and CF parallel to A and B respectively; the angle DFC will be equal and similarly situated to the angle at the vertex, which will be bisected by the line EF (*produced*), that bisects the angle DFC .

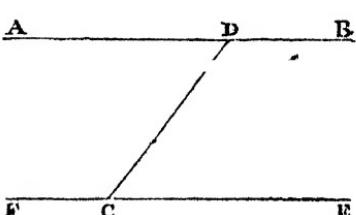


8. To draw a line parallel to a given line.

Through the point C to draw a line parallel to the given straight line AB .

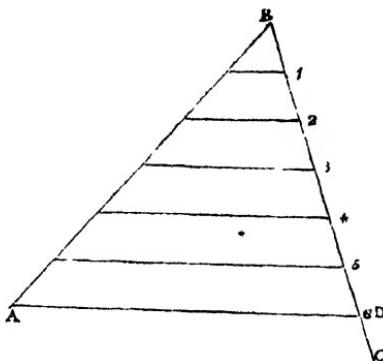
Take any point D in AB , join CD , and at the point C make the angle DCE equal to angle ADC ; produce EC to F .

EF shall be parallel to AB .



9. *To divide a given line, AB, into any number of equal parts.*

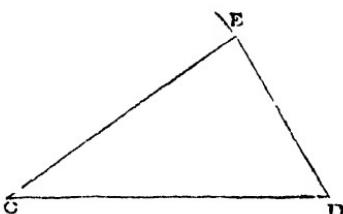
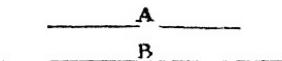
At the point B, making any angle with AB, draw the unlimited straight line BC; take the required number of (six) equal measurements of any length from B towards C, ending at D; join DA, and through the several points on BD, draw lines, parallel to AD, to the line AB; these lines will intersect AB equally, and will be of the required number.



10. *Upon a given base, to describe a triangle, whose other two sides shall be respectively equal to two given lines, any two of which lines, however, must be greater than the third.*

Let A and B be the given lines, and CD be the given base; any two of them being greater than the third. It is required to describe upon CD a triangle, whose other two sides shall be equal to A and B.

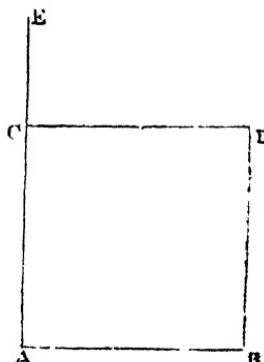
At the centre C, with the distance CE, equal to B, describe a circle; and at the centre D, with the distance DE, equal to A, describe another circle, intersecting the first in E; join EC and ED. The triangle CED shall be the triangle required.



11. *To describe a Square on a given line AB.*

From the point A erect a perpendicular to AB; make AC equal to AB; and through the point C draw CD parallel to AB; make CD equal to AB, and join DB.

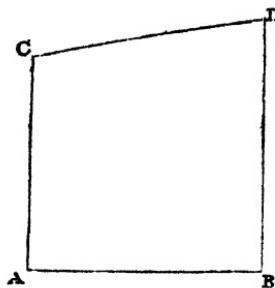
ACDB shall be the square required.



12. To construct a trapezoid, having its two perpendiculars and its base given.

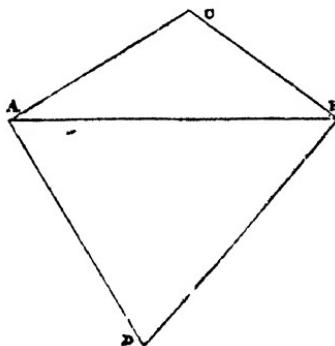
At the points A and B, of the base AB, erect two perpendiculars of the length required, and join CD.

ACDB shall be the figure required.



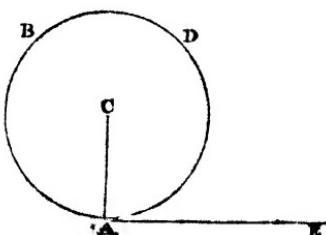
13. To construct a trapezium, ACBD, having the sides and the diagonal given.

Conceive the trapezium divided into two triangles, A~~C~~B and ADB, having the common base AB, which is the given diagonal. Draw the base AB, and upon it, on their respective sides, construct the required triangles (prop. 10) ADB, ACB; having the sides AD, DB; AC, BC, of the required length.



14. From a given point A, in the circumference, to draw a tangent to the circle ABD, whose centre is C.

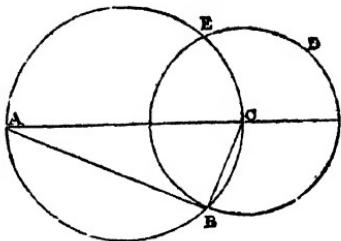
Join CA, and make EA perpendicular to AC; EA shall be the tangent required.



GEOMETRICAL PROBLEMS.

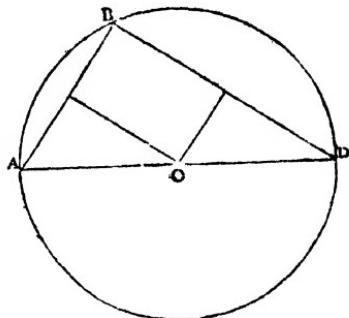
15. *From a given point A, without the circumference, to draw a tangent to the circle BED.*

Join AC, and upon AC describe the circle AECB. Join AB. AB shall be the tangent required; for ABC is a right angle, being in a semicircle, and therefore AB is at right angles to CB, which is the radius, and is therefore the tangent required.



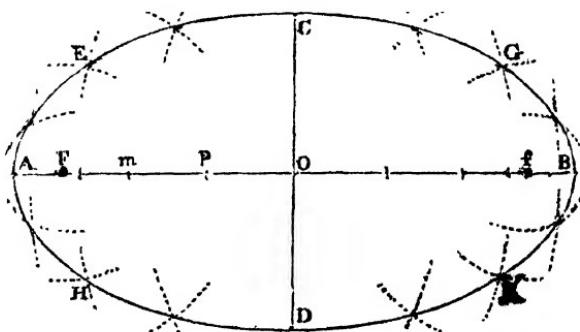
16. *Through the three given points A, B, D, not in a straight line, to describe a circle.*

Join AB and BD, and bisect them; erect perpendiculars till they meet in C; C will be the centre of the circle.



17. *To construct an ellipse, whose transverse and conjugate diameters are given.*

METHOD 1.



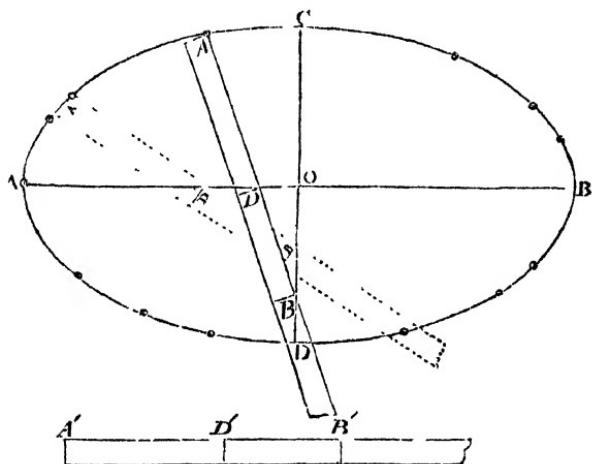
Let AB be the transverse diameter; bisect it in O; erect OC and OD perpendicular to AB, and equal each of them to the semi-conjugate; from C, as a centre, at the distance of the semi-transverse, describe the arcs intersecting AB in F and f. F, f are the foci. Then taking any point whatever m, and from F and f as centres, with distances Am, Bm, describe the arcs intersecting at E, G, H, K; thes

four points of intersection are points in the curve; again, by taking any other point p , and proceeding in the same way, four other points may be obtained.

By increasing the number of points taken upon the semi-transverse Ao , any number of points in the curve, that may be required, can be determined.

Through these points the circumference of the ellipse must be afterwards drawn.

METHOD 2.*

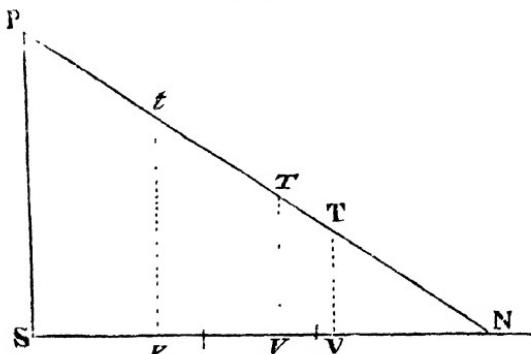
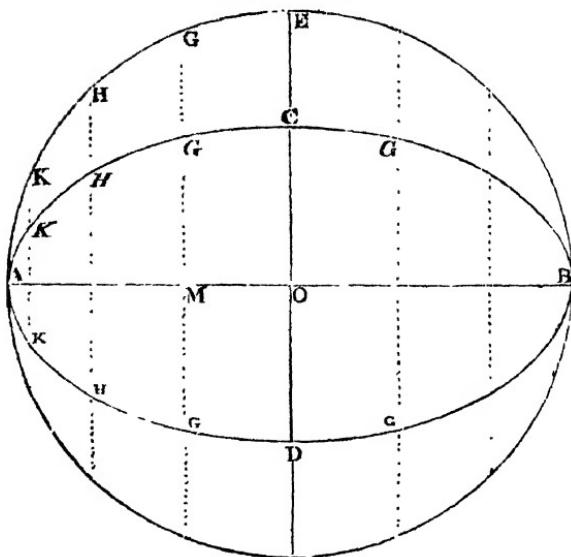


Having constructed the transverse and conjugate diameters, take a piece of card-board, with a fine clear edge, about the length of the transverse, and mark off upon it, from the same end, both the semi-transverse and semi-conjugate, so that $AB=AO$, and $AD=CO$; then, by keeping the semi-transverse mark B always in the semi-conjugate line CD , and the semi-conjugate mark D always, at the same time, in the semi-transverse line AB , and moving the point B upwards from D to C , and the point D sideways from O to A , the end of the card A will describe points of the curve between C and A .

By adopting the same method in the other quarters of the ellipse, the whole of the curve can be determined.

* This is a purely mechanical process, called striking the ellipse by the trammel. It is, however, a very correct and simple method, and one very much in use among Architects and Engineers.

METHOD 3. *



Construct the transverse and conjugate diameters as before, and upon the transverse, as diameter, describe a circle; then, because the ordinates of this circle are, to the corresponding ordinates in the ellipse, as the semi-transverse is to the semi-conjugate, we obtain a very simple method of determining the points in the curve.

Describe any right-angled triangle PNS, such that PN shall be to PS as the semi-transverse is to the semi-conjugate, then all lines TV , TV' , tv , drawn parallel to PS, will have the same proportion to all lines TN , $T'N$, as PS has to PN , or as the semi-conjugate has to the semi-transverse. Draw any ordinate to the circle, GM , and in the triangle PNS, cut off NT equal to GM ; let fall the perpendicular TV , (practically, place one leg of your compasses in T , and

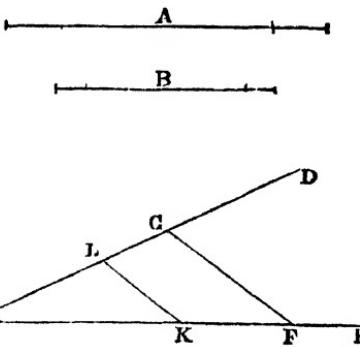
expand them till, describing an arc, the other leg touches the lines SN,) and measure off this distance (MG) on the line MG; MG is the corresponding ordinate of the ellipse, and G is a point in the curve. Produce GM to g; making Mg equal to MG; this is also another point. By taking equal abscissa from O to B, two other points, G and g, on the other side of DC, may be obtained also.

The other points H,K,L, are determined in the same way, by measuring the ordinates of the circle on NP, and letting fall perpendiculars, for the corresponding ordinates of the ellipse.

18. To find a third proportional to two given lines.

Let A,B be two given lines.
Draw any two unlimited lines, CD and CE, making any angle between them. From CE cut off CF, equal to A; and from CD, CG, equal to B, join GF; again, take CK, equal to CG, and through K draw KL parallel to GF. CL is a third proportional to A and B, because, by similar triangles,

$CF : CG :: CK : CL$.



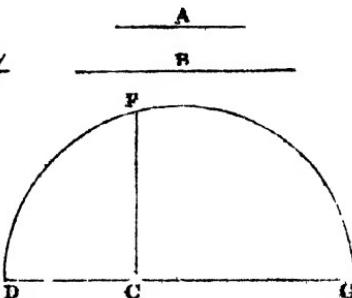
19. To find a fourth proportional to three lines.

Proceed as in the above, but instead of taking CK, equal to CG or B, take CK equal to the third line; then CL in this case also becomes the fourth proportional.

20. To find a mean proportional between two lines A and B.

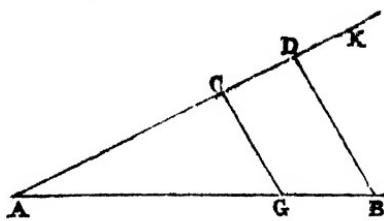
Draw any line DG; make DC equal A, and CG equal B; upon ~~DG~~^{as diameter}, describe a circle; erect the perpendicular CF, which is the mean proportional required. For $DC : CG :: CF : CG$.

$\therefore DC : CF :: CF : CG$.



21. To divide a given line, AB, into proportional parts.

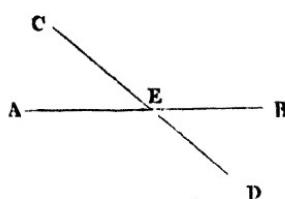
Through A draw any unlimited line AK, and take AC and CD of the required proportions; join DB, and through C draw CG parallel to DB. AG and GB, are the parts required.



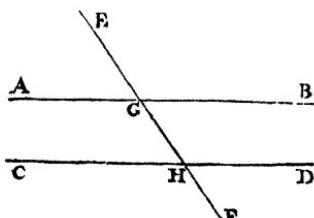
For, by similar triangles, $AC : AD :: AG : AB$,
convertendo. $AC : (AD - AC) :: AG : (AB - AG)$;
i. e. $AC :: CD :: AG :: GB$.

USEFUL THEOREMS.

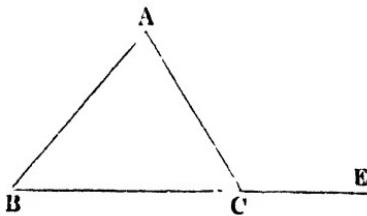
1. If two straight lines cut one another, the opposite angles are equal, and the four angles are together equal to four right angles; i.e., the angle AEC is equal to the angle DEB, and the angle CEB to the angle AED.
(*Euclid I, 15.*)



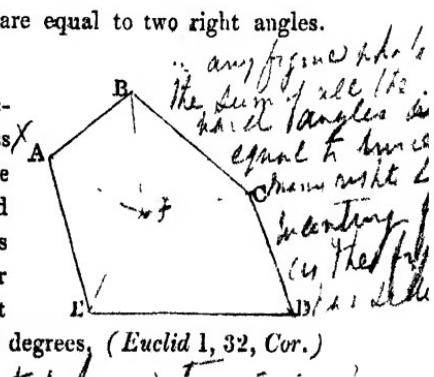
2. The alternate angles, made by a line cutting two parallel lines, are equal; and the exterior and interior angles are equal; and the two interior angles are, together, equal to two right angles; i.e., the alternate angles AGH, GHD are equal; the exterior EGB equal to the interior GIID; and the two interior, BGH, GHD, equal to two right angles. (*Euclid I, 29.*)



3. If any side of a triangle be produced, the exterior angle is equal to the two interior opposite angles; and the three interior angles are equal to two right angles; i.e., the angle ACE is equal to the two at A and B; and the angle ACB, together with the two at A and B, are equal to two right angles.
(*I, 32.*)



4. All the interior angles of any rectilineal figure are equal to four less than twice as many right angles as the figure has sides; i.e., in the five-sided figure ABCDE, all the interior angles at A, B, C, D, and E, are equal to four less than twice five right angles; that is, are equal to six right angles or 540 degrees, (*Euclid I, 32, Cor.*)



5. The greatest angle of every triangle is opposite the greatest side. (*Euclid I, 18.*)

6. Parallelograms, upon equal bases and between the same parallels, that is, having the same perpendicular height, are equal to each other; i.e., the parallelogram ABCD=the parallelogram DBCE.

Triangles, alike situated, are also equal; i.e., the triangle ABC=the triangle CFE. (*Euclid I, 35—38.*)

7. In every triangle, the square of the side, subtending any angle, is greater than, equal to, or less than, the square of the sides containing the angle, according as the angle is obtuse, right-angled, or acute; and the excess or deficiency is equal to twice the rectangle of one of the sides containing the angle, and that portion of it intercepted between the angle and a perpendicular, drawn to it from the opposite angle; i.e.,—

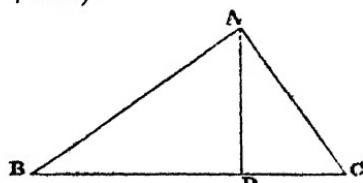
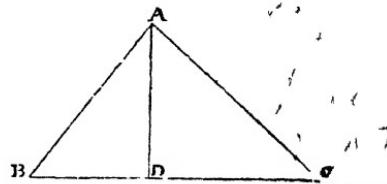
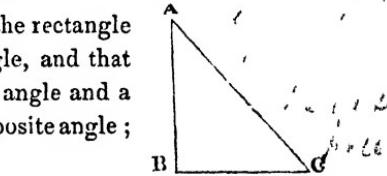
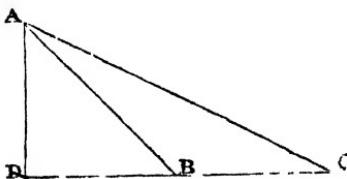
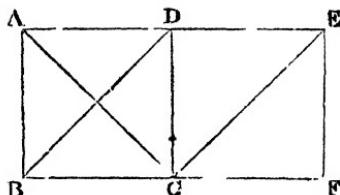
In the obtuse-angled triangle;
 $AC^2 = AB^2 + BC^2 + 2CB \cdot BD$.

In the right-angled triangle;
 $AC^2 = AB^2 + BC^2$.

In the acute-angled triangle;
 $AC^2 = AB^2 + BC^2 - 2CB \cdot BD$.

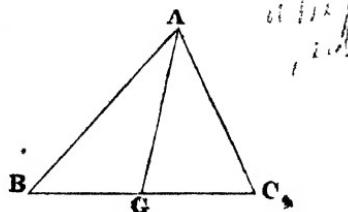
8. In any triangle, the sum of the squares of any two adjacent sides is equal to twice the square of half the other side, together with twice the square of the line drawn from the vertex to the point of bisection, i.e., $AB^2 + AC^2 = 2(BD^2 + DA^2)$.

9. In any right-angled triangle, if a line be drawn at right angles to the hypotenuse, this line is a mean proportional between the segments of the hypotenuse; and the base and perpendicular are, respectively, mean



proportionals between the hypotenuse and the segment adjacent to them; i.e., $BD \cdot DC = DA^2$, $CB \cdot BD = BA^2$, and $BC \cdot CD = CA^2$. (Euclid VI, 8.)

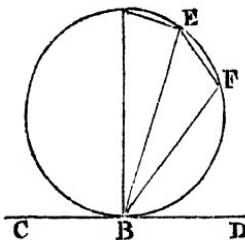
10. The rectangle of any two sides of a triangle is equal to the rectangle of the segments of the base, together with the square of the line, bisecting the included angle; i.e., $BA \cdot AC = BG \cdot GC + GA^2$ (Euclid VI, B), and equal to the rectangle of a perpendicular drawn from this angle and the diameter of the circumscribing circle. (Euclid VI, C.)



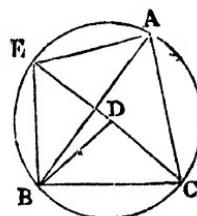
11. In every parallelogram, the squares of the two diagonals are together equal to the squares of all the sides.

12. Of any quadrilateral inscribed in a circle, the rectangle of the diagonals is equal to the two rectangles contained by its opposite sides. (Euclid VI, D.)

13. The angle, in a semicircle, is a right angle; in a segment, less than a semicircle, greater than a right angle; in a segment, greater, less; i.e., AB being the diameter, the angle AEB is a right angle; the angle EFB, being in a segment less than a semicircle, is greater than a right angle; and the angle BAE is less. (Euclid III, 31.)



14. Angles, standing upon equal circumferences, are equal, whether they be in the centre or the circumference; i.e., if the circumference BE be equal to the circumference BC, the angles EAB, BAC, in the circumference; and the angles EDB, BDC at the centre, are equal; and the angles EDB, BDC are respectively double the angles EAB, BAC. (Euclid III, 20 and 27.)



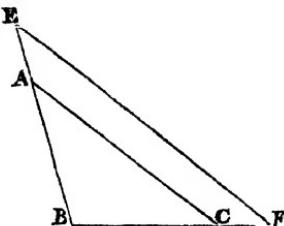
15. The two opposite angles of any quadrilateral figure, inscribed in a circle, are equal to two right angles; i.e., the angles BEA and ACB, and the angles EBC, EAC, are equal to two right angles (Vide figure in Theorem 14). (Euclid III, 22.)

16. If a line touch a circle, any angle made between this line at the point of contact, and a line cutting the circle, is equal to the angle in the alternate segment; i.e. (See Fig 13), the angle CBE equals the

angle EFB, and the angle CBA equals the angle AEB. (*Euclid III, 32.*)

17. Parallelograms, and triangles of equal altitude, are as their bases; and of equal bases, are as their altitudes. (*Euclid VI, 1.*)

18. Equiangular triangles are similar, or have their homologous sides proportional. (*Euclid VI, 4.*) Let the triangles ABC, EBF, be equiangular; $AB : BC :: EB : BF$ and $BA : AC :: BE : EF$ and $BC : CA :: BF : FE$. (*Euclid VI, 4.*)



19. Similar triangles have their areas, proportional to the square of their homologous sides. (*Euclid VI, 19.*)

20. Circumferences of circles are proportional to their radii. Areas of circles, to the squares of their radii. And, generally, all lineal measurements are proportional to their respective units of measurement.

21. All superficial areas are proportional to the compound ratio of the sides, that measure their areas; and, when these sides are similar, to the duplicate ratio of any one of them.

22. All solids also are proportional to the compound ratio of their measuring sides, and, when similar, to the triplicate ratio of any one of them.

but one integer, the index is 0: if they are all decimals, bring the first significant decimal figure into the fraction of an integer, and the power of 10, contained in the denominator, will in all cases be the index, with a negative sign—or, the index will be the number of figures, that this significant figure is removed from the unit place of the whole number, thus—

| | | | |
|------|---------|---|-----------|
| log. | 5151 | = | 3·711892 |
| log. | 515·1 | = | 2·711892 |
| log. | 51·51 | = | 1·711892 |
| log. | 5·151 | = | 0·711892 |
| log. | ·5151 | = | -1·711892 |
| log. | ·05151 | = | -2·711892 |
| log. | ·005151 | = | -3·711892 |

EXAMPLES.—What are the logarithms of the following numbers?

| | | | | | |
|------|----------------|----------|-----|-------------------|----------------|
| log. | $3 \cdot 1420$ | \times | $=$ | $0 \cdot 497206$ | + the 0 in cf. |
| log. | $41 \cdot 36$ | $=$ | $=$ | $1 \cdot 616581$ | |
| log. | $8 \cdot 910$ | $=$ | $=$ | $0 \cdot 949878$ | |
| log. | $647 \cdot 1$ | $=$ | $=$ | $2 \cdot 810971$ | |
| log. | $\cdot 000672$ | $=$ | $=$ | $-4 \cdot 827369$ | |
| log. | $3 \cdot 004$ | $=$ | $=$ | $-0 \cdot 477700$ | |

x If it should be found that the logarithms of the given number cannot be exactly found in the tables, the simple rule of proportion will obtain it.

Thus—As the difference between the next lower and the next higher number : the difference between the next lower and next higher logarithm :: so is the difference between the next lower and the given number : the difference between the next lower logarithm and the logarithm required.

For example—to find the logarithm of 235.756:

The log. of the next lower number 2357 is .372860
 And of the next greater 2358 is .372544

Their difference being

Then say, as $100 :: \cdot 000176 :: 56 : \cdot 000098$ which, added to the log. of $235\cdot 7$, which is $2\cdot 37236$, is equal to $2\cdot 372458$, the logarithm of the number required.

This method may be useful to a beginner, but to any one, who understands decimals, I should recommend the following in preference.

In taking the same example, observe that the logarithms of the next lower and next higher numbers have the common figures 872, and a difference of 176 only, between the next three figures.

Now, by the principles of decimals, each (decimal) figure is itself a decimal to the one immediately preceding; the figures '56, therefore, in the given numbers, are decimals to the preceding figure 7; the measure, of the difference between 7 and 8, or of an unit of difference, being 176, the figures to be annexed to the next lower logarithm must be less than 176, and must depend upon the proportion, that the given decimal '56 bears to an unit.

Multiply, therefore, this difference, or measure of the unit (176), by the given decimal '56, and you obtain the proportion of the 176, to be added to the next lower logarithm.

Ex. difference of logarithm = 176

$$\begin{array}{r} \text{decimal of numbers} = \underline{.56} \\ \phantom{\text{decimal of numbers}} \quad \underline{1056} \\ \phantom{\text{decimal of numbers}} \quad \underline{880} \end{array}$$

The figures to be added to $\underline{98\cdot56}$

the next lower logarithm = $\underline{2\cdot372360}$

The required logarithm = $2\cdot372458 = \log. 235\cdot756$

What are the logarithms of the following numbers?

$$\begin{array}{rcl} \log. 3421\cdot56 & = & 3\cdot534228 \\ \log. 2987\cdot245 & = & 3\cdot475271 \\ \log. .0342172 & = & -2\cdot534244 \end{array}$$

To find the number, answering to a given logarithm.

Seek in the tables for the given logarithm, opposite which, in the left hand column, is the number answering to the given logarithm.

What are the numbers corresponding to the following logarithms?

$$\begin{array}{rcl} 2\cdot848855 & = & \log. 698\cdot00 \\ -3\cdot530200 & = & \log. .00339 \\ 1\cdot301030 & = & \log. 20\cdot000 \\ 4\cdot600973 & = & \log. 39900 \\ -2\cdot714338 & = & \log. .05180 \\ 5\cdot804412 & = & \log. 637400 \\ -1\cdot937518 & = & \log. 0\cdot86660 \end{array}$$

When the given logarithm cannot exactly be found in the tables, seek for the next lower logarithm; opposite which will be found the next lower number corresponding thereto; then say,—

As the difference between the next lower and the next higher logarithm, is to the difference between the next lower and the next higher number, taken as an unit, so is the difference between the next

lower logarithm and the given logarithm, to the difference between the next lower number and the number required, or to the decimal of that unit, to be annexed) to the number of the next lower logarithm.

For example, to find the number answering to the logarithm 4.127860, the next lower logarithm is .127753; the difference of this and the next greater is 323; and the number answering to the former is 1342.

Then, as .323 : 1 :: .107 : .333, which, annexed to the first number, will be 1342333, and, pointing off according to the index, is 13423.33, the required number; or, briefly, the decimal to be annexed, is the decimal of a vulgar fraction, whose *denominator* is the difference between the next lower and next higher logarithms (323), or the logarithmic measure of an unit; and the *numerator* the decimal of that unit, or the difference between the given logarithm and the next lower (107); viz. $\frac{107}{323} = .333$.

EXAMPLES.—What are the numbers corresponding to the following logarithms?

$$\begin{aligned} - 3.246418 &= \log. 0017636727 \\ - 3.517694 &= \log. 3293.889 \\ - 8.106457 &= \log. 0'00000001277783 \end{aligned}$$

* *Multiplication* of numbers, by logarithms, is performed by finding the logarithms of the numbers; adding them, and finding the number answering to their sum. *Division*, by subtracting them.

EXAMPLES.—Divide 56.05 by .075.

$$\begin{array}{rcl} \log. 56.05 &=& 1.748576 \\ \log. \underline{.075} &=& -2\ 875061 \\ \log. 747.4 &=& -2.873515 \end{array}$$

$$\text{Multiply } 50 \text{ by } 75 = 3750$$

$$\text{Multiply } 48 \text{ by } 24 = 1152$$

$$\text{Divide } 4.167 \text{ by } 38.56 = 0.108065$$

$$\text{Divide } 42.167 \text{ by } .061089 = 690.255217$$

INVOLUTION.

Find the logarithm of the number; multiply it by the index of power required, and find the number answering to the product.

EXAMPLES.—Required the cube of 5.68.

$$\begin{array}{rcl} \log. 5.68 &=& 0.754348 \\ \text{index} &=& \underline{\underline{3}} \\ \log. 138.345 &=& 2.263044 \end{array}$$

What is the square of $4\cdot678 = 21\cdot883678$

Required the cube of $321\cdot5 = 33230963\cdot375$

Find the 4th power of $4\cdot23 = 320\cdot15587041$

EVOLUTION.

Divide the log of the number by the index of the root, and find the number answering to the quotient.

EXAMPLES.—What is the cube root of $57\cdot08$?

$$\log. 57\cdot08 = \underline{1\cdot75684}$$

$$\text{div. by } 3 = \log. 385 = \underline{0\cdot58594}$$

$$\text{Find the square root of } 5\cdot008 = \underline{2\cdot237856}$$

$$\text{What is the 5th root of } 67\cdot40 = \underline{2\cdot3213}$$

$$\text{Required the cube root of } 6 = \underline{1\cdot81712092}$$

Note.—That, in Involution, if the index of the logarithm of the number be negative (the logarithms of decimals being always affirmative), it will be necessary to multiply them separately.

EXAMPLE.—To find the cube of $\cdot05$

$$\begin{array}{r} \log. \cdot05 = -2\cdot698970 \\ \quad \quad \quad 3 \end{array}$$

$$\begin{array}{r} 2\cdot096910 \\ -2 \times 3 = -6 \\ \hline \end{array}$$

$$\log. \cdot000125 = \underline{4\cdot096910}$$

Also, in Evolution, where the index is negative, and is not some multiple of the index of the root, add to each, viz. (the index and the log.), some number that will make the index a multiple required, and divide the two separately, as in Involution. *make it a multiple of the index of the root*

EXAMPLE.—Find the 6th root of $\cdot05$.

$$\begin{array}{r} \log. \cdot05 = -2\cdot698970 \\ \quad \quad \quad 6 \\ \equiv -2\cdot +698970 \\ \text{Add} -4\cdot +4 \\ \hline -6\cdot +1098970 \end{array}$$

and dividing by 6 = — 1.183161, the number answering to which is .01525.

EXAMPLES TO THE TWO LAST CASES.

Required the cube of $\cdot105 = 0\cdot0011576245$.

What is the square of $\cdot00534 = 0\cdot0000285156$

Find the 4th power of $\cdot00062 = 0\cdot000014776336$

What is the 4th root of $\cdot365 = 777272325$

Required the cube root of $\cdot0003214 = \cdot068498635$

What is the 10th root of $\cdot0016 = 0\cdot525305609$

If the given number be a fraction, to find the logarithm.

Rule.—Subtract the logarithm of the denominator from that of the numerator, and the remainder is the logarithm of the fraction required.

EXAMPLES.—Required the logarithm of $\frac{7}{8}$.

$$\log. 7 = 0.84510$$

$$\log. 8 = 0.90309$$

$$\log. \frac{7}{8} = -1.94201$$

Required the logarithms of the following fractions, $\frac{3}{7}$, $\frac{3}{16}$, $\frac{5}{16}$, $\frac{13}{48}$,

$$\log. \frac{3}{7} = -1.5228787$$

$$\log. \frac{3}{16} = -1.7569620$$

$$\log. \frac{5}{16} = -1.4748500$$

$$\log. \frac{13}{48} = -1.6320232$$

$$\log. \frac{3}{32} = -3.4894550$$

$$\log. \frac{5}{768} = -4.8045432$$

MENSURATION OF PLANES.

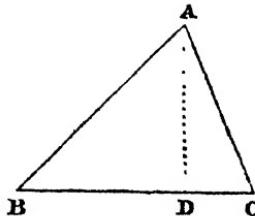
PROBLEM I.

To find the area of a triangle, when the base and perpendicular height are given.

Rule.—Multiply the base by $\frac{1}{2}$ the height.

EXAMPLE 1.—What is the area of a triangular field, whose base is 5 chains 50 links, and height 3 chains 20 links?

$$\begin{array}{r} \text{base} = 5.50 \text{ chains} \\ \frac{1}{2} \text{height} = 1.60 \text{ chains} \\ \hline & 33000 \\ & 550 \\ \hline 10 & | 8.8000 \text{ sq. chains} \\ & \cdot 88000 \text{ acres} \\ & 4 \\ \text{roods} & 3.52 \\ & 40 \\ \hline \text{perches} & 14.080 \end{array}$$



A. R. P.
Answer 0. 3. ~~14~~ 12

EXAMPLE 2.—Required the area of a triangle, whose base is 7 chains 25 links, and perpendicular height 90 links. A. R. P.

Answer 0. 1. 12

EXAMPLE 3.—How many square yards are contained in a triangular plot of ground, whose base and height measure respectively, 8 chains 50 links, and 5 chains 50 links?

$$\begin{array}{r} \text{base} = 8.5 \text{ chains} \\ \text{height} = 5.5 \text{ chains} \\ \hline & 425 \\ & 425 \\ \hline 2 & | 46.75 \\ \hline 10 & | 23.75 \text{ sq. chains} \\ & 2.375 \text{ acres} \end{array}$$

multiply by the sq. yards in 1 acre = 4840
area = 11313.5 sq. yards.

NOTE.—[The statute English measure of an acre is ten square chains, (each chain being 66 feet or twenty-two yards,) or 4840 square yards, or 43560 square feet. Areas are generally reckoned in acres, rods, and perches, square measures ; 4 square rods being equal to an acre, and 40 square rods to a rood.]

PROBLEM II.

To find the area of a triangle, when the three sides are given.

Rule.—Take half the sum of the three sides, subtract each side severally from this quantity ; then multiply this and the three remainders together, and take the square root for the area.

EXAMPLE 1.—What is the area of a triangle, whose three sides are 30, 40, and 50 chains ?

$$\frac{30 \times 40 \times 50}{2} = 60 = \text{the half sum.}$$

| | | |
|----|----|----|
| 60 | 60 | 60 |
| 30 | 40 | 50 |

$$30 \times 20 \times 10 \times 60 = 360000 \text{ sq. chains.}$$

$$\text{Answer} = \sqrt{360000} = 600 \text{ sq. chains} = 60 \text{ acres.}$$

EXAMPLE 2.—Required the area of a triangular field, whose three sides measure respectively 25 chains, 42 chains, and 56 chains ?

| | | |
|----|----|----|
| A. | R. | P. |
| 25 | 42 | 56 |

Answer ~~27.2 28.84~~ 49

NOTE.—Where the lengths of the sides are not full chains, as in the following example, the solution must be obtained by the general logarithmic expression, viz.:—

$\log. \text{area} = \frac{1}{2} \cdot \log. s \times \frac{1}{2} \cdot \log. s - a \times \frac{1}{2} \cdot \log. s - b \times \frac{1}{2} \cdot \log. s - c$
where s = the semi-perimeter, and a, b, c = the sides.

EXAMPLE 3.—The three sides of a triangular plot of ground are respectively 20 chains 40 links, 25 chains 20 links, and 30 chains 50 links. What is the area ?

| | | |
|--------------|--|--|
| 20·40 chains | | |
| 25·20 | $38\cdot05 - 20\cdot40 = 17\cdot65 = \overline{s-a}$ | |
| 30·50 | $38\cdot05 - 25\cdot20 = 12\cdot85 = \overline{s-b}$ | |
| 2 76 10 | $38\cdot05 - 30\cdot50 = 7\cdot55 = \overline{s-c}$ | |
| | 38·05 = s. | |

$$\log. s = 38.05 = 1.5803547$$

$$\log. \overline{s-a} = 17.65 = 1.2467447$$

$$\log. \bar{s} - \bar{b} = 12.85 = 1.1089031$$

$$\log \frac{s-c}{s+c} = 7.55 = 8.8779470$$

$$\begin{array}{r}
 2 | \underline{4 \cdot 8139495} & \text{sq. chains} & \text{acres} \\
 2 \cdot 4069748 = \log. 255 \cdot 255 = 25 \cdot 5255 \\
 & & \hline
 & & 4 \\
 & & 2 \cdot 1020 \\
 & & \hline
 & & 40 \\
 & A. & R. & P. \\
 \text{Area} = 25. & 2. & 4 & = 4 \cdot 0800
 \end{array}$$

EXAMPLE 4.—Given the sides of a triangle, 24 chains 72 links; 38 chains 75 links; and 44 chains 68 links; to ascertain the area in square yards.

Answer 31700 sq. yards.

PROBLEM III.

To find the area of a triangle, having two of its sides, and the included angle given.

Rule.—Multiply half the product of the two given sides by the natural sine of the given angle; or, where it is desirable to use logarithms, apply the following expression, viz.:—

$$\text{Log. area} = \log. \frac{b}{2} + \log. c + \log. \sin A - 10.$$

EXAMPLE 1.—What is the area of a triangle, whose two sides are 40 and 50 chains, and their contained angle 37 degrees 24 minutes?

Natural sine A, $37^{\circ} 24'$ = .60738

40 × 50 1000

10 | 607·38 sq. chains

60·738 acres

4

2·952

10

38-080

68

A. R. P.

EXAMPLE 2.—What is the area of a triangle, whose sides are 42 chains, 56 links, and 11 chains 28 links, and the included angle 46 degrees 15 minutes?

$$\begin{array}{r}
 \log. \frac{AB}{2}, 21\cdot28 \text{ chains} = 1\cdot3279716 \\
 \log. AC, 11\cdot28 \text{ chains} = 1\cdot0522091 \\
 \log. \sin. A, 46^\circ 15' = 9\cdot8587561 \\
 \hline
 & & 12\cdot2390368 \\
 \text{acres} & \text{— radius} = & 10 \\
 \log. 17\cdot339 = 173\cdot39 \text{ chains} = & \hline & 2\cdot2390368 \\
 \hline
 & 4 & \\
 & 1\cdot356 & \\
 & 40 & \\
 \hline
 & 14\cdot240 & \\
 & & \text{A. R. P.} \\
 & & \text{Area} = 17. 1. 14
 \end{array}$$

EXAMPLE 3.—Required the area of a triangular field, two of whose sides are 456 feet, and 327 feet, and the included angle 17 degrees 15 minutes?

A. R. P.
0. 2. 1 $\frac{1}{2}$

What is the area of a triangle, whose sides are 11 yards and 24 yards, and the included angle 165 degrees 46 minutes?

A. R. P.
Answer 1. 0. 1

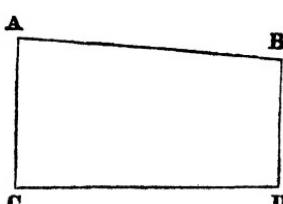
PROBLEM IV.

To find the area of a trapezoid.

Rule.—Multiply half the sum of the two parallel sides by the distance between them.

EXAMPLE 1.—In a trapezoid, whose parallel sides are 7 chains 25 links and 8 chains 63 links, the distance being 11 chains 65 links, how many acres are there?

$$\begin{array}{r}
 7\cdot25 \\
 8\cdot63 \\
 \hline
 2 | 15\cdot88 \\
 \quad \text{chains} \quad \text{acres} \\
 \quad 7\cdot94 \times 11\cdot65 = 92\cdot50 = 9\cdot250 \\
 \quad \text{A. R. P.} \\
 \text{Area} = 9. 1. 0 \quad \hline \quad 4 \\
 & & 1\cdot000
 \end{array}$$



EXAMPLE 2.—What is the area of a trapezoid, the parallel sides of which are 14 chains 20 links, and 12 chains 35 links, and the distance between them 27 chains 25 links?

A. R. P.

Answer 36. 0. 28

EXAMPLE 3.—Required the area of a trapezoidal field, whose sides are 40 chains and 27 chains, and distance 15 chains and a half.

A. R. P.

Answer 51. 3. 28

PROBLEM V.

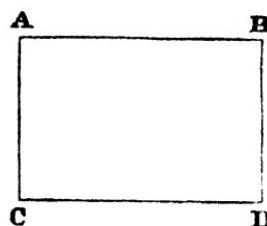
To find the area of a quadrilateral right-angled figure.

Rule.—Multiply the length by the breadth, the product will be the area.

EXAMPLE 1.—What is the area of a rectangular field, whose length is 35 chains 40 links, and breadth 24 chains 36 links?

$$\begin{array}{r} \text{chs.} & \text{chs.} & \text{sq. chs.} & \text{acres.} \\ 35.40 & \times 24.36 & = 862.34 & = 86.234 \\ & & \underline{-} & \\ & & 4 & \\ & & \underline{-} & \\ & & .936 & \\ & & \underline{-} & \\ & & 40 & \\ & & \underline{-} & \\ & & 37.440 & \end{array}$$

A. R. P.
Answer 86. 0. 37



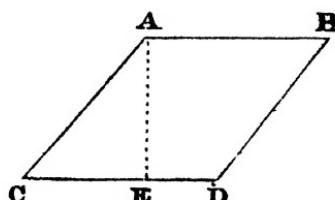
EXAMPLE 2.—Required the area of a right-angled parallelogram, whose length is 56 chains 24 links, and breadth 35 chains 42 links.

A. R. P.
Answer 99. 0. 32

PROBLEM VI.

To find the area of a parallelogram, whose angles are not right angles.

Let ABCD be the parallelogram, whose area is required; let fall the perpendicular AE. The measurement of the parallelogram is AE. CD, and AE is the sine of the angle at C, to radius CA; therefore the area = AC. CD. sine \angle C.



Rule.—Multiply the product of the two adjacent sides by the sine of the angle between them.

EXAMPLE 1.—Required the area of a four-sided regular field, whose sides are 20 chains 15 links, and 16 chains 89 links, and the included angle 30 degrees.

$$\sin \angle 30 = \frac{\text{rad}}{2} = \frac{1}{2} = .5$$

$$\text{AC. } CD = 20.15 \times 16.89 = 340.3335 = 34.03335$$

$$\text{Multiplying by sine } \angle 30 = \frac{\text{chains}}{17.016675} = \frac{\text{acres}}{4}$$

$$\frac{17.016675}{4}$$

$$\frac{.066700}{4}$$

$$\frac{.066700}{40}$$

$$\begin{array}{r} \text{A. R. P.} \\ \text{Answer } 17.0.2 \end{array}$$

$$\frac{.066700}{40}$$

$$\frac{.066700}{2.668000}$$

EXAMPLE 2.—When the sides are 15 chains 25 links, and 21 chains 18 links, and the included angle 45 degrees, what is the area of the field?

$$\begin{array}{r} \text{A. R. P.} \\ \text{Answer } 22.3.14 \end{array}$$

EXAMPLE 3.—With the same sides, but with an angle of 105 degrees, what is the area?

$$\begin{array}{r} \text{A. R. P.} \\ \text{Answer } 31.0.31 \end{array}$$

PROBLEM VII.

To find the area of a trapezium.

Rule.—Divide the trapezium into two triangles by the longest diagonal; take this as the common base of the two triangles, and multiply it by half the sum of the two perpendiculars, let fall upon it, from the opposite angles.

EXAMPLE 1.—To find the area of a trapezium, whose diagonal is 20 chains, and the two perpendiculars 2 chains 50 links, and 3 chains 40 links?

$$\begin{array}{r} 2.50 \\ 3.40 \\ \hline \end{array}$$

$$2 | \underline{5.90}$$

$$2.95 \times 20 = 59 \text{ sq. chains} = 5.9 \text{ acres.}$$

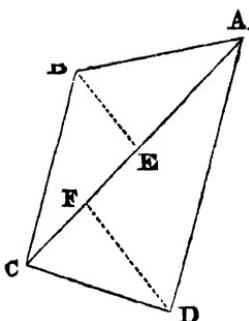
$$\begin{array}{r} \text{A. R. P.} \\ \text{Answer } 5.3.2 \end{array} \quad \begin{array}{r} 4 \\ \hline 3.6 \end{array}$$

EXAMPLE 2.—How many square feet of paving are there in a court yard, whose diagonal is 2 chains 64 links, and perpendiculars 95 links and 84 links?

$$\text{Answer } 10292 \text{ square feet.}$$

EXAMPLE 3.—In the field ABCD on account of obstructions, the following distances only could be taken, viz., AB, 12 chains; BC, 8 chains; and the distances AC, 10 chains; and DC, 6 chains 50 links, (on the diagonal which was 15 chains,) required the area?

Answer A. R. P.
 8. 1. 33



EXAMPLE 4.—Given the distances AB, 6 chains; and AD, 8 chains. In consequence of obstructions, DB could not be measured; though the perpendicular points E, and F, upon it, were ascertained, and the lengths of the perpendiculars were found to be 3 chains and 4 chains. What is the area?

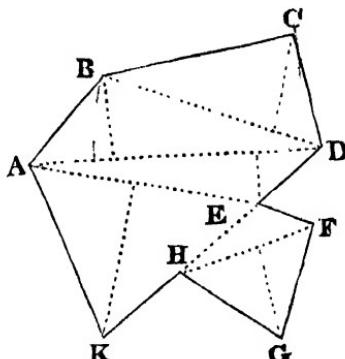
Answer A. R. P.
 3. 3. 38

PROBLEM VIII.

To find the area of an irregular polygon.

Rule 1.—Divide the polygon into trapeziums and triangles. Then find the sum of the areas of each, as in the following example:—

Let ACFK be an irregular polygon, whose diagonals BD, AD, AE, HF, and sides AB, BC, CD, &c., are given, viz. :—



$$\begin{aligned} \text{BD} &= 7 \text{ chains} \\ \text{the diagonals } &\left\{ \begin{array}{l} \text{AD} = 10.00 \\ \text{AE} = 8.00 \\ \text{HF} = 6.00 \end{array} \right. \\ \text{sides} & \end{aligned}$$

$$\begin{aligned} \text{AB} &= 4.00 \\ \text{BC} &= 6.35 \\ \text{CD} &= 3.00 \\ \text{DE} &= 2.50 \\ \text{EF} &= 3.50 \\ \text{FG} &= 4.00 \\ \text{HG} &= 4.00 \\ \text{HK} &= 3.00 \\ \text{HE} &= 3.50 \\ \text{AK} &= 6.00 \end{aligned}$$

chains.

$$\text{Area of } \triangle ABD = 10.928$$

$$\triangle BCD = 9.140$$

$$\triangle ADE = 6.684$$

$$\triangle EFH = 5.408$$

$$\triangle FGH = 7.936$$

$$\triangle AEK = 19.171$$

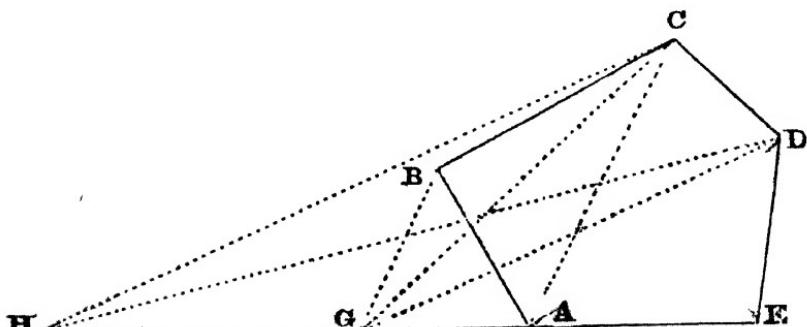
$$\text{Total area of polygon} = \frac{59.267}{5.267} \text{ square chains.}$$

$$\begin{array}{r} 4 \\ -7068 \\ \hline 40 \\ \hline 28.2720 \end{array}$$

Note.—[The lengths of the sides of the figures have been preferred to those of the perpendiculars, because the lengths of the sides would have to be taken, in practice, for the sake of the offsets, and the sides, together with the diagonals, furnish sufficient data for the polygon; whereas the perpendiculars, though more easily measured, and more readily effective, in determining the areas, would be useless for ascertaining the irregularities of the hedges.]

Rule 2.—Construct carefully the given polygon, and reduce it geometrically to an equivalent triangle, as in the following example. Then measure the sides of this triangle, off the scale, by which the polygon was constructed, and find the area. This will be the area of the polygon.

Let ABCDE be the given polygon; it is required to reduce it to an equivalent triangle.



Join AC, through B; draw BG, parallel to AC; join GC; then the triangle GAC is equal to the triangle ABC, being on the same base

AC , and between the same parallels. And, therefore, $GCDE$, the four-sided figure, is equal to the original polygon. Again join GD ; and through C , draw CH parallel to DG ; and join HD . HDE is the triangle required.

PROBLEM IX.

To find the area of any regular polygon.

Let the given polygon be divided into as many triangles as it has sides, by lines drawn from the centre of the circumscribing circle; then the *area of one of these triangles*, multiplied by the number of sides of the polygon, will give the area required.

To find this unit of area.

Let ABD be one of the triangles of a regular polygon, whose side $BD =$ unity, then as $BC \times AC =$ the area of the triangle BAD ,

and $BC = \frac{BD}{2} = \frac{1}{2}$; and $AC = \cot$

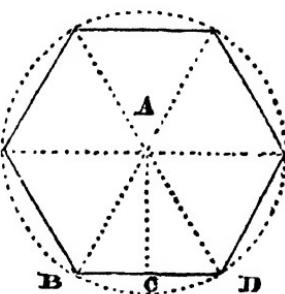
$\angle BAC$ to radius BC , which is equal to $\frac{1}{2} \cot \frac{180^\circ}{n}$, (because $BC = \frac{1}{2} BD = \frac{1}{2}$)

where $n =$ number of sides of the polygon; therefore $\frac{1}{2} \times \frac{1}{2} \cot \frac{180^\circ}{n} = \frac{1}{4} \cdot \cot$

$\frac{180^\circ}{n} =$ the required area of the triangle ABD .

Multiply this by n , the number of sides, and you obtain $\frac{n}{4} \cot \frac{180^\circ}{n} =$ the area of any sided polygon, whose side $BD =$ unity. But areas of similar figures are to each other as the squares of their homologous sides. Therefore $L^2 \cdot \frac{n}{4} \cot \frac{180^\circ}{n} =$ the area of any regular polygon, when $L =$ the length of its side; hence is deduced the

1st Rule.—Square the side of the polygon, and multiply it by one fourth the number of sides, multiplied into the cot. of (180 degrees, divided by the number of sides).



EXAMPLE 1.—Let the side BD of the regular hexagon be equal to 20 chains; it is required to find the area.

$$L^2 \cdot \frac{n}{4} \cot \frac{180^\circ}{n} = \text{area.}$$

$$\begin{array}{r}
 \log. \quad 400 = 2.602060 \\
 + \log. \quad 6 = 0.778151 \\
 + \log. \cot. 30^\circ = 10.238561 \\
 \hline
 & & 3.618772 \\
 - \log. \quad 4 = & 0.602060 \\
 \log. 1039.23 \text{ chs.} = & 3.016712 = \\
 1039.23 \text{ chs.} = 103.923 \text{ acres.}
 \end{array}$$

$$\begin{array}{r}
 4 \\
 \hline
 3.692 \\
 40 \\
 \hline
 27.680
 \end{array}$$

A. R. P.
Answer 103. 3. 27

Rule 2.—Square the side of the polygon, as in the first rule, and multiply it by the tabular area, or multiplier placed, collaterally with it, in the following table.

EXAMPLE 1.—Taking the same data as in the preceding, making the side of the hexagon 20 chains, we have

| No. of sides. | Names. | Areas, side = 1 | |
|---------------|-----------|-----------------|-------------------------------------|
| 3 | triangle | 0.4330127 | $20^2 = 400$ |
| 4 | square | 1.0000000 | tabular area of the |
| 5 | pentagon | 1.7264774 | hexagon = 2.5980762 |
| 6 | hexagon | 2.5980762 | multiply by 400 |
| 7 | heptagon | 3.6339124 | $\text{sq. chs.} = 1039.2304800$ |
| 8 | octagon | 4.8284271 | acres = 103.923 |
| 9 | nonagon | 6.1818242 | $\frac{4}{3.692}$ |
| 10 | decagon | 7.6942088 | $\frac{40}{27.680}$ |
| 11 | undecagon | 9.3656399 | <i>Ans.</i> = 103. 3. 27 as before. |
| 12 | dodecagon | 11.1961524 | |

EXAMPLE 2.—Required by both rules, the area of the regular pentagon, whose side BD is 27 chains.

A. R. P.
Answer 125. 1. 27.

EXAMPLE 3.—What is the area of a regular dodecagon, whose side is 3 chains 49 links.

A. R. P.
Answer 136. 1. 19

PROBLEM X.

To find the circumference of a circle.

Rule.—Multiply the diameter by 3·1416.

Circumferences are to each other as their diameters, for $\pi = 3\cdot1416 =$ the semi-circumference, when radius is unity; $=$ circumference of the circle, when diameter is unity.

EXAMPLE 1.—What is the circumference of a circle, whose diameter is 30 chains?

$$\text{Circumference} = 30 \pi = 3\cdot2426 \times 30 = 94\cdot2480 \text{ chains.}$$

EXAMPLE 2.—Required the circumference of a circle, whose diameter is 17 chains 40 links. *Answer* 54·66 chains.

EXAMPLE 3.—What is the diameter of a circle, whose circumference measures 1,000 chains? *Answer* 318 chains 30 links.

PROBLEM XI.

To find the length of any arc.

As 360° : to the given degrees of the arc :: so is the whole circumference : to the length of the arc required.

EXAMPLE 1.—What is the length of an arc of 20° , of a circle, whose circumference measures 850 chains?

$$\text{As } 360^\circ : 20^\circ :: 850 \text{ chains} : x$$

$$x = \frac{850 \times 20}{360} = 47\cdot22 \quad \text{Ans. } 47\cdot22 \text{ chs.}$$

EXAMPLE 2.—Required the length of the quadrant, the circumference measuring 300 chains. *Answer* 75 chains.

EXAMPLE 3.—What is the circumference of a circle, when the arc of 30° measures 17 chains 20 links? *Answer* 206·40 chains.

PROBLEM XII.

To find the area of a circle.

Rule.—Multiply the square of the diameter by ·7854.

Areas of circles are as the squares of the diameters—

$$\therefore \text{area of circle.} = \text{rad.}^2 \times \pi$$

$$= (2 \text{ rad.})^2 \frac{\pi}{4} = \text{diam.}^2 \frac{\pi}{4}.$$

EXAMPLE 1.—What is the area of a circular field, whose diameter is 18 chains?

$$\begin{array}{r}
 \begin{array}{c} \cdot 7854 \\ 18^2 = \underline{\quad 324} \\ 31416 \\ 15708 \\ 23562 \\ \hline 10 | \underline{254\cdot 4696} \\ 25\cdot 4469 \\ 4 \\ \hline 1\cdot 7876 \\ 40 \\ \hline 31\cdot 5040 \end{array} & \text{A. R. P.} \\
 \end{array}$$

Answer 25. 1. 31.

EXAMPLE 2.—The area of a circular plot of ground is required, whose diameter is 27 chains.

A. R. P.
Answer 2. 0. 19.

EXAMPLE 3.—What is the area of the circle, that can be described by a rope, measuring $5\frac{1}{4}$ chains, having one end fixed, as a centre?

A. R. P.
Answer 1. 2. 37.

PROBLEM XIII.

To find the area of an ellipse.

Rule.—Multiply the product of the major and minor axes by .7854. An ellipse being a mean proportional between a circle described on its minor, and one on its major axis.

Let b = the diameter of the smaller circle ; and a, that of the greater ;

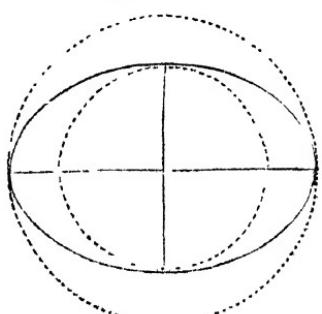
then $\left\{ \begin{array}{l} .7854 \times a^2 = \text{area of greater circle,} \\ .7854 \times b^2 = \text{area of less circle,} \end{array} \right.$

therefore $\sqrt{.7854^2 (a^2 b^2)} = .7854 ab = \text{ellipse.}$

EXAMPLE 1.—Required the area of an ellipse, whose transverse diameter is 12 chains, and conjugate, 8 chains 50 links.

sq. chains.
 $.7854 \times 12 \times 8\cdot 50 = 80.11.$

A. R. P.
Answer 8. 0. 1.



EXAMPLE 2.—What is the area of the ellipse, whose major and minor axes are, respectively, 22 chains 40 links, and 15 chains 50 links?

A. R. P.
Answer 27. 1. 3.

EXAMPLE 3.—Required the area of an ellipse, whose transverse diameter is 36 chains 20 links, and conjugate, 24 chains.

A. R. P.
Answer 68. 0. 37.

PROBLEM XIV.

To find the area of a parabola.

Rule.—Multiply the base by $\frac{2}{3}$ of the perpendicular height. The area of a parabola being equal to $\frac{2}{3}$ of its circumscribing rectangle.

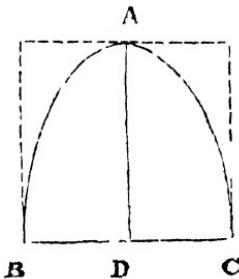
EXAMPLE 1.—Required the area of a parabola, whose base is 22 chains 50 links, and perpendicular height, 22 chains 50 links also.

$$22\cdot50 \times \frac{2}{3} = \frac{45}{3} = 15$$

sq. chains.

$$15 \times 22\cdot50 = 337\cdot50 = \text{area.}$$

A. R. P.
Answer 33. 3. 0.



EXAMPLE 2.—What is the area of a parabola, whose base is 15 chains 25 links, and perpendicular height, 11 chains 30 links.

A. R. P.
Answer 11. 1. 38.

EXAMPLE 3.—The base of a parabola is 12·50 chains, and its perpendicular height, is equal to its base. Required the area.

A. R. P.
Answer 13. 1. 26.

PLANE TRIGONOMETRY.

DEFINITIONS.

1. Angles are measured by the arcs of circles, cut off by the two sides, that compose the angles ; *i.e.*, the angle GCB is measured by the arc GB.

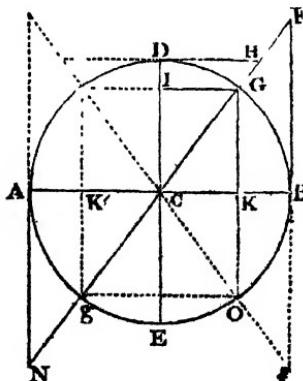
2. The chord of an arc, is a straight line, joining the extremities of an arc, GO being the chord of the arc GBO.

3. The circumference of a circle, is divided by English Mathematicians into 360 equal parts or degrees ; each degree, into 60 minutes ; and each minute, into 60 seconds, which are thus marked : $4^{\circ}, 3', 15''$; *4 degrees, 3 minutes, and 15 seconds.* A quarter of a circle, or a quadrant, contains therefore 90 degrees.

4. The complement of an angle, is the difference between that angle and 90 degrees ; therefore, *each of the acute angles of a right-angled triangle is the complement of the other.*

5. The supplement of an angle, is the difference between that angle and 180 degrees ; or, measured on the circumference, the difference between the arc and the semi-circumference. And, as the sum of the three angles of any triangle is equal to two right angles, or 180 degrees, *each angle of any triangle has for its supplement, the sum of the other two.*

6. *The sine of an arc,* is a perpendicular, drawn from one extremity of the arc, upon the diameter of the circle, which passes through the other extremity, *therefore the sine of any two angles is equal to the sine of the third* ; thus, in the annexed figure, GK, is the sine of the arc BG. Produce GK to O ; GO is bisected ; and the arc GB = the arc BO ; therefore *every sine is equal to half the chord of twice the arc.*



7. *The Cosine of an arc,* is the sine of its complement, or, is that portion of the diameter, subtended between the sine and the centre, and measured from the centre to the sine. Thus, CK is the cosine of the angle GCB.

8. The *versed sine* of an arc; is that portion of the diameter, subtended between the sine, drawn from one extremity of the arc, and the other extremity, and is equal to (radius—cosine of arc,) being either BK or BK'.

9. The *tangent* of a circle, is a line drawn from any point of the circumference, at right angles to the diameter; the tangent of an arc, therefore, is a line drawn at right angles to the diameter *from one extremity*, and intersected by

10. The *secant*, which is a line drawn *from* the centre *through* the other extremity, or, in a *contrary* direction; thus BF is the tangent to the arc BG, and CF is the secant of the same arc.

11. The *cotangents*, *cosecants*, &c., are but the tangents and secants of the complements of the arc;

Thus DH is the cotangent, and CH the cosecant of the arc BG.

And in the same way GK, BF, and CF are respectively the cosine, cotangent, and cosecant of the co-arc DG; and BK, the co-versesine of DG.

12. The supplemental arc ADG has the same sine GK, and equal tangents and secants; but the tangents and secants are of the opposite kind, being measured the other way; if the supplemental arc is greater than 90 degrees, they are negative, and those of the arc positive; if less than 90 degrees, the reverse.

CHAPTER I.

I. EQUIVALENTS.

The several triangles CKG, CBF, and CDH, are equilateral, and therefore similar; hence result, by proportion, the several values of the sines, cosines &c.

$$1. \text{ GK : CG :: CD : CH ; } \quad \text{or } \text{GK} = \frac{\text{CG.CD}}{\text{CH}}$$

$$\text{i.e. } \text{sine} = \frac{\text{rad.}^2}{\text{cosec.}} ; \quad \text{or cosec.} = \frac{\text{radius}^2}{\text{sine}} = \frac{1}{\text{sine}}, \text{ where R} = 1.$$

$$2. \text{ CK : CG :: CB : CF ; } \quad \text{or } \text{CK} = \frac{\text{CG.CB}}{\text{CF}}$$

$$\text{i.e. } \text{cosine} = \frac{\text{rad.}^2}{\text{sec.}} = \frac{1}{\text{sec.}}, \text{ where R} = 1 :$$

$$\text{and sec.} = \frac{\text{rad.}^2}{\text{cosine}} = \frac{1}{\text{cosine}}.$$

$$3. CK : KG :: CB : BF; \quad \text{or } CK = \frac{KG \cdot CB}{BF};$$

$$\text{i. e. } \cosine = \frac{\text{sine rad.}}{\tan.} = \frac{\text{sine}}{\tan.} \quad \text{or } \tan. = \frac{\text{sine}}{\cosine}.$$

$$4. KG : GC :: BF : FC; \quad \text{or } KG = \frac{GC \cdot BF}{FC};$$

$$\text{i. e. } \sin = \frac{\text{rad. tan.}}{\sec.} = \frac{\tan.}{\sec.}.$$

$$5. GK : KC :: DC : DH; \quad \text{or } GK = \frac{HC \cdot DC}{DH}$$

$$\text{i. e. } \sin = \frac{\cosine \text{ rad.}}{\cotan.} = \frac{\cos.}{\cotan.} \quad \text{or } \cot. = \frac{\cos.}{\sin.}.$$

$$6. BF : CB :: CD : DH; \quad \text{or } BF = \frac{CB \cdot CD}{DH}$$

$$\tan. = \frac{\text{rad}^2}{\cot.} = \frac{1}{\cot.}; \text{ where } R = 1:$$

Hence we have these general values.

$$1. \sin = \frac{\text{rad}^2}{\cosec.} = \frac{1}{\cosec.} = \frac{\tan.}{\sec.} = \sqrt{\text{rad}^2 - \cosine^2} = \sqrt{1 - \cosine^2}.$$

$$2. \cosine = \frac{\text{rad}^2}{\sec.} = \frac{1}{\sec.} = \frac{\cot.}{\cosec.} = \sqrt{\text{rad}^2 - \sin^2} = \sqrt{1 - \sin^2}.$$

$$3. \tan. = \frac{\text{rad}^2}{\cot.} = \frac{1}{\cot.} = \frac{\sin.}{\cos.} \times R = \frac{\sin.}{\cos.}.$$

$$4. \cotan. = \frac{\text{rad}^2}{\tan.} = \frac{1}{\tan.} = \frac{\cosine}{\sin.} \times R = \frac{\cosine}{\sin.}.$$

$$5. \sec. = \frac{\text{rad}^2}{\cosine} = \frac{1}{\cos.} = \sqrt{\text{rad}^2 + \tan^2} = \sqrt{1 + \tan^2}.$$

$$6. \cosec. = \frac{\text{rad}^2}{\sin.} = \frac{1}{\sin.} = \sqrt{\text{rad}^2 + \cotan^2} = \sqrt{1 + \cotan^2}$$

$$7. \text{and versed sine} = 1 - \cos.:$$

$$8. \text{The tan. of } 45^\circ = \text{chord of } 60^\circ = \sin 90^\circ = \text{radius:}$$

And *generally*, if, in a right-angled triangle, the *base* be taken as the actual measurement of the radius, the hypotenuse and perpendicular become the actual lengths of the secant and tangent (to the same radius); and, if the *hypotenuse* be taken as radius, the perpendicular and base become the actual lengths of the sine and cosine, to the angle, at the point, considered as the centre of the circle.

* II. VALUES OF THE SINES, COSINES, &c.

In the different quadrants of the circle.

Let A be any arc in the first quadrant, BG; then $\pi - A$ or BDG' , $\pi + A$, or $BDAg$; $2\pi - A$, or $BDAgO$, will be the arcs in the other quadrants.

| In the first quadrant. | In the second quadrant. |
|---|--|
| $GK = \sin. A$ $CK = \cos. A$ $FB = \tan. A$ $CF = \sec. A$ See figure page 36. | $\sin. \pi - A = GK = + \sin. A$ $\cos. \pi - A = CK' = - CK = - \cos. A^*$ $\tan. \pi - A = Bf = - BF = - \tan. A^+$ $\sec. \pi - A = Cf = - CF = - \sec. A$ |

* CK' being measured in an opposite direction to CK is *negative*.

† Bf , being measured opposite to BF , is also *negative*; and Cf being measured, not *through* the other extremity (see def. 10), but, away from it, is *negative* also.

The values of the $\tan. \pi - A$ and $\sec. \pi - A$ may be thus verified:—

$$\tan. \pi - A = \frac{\sin. \pi - A}{\cos. \pi - A} = \frac{\sin. A}{-\cos. A} = -\tan. A.$$

$$\text{and see. } \pi - A = \frac{\text{rad.}^s}{\cos. \pi - A} = \frac{1}{-\cos. A} = -\sec. A.$$

| In the third quadrant. | In the fourth quadrant. |
|---------------------------|--|
| $\sin. \pi + A = K'g =$ | $-\sin. A$ |
| $\cos. \pi + A = CK' =$ | $-\cos. A$ |
| $\tan. \pi + A = BF =$ | $+\tan. A$ |
| $\sec. \pi + A = -(CF) =$ | $-\sec. A$ |
| | $\sin. 2\pi - A = KO = -KG = -\sin. A$ |
| | $\cos. 2\pi - A = CK = +\cos. A$ |
| | $\tan. 2\pi - A = Bf = -BF = -\tan. A$ |
| | $\sec. 2\pi - A = Cf = +\sec. A$ |

Sec. $\pi + A = CF$, but *negative*, for it is not drawn through g , the other extremity, but away from it through G .

Sec. $2\pi - A$ is positive, being measured through the other extremity O .

These values also, may be verified by their equivalents :—

$$\tan. \pi + A = \frac{\sin. \pi + A}{\cos. \pi + A} = \frac{-\sin. A}{-\cos. A} = \tan. A;$$

$$\sec. \pi + A = \frac{1}{\cos. \pi + A} = \frac{1}{\cos. A} = \sec. A.$$

$$\tan. 2\pi - A = \frac{\sin. 2\pi - A}{\cos. 2\pi - A} = \frac{-\sin. A}{\cos. A} = -\tan. A;$$

$$\sec. 2\pi - A = \frac{1}{\cos. (2\pi - A)} = \frac{1}{\cos. A} = \sec. A.$$

III. NUMERICAL VALUES,

Of the sines, cosines, &c., of 30° , 45° , and 60° .

Chord of 60° = radius = 1 ;

$$\sin. 30^\circ = \frac{\text{chord } 60^\circ}{2} = \frac{1}{2} = .5$$

$$\cos. 30^\circ = \sqrt{1 - \sin.^2 30^\circ} = \sqrt{1 - \frac{1}{4}} = \sqrt{\frac{3}{4}} = \frac{1}{2}\sqrt{3}$$

$$\tan. 30^\circ = \frac{\sin. 30^\circ}{\cos. 30^\circ} = \frac{1}{2} \times \frac{2}{\sqrt{3}} = \frac{\sqrt{3}}{3}$$

$$\sec. 30^\circ = \frac{1}{\cos. 30^\circ} = 1 \times \frac{2}{\sqrt{3}} = \frac{2}{\sqrt{3}} = \frac{2}{3}\sqrt{3}$$

$$\begin{aligned} \sin. 45^\circ &= \cos. 90^\circ - 45^\circ = \cos. 45^\circ \\ \text{rad.} &= 1 = \sqrt{\sin.^2 45 \times \cos.^2 45} = \sqrt{2 \sin.^2 45} = 2 \sin. 45 \\ \therefore \sin. 45^\circ &= \sqrt{\frac{1}{2}} = \frac{1}{2}\sqrt{2} \end{aligned}$$

$$\cos. 45^\circ = \sin. 45^\circ = \frac{1}{2}\sqrt{2}$$

$$\tan. 45^\circ = \frac{\sin. 45^\circ}{\cos. 45^\circ} = \frac{1}{1} = 1.$$

$$\sec. 45^\circ = \frac{1}{\cos. 45^\circ} = 1 \times \frac{\sqrt{2}}{1} = \sqrt{2}$$

$$\sin. 60^\circ = \cos. 30^\circ = \frac{1}{2}\sqrt{3}$$

$$\cos. 60^\circ = \sin. 30^\circ = \frac{1}{2} = .5$$

$$\tan. 60^\circ = \frac{\sin. 60^\circ}{\cos. 60^\circ} = \frac{1}{2}\sqrt{3} \times \frac{1}{1} = \sqrt{3}$$

$$\sec. 60^\circ = \frac{1}{\cos. 60^\circ} = 1 \times \frac{2}{1} = 2$$

IV. RELATIVE VALUES

Of the sines, cosines, &c., to different radii.

As the actual lengths of the sines, cosines, &c., of different angles, and to different radii, vary in a ratio, compounded of the ratio of their subtending angles and their given radii, so, when the *angles* are constant, they vary as their radii; and when the *radii* are constant, as their angles.

Let sine a = sine of any angle to radius (unity); and sine A = sine of the same angle, to radius (R); then sine a : sine A :: 1 : R;

$$\therefore \frac{\text{sine } a}{1} = \frac{\text{sine } A}{R}.$$

whence, to transform formulas, calculated to radius unity—to others, to radius R; wherever sine a , tan. a , &c., occur, their equivalents, $\frac{\text{sine } A}{R}$, $\frac{\tan. A}{R}$, &c., must be substituted in their stead.

EXAMPLE 1.—To find the value of the tan. A (to radius R,) in terms of the sine and cosine.

$$\tan. a = \frac{\sin. a}{\cos. a}$$

$$\frac{\tan. A}{R} = \frac{\sin. A}{R} \cdot \frac{R}{\cos. A} = \frac{\sin. A}{\cos. A}.$$

$\therefore \tan. A = \frac{\sin. A}{\cos. A} \times R$, which also verifies the following proposition, that

$$\text{as sin. } a = \frac{\sin. A}{R}$$

$$\therefore \sin. a \times R = \sin. A,$$

or, that the actual value of the sine, cosine, &c., of any angle A, whose radius is any length, R, is equal to that of the sine, cosine, &c., of the same angle, *calculated to unity*, multiplied by the given radius, R.

EXAMPLE 2.—What is the numerical value of the tan. 45° to radius 500?

$$\tan. 45^\circ \text{ to rad. 1} = 1.$$

$$\tan. 45^\circ \text{ to rad. } R = \tan. A = \tan. a \times R = 1 \times 500 = 500.$$

EXAMPLE 3.—What is the value of the sec. 30° to radius 300?

$$\sec. a = \frac{1}{\cos. a} = \frac{1}{\cos. 30} = 1 \times 2\sqrt{3} = 3\sqrt{3}$$

$$\sec. 30^\circ \text{ to rad. } 300 = \sec. A = \sec. a \times R = 3\sqrt{3} \times 300 = 200\sqrt{3}$$

CHAP. II.

and for
are called **RIGHT-ANGLED TRIANGLES.**

CASE. I.—When one acute angle and one side are given.

There will be no difficulty, if the following be borne in mind, viz.:—

1. That, if the *hypotenuse* be made *radius*, each leg will represent the sine of its opposite angle to that radius.

2. And if either the *base* or *perpendicular* be made *radius*, the hypotenuse and other side will represent the secant and tangent of the angle at the centre, and *that the sides are proportional to the lines they represent.*

Hence (as in the first case) as radius : to the hypotenuse :: sine of either angle : to its opposite side.

And (in the second case) 1st, as radius : is to the *perpendicular* :: so is tangent of the vertical angle : to the base ; and also : so is secant of the vertical angle : the hypotenuse.

And 2nd, as radius : is to the base :: so is tangent of the angle at base : to the perpendicular ; and : so is secant of the same angle : to the hypotenuse.

EXAMPLE 1.—Let the hypotenuse of a right-angled triangle be 3 chains 50 links, and the angle at the base 25 degrees 15 minutes. Required the perpendicular and the base.

$$\begin{array}{ll}
 \text{As radius} & = 10\cdot000000 \\
 \text{to log. hypotenuse } 3\cdot50 & = 0\cdot544068 \\
 \text{so is log. sine } 25^\circ 15' & = 9\cdot629989 \\
 \text{to log. perpendicular } 1\cdot49 & = 0\cdot174057
 \end{array}$$

added
after a

$$\begin{array}{ll}
 \text{and as radius} & = 10\cdot000000 \\
 \text{to log. hypotenuse } 3\cdot50 & = 0\cdot544068 \\
 \text{so is log. cosine } 25^\circ 15' & = 9\cdot956387 \\
 \text{to log. base } 3\cdot16 & = 0\cdot500455
 \end{array}$$

Ans. Perpendicular = 1·49 chains

Base = 3·16 chains.

Ex. 2.—Given the vertical angle $35^\circ 54'$, and the perpendicular 12·20 chains. Required the base and hypotenuse.

Ans. Base = 8·83 chs.; Hyp. = 15·06 chs.

Ex. 3.—What are the hypotenuse and perpendicular of a right-angled triangle, whose base is 25 chains 80 links, and the vertical angle $89^\circ 27'$?

Ans. Perp. = 24 links; Hyp. = 25·80 chs.

CASE II.—When two sides are given.

As in all right-angled triangles, the square of the hypotenuse is equal to the sum of the squares of the base and perpendicular, if any two sides are given, the third can be found by one of the following formulas :—

$$\text{Hypotenuse} = \sqrt{\text{perp.}^2 + \text{base}^2}$$

$$\text{Perpendicular} = \sqrt{\text{hyp.}^2 - \text{base}^2} = \sqrt{(\text{H} + \text{B})(\text{H} - \text{B})}$$

$$= \frac{\log. \text{H} + \text{B}}{2} + \frac{\log. \text{H} - \text{B}}{2}$$

$$\text{Base} = \sqrt{\text{hyp.}^2 - \text{base}^2} = \sqrt{(\text{H} + \text{P})(\text{H} - \text{P})}$$

$$= \frac{\log. \text{H} + \text{P}}{2} + \frac{\log. \text{H} - \text{P}}{2}$$

In the case, however, of the base and perpendicular being given, it is sometimes useful to obtain the hypotenuse *trigonometrically*, thus :

As perp. :: rad. :: base : to tang. angle at vertex ;
then, as sine angle at vertex : base :: radius : hypotenuse.

EXAMPLE 1.—What is the hypotenuse, when the perpendicular is 2·50 chains, and the base 3·90 chains ?

$$\begin{aligned}\text{First hyp.} &= \sqrt{2\cdot50^2 + 3\cdot90^2} = \sqrt{21\cdot4600} = 4\cdot63 \text{ chains.} \\ \text{secondly, by the above formula,} \\ \text{by saying, as log. perpendicular } 2\cdot50 &= 0\cdot397940 \\ \text{is to log. radius} &= 10\cdot000000 \\ \text{so is log. base } 3\cdot90 &= 0\cdot591065 \\ \text{to tang. angle at vertex } 57\cdot20 &= 10\cdot193125\end{aligned}$$

and then,

$$\begin{aligned}\text{as log. sine angle at vertex } 57\cdot20 &= 9\cdot925222 \\ \text{log. base } 3\cdot90 &= 0\cdot591065 \\ \text{so is radius} &= 10\cdot000000 \\ \text{to log. hyp. } 4\cdot63 &= 0\cdot665843\end{aligned}$$

we obtain the hypotenuse 4·63, the same length as in the preceding.

The angles can be obtained from the first method, by the following proportion, viz. :—

$$\begin{aligned}\text{As log. hyp. } 4\cdot63 \text{ chains} &= 0\cdot665581 \\ :\text{log. rad.} &= 10\cdot000000 \\ :\text{log. perpend. } 2\cdot50 &= 0\cdot397940 \\ :\text{log. sine angle at base } 32^\circ 41' &= 9\cdot732359 \\ \text{and :log. cos. angle at vertex } 57^\circ 19'; &\quad "\end{aligned}$$

EXAMPLE 2.—Let AC equal 45 chains, BC 36 chains, and the angle A, $51^{\circ} 25'$; required the base AB.

$$\begin{array}{rcl} \text{As log. BC, 36 chains} & = & 1.556303 \\ \text{to log. sin. } \angle A, 51^{\circ} 25' & = & 9.899041 \\ \text{so is log. AC, 45 chains} & = & 1.653213 \\ & & \hline & & 11.546254 \\ & & & & 1.556303 \end{array}$$

$$\begin{array}{rcl} \text{to log. sin. ABC, } 77^{\circ} 43' & = & 9.989951 \\ \text{or to its supplement, } 102^{\circ} 17'. & & \end{array}$$

The angle ABC has therefore two values, depending upon whether it is greater or less than a right angle; and also the angle ACB, and its opposite side AB, which may be either AB or AD.

EXAMPLE 3.—The two sides of a triangle are AC, 50 chains; CB, 25 chains; and the angle ACB, 16 degrees 45 minutes; what are the other sides and angles? *Answer* Base 27.07 chs. $\angle B = 147^{\circ} 49'$
~~120° 11'~~ $\angle A = 15.26$.

CASE II.—When the two sides and included angle are given.

Rule.—Subtract the given angle from 180° , which will give the sum of the other two; then, x and y being any two angles,

$$\begin{aligned} x &= \frac{1}{2}(x+y) + \frac{1}{2}(x-y), \\ \text{and } y &= \frac{1}{2}(x+y) - \frac{1}{2}(x-y). \end{aligned}$$

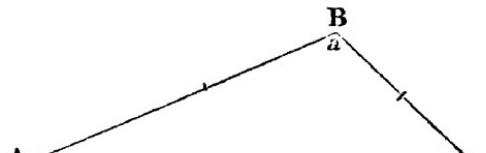
Then, to obtain $(x-y)$; say, as the sum of the two sides : is to their difference :: so is the tan. $\frac{1}{2}$ sum of the two unknown angles, $(x+y)$: to the tan. $\frac{1}{2}$ difference $= (x-y)$.

By substituting the values of $x+y$, and $x-y$, which are now both known, in the above formula, the values of x and y will be obtained.

In the triangle ABC, let the two sides, AB and BC (AB being greater than BC) be given, and the angle, at B $= a^{\circ}$

then $180^{\circ}-a^{\circ}$ = sum of angles at A and C; and $(x-y)$ = difference.

$$\begin{array}{l} \angle ACB = \frac{180-a}{2} + \frac{x-y}{2} \\ \text{and } \angle BAC = \frac{180-a}{2} - \frac{x-y}{2} \end{array}$$



It is now reduced to the condition of Case 1st; proceed, therefore, according to the rules there given.

The following formula may sometimes be useful, in at once obtaining the base.

Find first the value of $(x-y)$ as above, then say—

$$\text{As } \cos. \frac{1}{2}(x-y) : \cos. \frac{1}{2}(x+y) :: AB+BC : AC$$

or,

$$\text{As } \sin. \frac{1}{2}(x-y) : \sin. \frac{1}{2}(x+y) :: AB-BC : AC$$

$$AC = \frac{\cos. \frac{1}{2}(x+y)}{\cos. \frac{1}{2}(x-y)} \cdot (AB+BC).$$

$$\text{or } AC = \frac{\sin. \frac{1}{2}(x+y)}{\sin. \frac{1}{2}(x-y)} (AB-BC)$$

that is,

| | | |
|---|----|---|
| as cos. $\frac{1}{2}$ (difference of angles), | or | as sine $\frac{1}{2}$ (difference of angles), |
| : cos. $\frac{1}{2}$ (sum), | | : sine $\frac{1}{2}$ (their sum), |
| : sum of the two sides, | | : difference of the two sides, |
| : to the base, | | : to the base. |

The advantage of using this formula is, that the logarithmic values of the cosines of $(x+y)$ and $(x-y)$ can be obtained at the same time as those of their tangents, which are wanted, in the first formula, to obtain the value of $x-y$; and the separate values of the angles are not required.

EXAMPLE 1.—Let the two sides AB, BC of the triangle ABC be 36·40 and 24·40 chs., and the included angle 16 degrees 26 minutes. Required the other side and the remaining angles.

$$\begin{array}{r} 180^{\circ}00' \\ 16\cdot26 \\ \hline 2 | 163\cdot34 \\ 81\cdot47 = \frac{x+y}{2} \end{array} \qquad \begin{array}{r} 36\cdot40 \\ 24\cdot40 \\ \hline 60\cdot80 = AB + BC \\ 12\cdot00 = AB - BC \end{array}$$

As log. AB + BC, $60^{\circ}80' = 1\cdot783904$
is to log. AB - BC, $12^{\circ}00' = 1\cdot079181$

so is log. tan. $\frac{x+y}{2}$, $81^{\circ}47' = 10\cdot840435$

11·919616

to log. tan. $\frac{x-y}{2}$ $53^{\circ}49' = \underline{10\cdot135712}$

81·47

53·49

135·36 = \angle ACB

27·58 = \angle BAC

Add \angle ABC = 16·26

180·00 sum of the three angles.

The question being now reduced to the first case, the base may be obtained, as follows, by saying:—

| | |
|--|------------------|
| As log. sine \angle BAC, $27^\circ 58'$ | = 9.671134 |
| is to log. BC, 24.40 | = 1.387390 |
| so is log. sine \angle ABC, $16^\circ 26'$ | = 9.451632 |
| | <u>10.839022</u> |
| | <u>9.671134</u> |
| to log. AC - 14.72 | = 1.167888 |

or, otherwise, by using the formula, above referred to, of the cosines of the sum and difference of the unknown angles, and saying :—

| | | |
|--|---|-----------|
| As log. cosine $\frac{1}{2} \overline{x-y}$, 53°49' | = | 9.771125 |
| : to log. cosine $\frac{1}{2} x+y$, 81°47' | = | 9.155083 |
| :: is log. (AB + BC ₁) 60°80' | = | 1.783904 |
| | | 10.938987 |
| | | 9.771125 |
| to log. AC, 14°72' | = | 1.167862 |

differing only in the two last places of decimals, which do not agree with the preceding, in consequence of the cos. $\frac{x-y}{2}$ ($53^{\circ}49.$) not being the exact value of the logarithm, 10.185712 ; there is no difference, however, in their values, as to links, which is sufficiently near for practical purposes.

For finding the exact values of logarithmic sines, cosines, &c., and carrying the whole work out to seconds, see chapter on the "Theodolite and Trigonometrical Calculations."

EXAMPLE 2.—Given the two sides 45 chains 44 links, and 40 chains 48 links, of any triangle ABC, and the included angle 117 degrees 50 minutes; what are the two other angles and the remaining side?

Ans. Base = 158.94 chs. \angle at B = $82^{\circ} 47' 30''$. 33
 \angle at C = $29.22^{\circ} 30' 47.99''$.

EXAMPLE 3.—The included angle of a triangle is 45 degrees; the two sides are respectively 70 chains and 50 chains; what will be the length of the base? *Ans.* 49·50 chs.

Ans. 49·50 chs.

CASE III.—*When the three sides only are given.*

Let fall a perpendicular from the vertex upon the base, dividing the triangle into two right-angled triangles. We shall then have one side, and the right-angle known in each. And, by finding the lengths of the

segments of the base of the whole triangle, we obtain in each of the smaller right-angled triangles two given sides and an angle. Now, by saying,—

As the whole base, or sum of the segments
 : is to the sum of the two other sides,
 :: so is the difference of those two sides,
 : to the difference of the segments of the base.

And, by adding half the sum to half the difference, we obtain the *greater* segment, which is always adjoining the *greater* side of the given triangle, and, by subtracting half the difference from half the sum, we obtain the *smaller* segment, adjoining the *smaller* side of the whole triangle.

The case having come under the form of two right-angled triangles, the required sides and angles must be worked out by the rules before given.

Let ABC be a triangle, whose three sides AB, BC, and AC, are given.

From B, let fall BD perpendicular to AC. In the two triangles ABD, CBD, AB and BC being known, and the angles at D right angles, we have to find first the two sides AD and DC; say,—

As $AC : AB + BC :: BC - AB : x$
 when $x = AD \sim DC$,
 and because BC is greater than AB,
 therefore DC is greater than AD;

then, by saying, (*see case of right-angled triangles*),—

as $AB : \text{rad.} :: AD : \cos. \angle \text{ at A}$

we obtain the angle at A; and, by similar proportions, the remaining angles at C and B.—*The angles ABD, CBD, calculated from the smaller triangles, should be together equal to the supplement of the sum of the angles at A and C.*

EXAMPLE 1.—Let ABC be a triangle, having the sides AB, BC, and AC, respectively, equal to 25 chains, 36 chains, and 48 chains. It is required to find the angles.

Let fall the perpendicular BD, which will divide the given triangle into two smaller right-angled triangles.

Then, to find the difference of the segments of the base, say,—

$$\text{As } AC : BC + AB :: BC - AB : (AD \sim DC)$$

$$\text{or, } 48 : 61 :: 11 : 13.98 -$$

and adding half the sum to half the difference, for the greater segment, and subtracting them, for the less, we have,—

$$\frac{1}{2} \text{ of } 48 = 24; \text{ and } \frac{1}{2} \text{ of } 13.98 = 6.99.$$

where $24 + 6.99 = 30.99 =$ greater segment, DC,

and $24 - 6.99 = 17.01 =$ smaller segment, AD,

It is now reduced to the case of right-angled triangles; next say,—

$$\text{As log. AB, } 25, \quad = 1.39794$$

$$\text{is to log. rad.} \quad = 10.00000$$

$$\text{so is log. AD, } 17.01, \quad = \underline{\underline{1.23070}}$$

$$11.23070$$

$$1.39794$$

$$\text{to log. cos. angle A, } 47^\circ 8', \quad = 9.83276$$

$$\text{And as log. BC, } 36, \quad = 1.55630$$

$$\text{to log. rad.} \quad = 10.00000$$

$$\text{is log. DC, } 30.99, \quad = \underline{\underline{1.49122}}$$

$$11.49122$$

$$1.55630$$

$$\text{to log. cos. angle C, } 30^\circ 35', \quad = 9.93492$$

Adding the angle at A and C, and deducting them from 180° , we obtain the angle at B.

$$47^\circ 8' + 30^\circ 35' = 77^\circ 43'; \text{ and}$$

$$180^\circ - 77^\circ 43' = 102^\circ 17' = \text{angle B.}$$

EXAMPLE 2.—Given a triangle, whose sides are 25 chains 16 links, 24 chains 13 links, and 16 chains 17 links, respectively; required the angles.
Ans. $74^\circ 20'$; $67^\circ 25'$; $38^\circ 15'$.

EXAMPLE 3.—A triangle, whose sides are respectively, 40, 50, and 60 chains, being given, it is required to find the angles.

$$\text{Ans. } 82^\circ 49'; 41^\circ 25'; 55^\circ 46'.$$

EXAMPLE 4.—What are the angles of that triangle, whose sides are 498 chains 16 links, 464 chains, and 40 chains, respectively?

$$\text{Ans. } 125^\circ 29'; 52^\circ 11'; 2^\circ 20'.$$

2ND METHOD.—When the nature of the question merely requires the determining of one of the angles, the following formula may be used, viz.:—

$$\cos. \angle C = \frac{AC^2 + BC^2 - AB^2}{2 AC \cdot CB} \text{ or, (vide fig. p. 49) making } AB = c$$

$BC = a$, and $AC = b$, corresponding to the angles to which they are opposite. $\cos. C = \frac{a^2 + b^2 - c^2}{2 ab}$

For $AB^2 = AC^2 + BC^2 \pm 2 AC \cdot CD$ (see Theor. 7, page 13.)

$$\therefore \pm CD = \frac{AC^2 + BC^2 - AB^2}{2 AC}$$

now $BC : CD :: \text{rad.} : \cos. C$

$$\therefore \pm \frac{CD}{BC} = \pm \frac{\cos C}{R} \text{ and } \pm CD = BC \cos. C \text{ (when radius } = 1)$$

$$\text{(by substituting)} BC \cos. C = \frac{AC^2 + BC^2 - AB^2}{2 AC}$$

$$\therefore \cos. C = \frac{AC^2 + BC^2 - AB^2}{2 AC \cdot CB} = \frac{a^2 + b^2 - c^2}{2 ab};$$

Where $\cos. C$ is that of an angle, either greater or less than a right angle, whose value is expressed in terms of its sides; a and b being the sides that include it, and c the side opposite to it.

This formula, however, can only be used in its *present* form, when the sides are *small*, and when they can be calculated arithmetically, as in the case of the 1st Example, in the *first method*, the result of which we will now proceed to verify, by the present formula.

The given sides are 25, 36, and 48 chains.

Substituting these lengths in the above formula, we have,—

$$25^2 = 625; 36^2 = 1296; \text{ and } 48^2 = 2304$$

$$\text{then } \cos. C = \frac{625 + 2304 - 1296}{2(1200)} = \frac{1633}{2400}$$

$$\frac{1633}{2400} = 0.6804166 = \text{natural cos. } C$$

natural cos. = cos. to rad. (1) = cos. α .

log. cos. = log. cosine to rad. 10^{10} = cos. A.

cos. A = cos. $\alpha \times R$ (see page 41)

$\therefore \log. \cos. A = \log. \cos. \alpha + \log. 10^{10} (10)$

by substituting natural cos. $a = 0.6804166$

$$\log. \cos. a = -1.832781$$

$$+ \log. R = \underline{10}$$

$47^\circ 8' = \log. \cos. A = -1.832781$, the same result as before.

To find the angle C in the same triangle.

$$\cos. C = \frac{AC^2 + BC^2 - AB^2}{2 AC \cdot BC}$$

$$\text{or } \cos. C = \frac{2304 + 1296 - 625}{2(1728)} = \frac{2975}{3456} = .85999$$

$$\log. .85999 = -1.93492$$

$$+ \log. R = \underline{10}$$

$$30^\circ 35' = \log. \cos. C = \underline{.93492}$$

To find the angle at B.

$$\cos. B = \frac{AB^2 + BC^2 - AC^2}{2 AB \cdot BC}$$

$$\text{or } \cos. B = \frac{625 + 1296 - 2304}{2(900)} = \frac{-383}{1800} = -.21278$$

$$\log. -.21278 = -1.32793$$

$$+ \log. \text{rad.} = \underline{10}$$

$$\log. -\cos. 77^\circ 43' = \underline{.932793}$$

but this is the supplemental angle, being the value of $-\cos. B$;

therefore $180^\circ - 77^\circ 43' = 102^\circ 17' = \cos. \text{angle B.}$

It will be seen that these angles are the same as those which were obtained by the first method, where the same triangle was used.

WHEN THE SIDES, HOWEVER, ARE LARGE, it is necessary to convert this formula into a more convenient expression, for logarithmic calculation. viz.—

$$\text{Because, } \cos. C = \frac{a^2 + b^2 - c^2}{2 ab} \text{ (to rad. 1)}$$

$$\frac{\cos. C}{R} = \frac{a^2 + b^2 - c^2}{2 ab} \text{ to rad. R (see page 51)}$$

$$\therefore \cos. C = R \frac{(a^2 + b^2 - c^2)}{2 ab}$$

and versed sine \equiv rad. $-\cos.$

$$\text{vers. sin. C} = R - R \frac{(a^2 + b^2 - c^2)}{2 ab} = R \left(1 - \frac{a^2 + b^2 - c^2}{2 ab}\right);$$

$$= R \frac{(2 ab - a^2 - b^2 + c^2)}{2 ab}$$

$$\text{but } \sin. \frac{1}{2} C = \frac{1}{2} \text{ rad. (vers. sine C)}$$

$$\therefore \sin. \frac{1}{2} C = \frac{\text{rad.} (2 ab - a^2 - b^2 + c^2)}{4 ab}$$

$$\begin{aligned}
 &= \text{rad.}^2 \frac{(c^2 - (a - b)^2)}{4 ab} \\
 &= \text{rad.}^2 \frac{(c + a - b) \cdot (c - a + b)}{4 ab} \\
 \text{sine } \frac{1}{2} C &= \text{rad.} \sqrt{\frac{(s - a) \cdot (s - b)}{ab}};
 \end{aligned}$$

where $s =$ half the perimeter—

and for the $\cos. \frac{1}{2} C$, it may be proved in the same way, that

$$\cos. \frac{C}{2} = \text{rad.} \sqrt{\frac{(s - c) s}{ab}}$$

Assuming the same data as before, for the purpose of verifying the former results, and substituting their values, respectively, in the present formula of the values of half the sines and cosines, we obtain,—

$$\text{where } s = \frac{1}{2} \text{ perimeter} = \frac{25 + 36 + 48}{2} = 54.50$$

$$\text{then } \text{sine } \frac{1}{2} A = \text{rad.} \sqrt{\frac{(54.50 - 25) \cdot (54.50 - 48)}{25 \times 48}}$$

$$\log. 29.50 = 1.46982$$

$$\log. 6.50 = 0.81291$$

$$-\log. 25 = 1.39794 \quad 2.28273$$

$$-\log. 48 = 1.68124 \quad -3.07918$$

$$2 | \underline{-1.20355} \\ \underline{-1.60178}$$

$$+ \log. \text{rad.} = 10.00000$$

$$\log. \text{sine } \frac{1}{2} A = 23^\circ 34' = \frac{9.60178}{2}$$

angle $A = 47^\circ 8'$, the same as before.

By the 2nd formula—that of $\cos. \frac{1}{2} A = \text{rad.} \sqrt{\frac{(s-a)s}{bc}}$, where A is the given angle required, and a is the side opposite to it.

$$\log. 18.50 = 1.26717$$

$$\log. 54.50 = \frac{1.73640}{8.00357}$$

$$-\log. 25 = 1.39794$$

$$-\log. 48 = \frac{1.68124 \quad -3.07918}{2 | \underline{-1.92439}}$$

$$\underline{-1.96219}$$

$$\underline{10.00000}$$

$$\log. \cos. \frac{1}{2} A = 23^\circ 34' = \frac{9.96219}{2}$$

angle $A = 47^\circ 8'$ as before.

A few Examples, without answers, are subjoined, which the student is required to calculate and verify, by the various methods above.

EXAMPLE 1.—Given the three sides 40 chains, 90 chains and 60 chains, to find the opposite angles.

EXAMPLE 2.—The three sides of a triangle are 24·50 chains, 31·60 chains, and 31·96 chains, what are the angles?

EXAMPLE 3.—The sides of a triangular field are, respectively, 20 chains, 3·50 chains, and 45·20 chains. What are the opposite angles?

As a knowledge of some of the general relative values of sines, cosines, and tangents, in terms of each other, may be found exceedingly useful in practice to the Surveyor, some of the most useful ones are subjoined.

The most important formula, the basis, in fact, of the whole, is—

$$\text{sine } (a + b) = \text{sine } a. \cos. b + \cos. a \text{ sine } b$$

$$\text{sine } (a - b) = \text{sine } a. \cos. b - \cos. a \text{ sine } b$$

$$\cos. (a + b) = \cos. a. \cos. b - \text{sine } a \text{ sine } b$$

$$\cos. (a - b) = \cos. a. \cos. b + \text{sine } a \text{ sine } b$$

These are the values of the sines and cosines of the sum and difference of two arcs, in terms of the simple arcs.

By adding and subtracting, we obtain,—

$$\text{sine } (a + b) + \text{sine } (a - b) = 2 \text{ sine } a \cos. b$$

$$\text{sine } (a + b) - \text{sine } (a - b) = 2 \cos. a \text{ sine } b$$

$$\cos. (a + b) + \cos. (a - b) = 2 \cos. a \cos. b$$

$$\cos. (a - b) - \cos. (a + b) = 2 \text{ sine } a \text{ sine } b$$

which are the values of the sum and difference of the sines and cosines of the sum, and difference of the arcs.

Again, by making $a + b = m$

$$\text{and } a - b = p.$$

we have—

$$\text{sine } m + \text{sine } p = 2 \text{ sine } \frac{1}{2}(m + p) \cos. \frac{1}{2}(m - p)$$

$$\text{sine } m - \text{sine } p = 2 \cos. \frac{1}{2}(m + p) \text{ sine } \frac{1}{2}(m - p)$$

$$\cos. m + \cos. p = 2 \cos. \frac{1}{2}(m + p) \cos. \frac{1}{2}(m - p)$$

$$\cos. p - \cos. m = 2 \text{ sine } \frac{1}{2}(m + p) \text{ sine } \frac{1}{2}(m - p)$$

but m and p are *any angles whatever*, so that

$$\text{sine } a + \text{sine } b = 2 \text{ sine } \frac{1}{2}(a + b) \cos. \frac{1}{2}(a - b)$$

$$\text{sine } a - \text{sine } b = 2 \cos. \frac{1}{2}(a + b) \text{ sine } \frac{1}{2}(a - b)$$

$$\cos. a + \cos. b = 2 \cos. \frac{1}{2}(a + b) \cos. \frac{1}{2}(a - b)$$

$$\cos. b - \cos. a = 2 \text{ sine } \frac{1}{2}(a + b) \text{ sine } \frac{1}{2}(a - b)$$

This formula is useful in facilitating the transformation of the product of the sines of the sums and differences of angles into simple angles.

Now, by dividing the last class of formula, the one by the other, we have,—

$$\begin{aligned}\frac{\sin a + \sin b}{\sin a - \sin b} &= \frac{\sin \frac{1}{2}(a+b)\cos \frac{1}{2}(a-b)}{\cos \frac{1}{2}(a+b)\sin \frac{1}{2}(a-b)} = \frac{\tan \frac{1}{2}(a+b)}{\tan \frac{1}{2}(a-b)} \\ \frac{\sin a + \sin b}{\cos a + \cos b} &= \tan \frac{1}{2}(a+b) \\ \frac{\sin a + \sin b}{\cos b - \cos a} &= \cot \frac{1}{2}(a-b) \\ \frac{\sin a - \sin b}{\cos a + \cos b} &= \tan \frac{1}{2}(a-b) \\ \frac{\sin a - \sin b}{\cos b - \cos a} &= \cot \frac{1}{2}(a+b) \\ \frac{\cos a + \cos b}{\cos b - \cos a} &= \frac{\cot \frac{1}{2}(a-b)}{\tan \frac{1}{2}(a-b)}\end{aligned}$$

And by making $b = 0$, we obtain the value of the $\tan \frac{1}{2}a$; $\cot \frac{1}{2}a$, $\tan^2 \frac{1}{2}a$, $\cot^2 \frac{1}{2}a$.

Again,—

$$\text{As } \tan \frac{a+b}{2} = \frac{\sin(a+b)}{\cos(a+b)} = \frac{\sin a \cdot \cos b + \cos a \cdot \sin b}{\cos a \cdot \cos b - \sin a \cdot \sin b}$$

Dividing by $\cos a \cdot \cos b$, we have,—

$$\frac{\frac{\sin a}{\cos b} + \frac{\sin b}{\cos b}}{1 - \frac{\sin a \cdot \cos b}{\cos a \cdot \cos b}} = \frac{\tan a + \tan b}{1 - \tan a \cdot \tan b}$$

$$\text{so also, } \tan \frac{(a-b)}{2} = \frac{\tan a - \tan b}{1 + \tan a \cdot \tan b}$$

Make $b = a$ (in the first value)

$$\text{and } \tan 2a = \frac{2 \tan a}{1 - \tan^2 a} \text{ &c., &c.}$$

[Note.—For a fuller investigation of this subject, see Hall's Trigonometry, which I would strongly recommend to every practical man, who might have leisure to follow it up.]

LAND SURVEYING.

Part the First.

CHAP. I.

ON THE CHAIN.

As every measurement, whether of work, solidity, or superficial extent, must be measured by some unit, to which it can bear constant relation, it has been concluded in the case of land measurement, to make that unit an acre; the acre, or arpent, is the generally recognised unit of land measurement; it varies, however, considerably in different Counties.*

THE STATUTE ACRE in England consists of ten square chains, or one chain front by 10 chains deep, or of its equivalent rectangle ax , where a can be any number, and $x = \frac{10}{a}$ square chains; each chain containing 22 yards, or 4 poles, or 66 feet, or 100 links, and each link 792 inches.

The acre, therefore, is equal to 10 square chains

$$\text{or } (4)^2 \text{ poles} \times 10 = 160 \text{ square poles.}$$

$$\text{or } (22)^2 \text{ yds.} \times 10 = 4840 \text{ square yards.}$$

$$\text{or } (66)^2 \text{ feet} \times 10 = 43560 \text{ square feet.}$$

$$\text{or } (100)^2 \text{ links} \times 10 = 100,000 \text{ square links.}$$

* See the different values of the acre in different Counties of England, in the subject, at the end of the book.

The chain, in common use, is called Gunter's chain, from its inventor, and is divided into ten equal parts, distinguished by a piece of brass, with notches; the brass at the first division, from either end, having one notch; at the second division, two notches; at the third, three; at the fourth, four; and at 50 links, or the middle of the chain, there is a round piece of brass.

The object of marking these divisions from either end of the chain, is to enable the surveyor to measure *either way* from each end; but it is productive sometimes of serious mistakes, as in consequence of 60 links being marked the same as 40, when the eye is not accustomed to distinguish the distance between them, the one is frequently mistaken for the other; and, in measuring with the chain, no mistake is so common as this to a young beginner; and I would particularly charge him to be careful, that in taking off a distance of, what he may suppose to be, 2 chains 45 links, he is not mistaking for it 2 chains 65 links; there is less likelihood of mistaking 30 links for 70, though a stupid inattentive assistant will sometimes even make that mistake.

Each of the above brass divisions of the chain is again sub-divided into other ten parts, or links. If therefore a chain = unity;

$$\text{each brass division} = \frac{1}{10}, \text{ or } 0\cdot 1 \text{ chains};$$

$$\text{and each link} = \frac{1}{100}, \text{ or } 0\cdot 01 \text{ chains.}$$

The advantages of this arrangement is, that, in measuring a line, it matters not whether the distance be termed 7·32 chs. (*7 chains 32 links,*) or 732 links; and, as 10 square chains make one acre, or 10 times 100 links squared, it is only necessary to multiply together the length and breadth, given in links, of a piece of ground, whose area is required,

and set off four decimal places, when the integers will be square chains; or set off five decimal places, and the integers will be the required acres.

Note.—[In doing the chain up, after the day's work, begin always at the middle, folding two links up at once. When done up in this way, by taking hold of the two handles, and throwing the other part from you, you are safe from any entanglement of the chain, when you want to use it.]

To measure a straight line with a chain.

In measuring with the chain, it is requisite to have ten small arrows, or pins of strong iron wire, about a foot and a half long, and pointed at one end, to stick into the ground.

A piece of red cloth should also be tied to the ring of each arrow, so that they may be more easily perceived in the midst of grass or underwood.

Having determined upon the starting point and the direction of the line, it is always requisite to have the line carefully ranged, and generally one or two poles or flags set between the two end flags; these flags should consist of a straight pole about an inch or $\frac{3}{4}$ inch diameter, pointed and shoed at the bottom, and having a white and red flag, about nine inches square at the top, they are generally about six feet high, though in the taking of long base lines, they are occasionally made of from 10 to 15 feet.

On a large survey, a Surveyor should be at least furnished with three of the latter and half a dozen of the former.

There is a great deal of mechanical nicety in setting these poles, and preserving a correct line throughout. To obtain this,— Set up a flag at your starting point, and one at the termination of your intended line, and, having selected the distances between, where you are desirous of setting up flags, direct your assistants to hold the flags carefully upright and move as you direct them, one way or the other.

It is frequently the practice for persons, who are directing the ranges, to stand immediately behind the starting flag, and, by first looking on one side and then on the other, to range the intervening flags with the distant one : this is by no means a correct method ; it is, in all cases, whether of directing the fixing of a flag by another between two fixed points, or the continuing of those points onwards oneself, an act of necessity, to stand some 10 or 20 paces off from the nearest flag, and, by contracting its visual breadth, to secure the correct position of the flag that is to be put down, by at once covering the three.

Having thus obtained a practical method of measuring along a base line, and not deviating at the fixing of every chain's length to the right or the left, we have but to secure a correct and equal extension of the chain at every length, to obtain the true length of the line required. Let one assistant take the lead, who is called the *leader*, having the ten pins and the chain handle in his left hand, and proceed towards the mark at the end of the line ; while the other assistant, called the *driver*, holding the other end of the chain also in his left hand, keeps it close against the mark or starting point ; when the leader has come to the end of the chain, let him turn round with his face towards the driver, and, taking one of the arrows in his right hand, let him pull the chain tightly, keeping it on the ground, and fix the arrow he held in his right hand in the ground.

As the chain is seldom straight at first, when the leader faces the driver, the driver must always keep *his* end to the ground, close to the mark ; but the leader, previously to finally setting the arrow, should raise the chain with both hands, shake it to the very end, and before he puts his arrow down, see that it is perfectly straight.

The leader must also observe, as he is putting down the chain, to look towards the driver, to see if he is himself in the right direction, or whether the driver is noting him to the right or to the left ; when he has succeeded in hitting the right direction, (which he knows by the driver calling out "*down*,") and, obtaining the proper measurement of the chain, by carefully complying with the directions above, and has put his first arrow down, he must *return* the cry of "*down*" to the driver, and, taking the chain up in his right hand, must proceed onwards until the driver, coming to the first arrow, cries out "*stop*," when the same process is repeated. The leader having a second time returned the cry of "*down*," the driver picks up the arrow, carrying it in his left hand, taking care, as long as he has two or more flags to cover before him, to direct the leader ; when (the leader), however, has past all

but the last flag, the proper position of the arrows depends upon himself, which he must carefully do, by covering the backward flags.

When the leader has put down the ten pins, he calls out "*tally*," and the driver, dropping his end of the chain, comes up to the place, picks up the last arrow put down, counts over the whole of the arrows, and, putting his foot to the mark, returns them again to the leader, who finding they are the right number (10), proceeds with the chaining as before; the second "*tally*" is then called; then the third; each cry, to prevent mistakes, being repeated by the driver. I would here observe, that many errors have arisen from the omission of these apparently trifling superfluities, and much valuable time lost. It is this superfluity of care which distinguishes the man of practice from the novice—the latter *thinks* it so simple that he cannot make a mistake; the former *knows* that, from that very simplicity, error is more likely to ensue.

In this manner they continue, till the whole line is measured. Should it not be a complete chain to the end, it is usual for the leader to put his end of the chain down to the mark, and for the driver to read off the distance; and this is certainly the correct method, except there are offsets to be taken between the last full chain and the end of the line, when the driver had better put his end down, and the leader pull up, as the position of the offsets is measured from the driver's end.

It is advisable, at the end of each tally, to leave a mark in the ground, and note it in the field book, especially when the line is long, as the latter prevents mistakes, and the former makes the correction of it more easy.

It need only be added that the leader having put down his arrow, should, on proceeding, walk so as to keep the flags, that are before him, always covered, and, on turning round, should so arrange himself as to cover the starting point (as near as he can) with the last arrow put down; this will be saving the driver some considerable trouble in keeping him in the proper direction, and will, at the same time, give the leader himself a facility in keeping the course, when a hill, or other local obstruction, may hide the forward flags from the driver's view.

For practice—I would recommend some line to be selected of about half a mile, which should be carefully measured two or three times, until not more than an error of a link should be detected, in two or three consecutive trials.

Note.—[It is requisite, to ensure accuracy, that the chain should be frequently measured. If found incorrect, take care that the error be divided among the whole, so that *each* 10 links be of equal length; in each 10 links, it is better to cut off a little from several rings, than take a whole ring away.]

Professional Surveyors measure their chains regularly every morning, when they are on service.

CHAP. II.

THE OFFSET STAFF,

(*For the Chain.*)

Is a narrow slip of wood about $1\frac{1}{2}$ inch wide by 1 inch thick, and generally 10 links long, divided into links; it is made of deal or some light wood, and should be furnished at one end with a small notch or hook, to put the chain through the hedges, and be numbered on both sides, from different ends. It is used for the purpose of measuring short distances, called offsets,* from the line to the hedges, &c.

As these offsets must be measured at right-angles to the chain, the Surveyor should stand on the opposite side of the chain to the hedge, or object to be measured to, and walking along the chain, looking at either end, mark where a perpendicular from the given object would fall upon the chain. These are but approximations, but practice will soon make them as practically accurate as is necessary.

* These offsets should never exceed one chain.

CHAP. III.

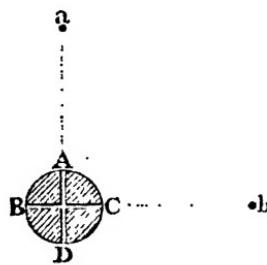
ON THE CROSS STAFF.

THE cross staff is about 5 feet 6 inches high, pointed, and strongly shoed, to drive into the ground, and having at the top a circular piece of brass, or hard wood, so fixed as to be taken on and off.

The piece of brass is furnished with four brass pins, set at right-angles to each, and the wood head is divided into four divisions, by grooves, also at right angles, cut about half an inch down into the wood, which is about 2 or $2\frac{1}{2}$ inches thick. This head is sometimes made with a spring, to move up and down the pole of the staff, and to remain firm at any height.

It is always requisite to test the accuracy of these grooves, which can be done in the following way :—

Let ABCD be a cross staff; and AD, BC, its sights or grooves; place a flag, *a*, in the direction of the groove AD, and another at *b*, in that of BC; half reverse the cross, so that DA point to *b*, and CB to *a*. Then, if the flags are still in the direction of the grooves, the instrument is correct.



The principal use of the cross staff is in the measurement, by perpendiculars, of straight-sided fields; and in the setting off of short perpendiculars from a base line; in the laying out of streets and building lots: and generally, in determining, by means of a perpendicular therefrom, the position of a house, mile-post, pond, or any conspicuous object, whose approximate situation alone is required, either to fill up or elucidate the plan, and whose distance from the measured line might exceed the limit of an offset.

It is especially useful where no great accuracy is required, when, in the computation of the areas, it is not requisite to take into account the irregularities

of the hedges, as, *in surveying a field*, it saves the necessity of measuring the whole way round, and makes a diagonal, with two perpendiculars from each of the opposite corners, all the admeasurements required.

In the measurement of straight-sided fields, when the perpendiculars are taken from the diagonal to the opposite corners, considerable trouble is sometimes experienced in determining the position of the perpendiculars on the diagonal; and two or three trials are often required, when accuracy is wished for. To remedy this, a little portable instrument was invented by a friend, which can be carried in the hand, and which at once determines the required position of the perpendicular. It is a small oblong piece of wood, of about two inches thick, with two pins placed vertically, having a small mirror, at an angle of 45° , which being held in the line of the diagonal, and turned towards the flag, which is set in the corner of the field, gives you, when your hand is at the required position, the reflection of the flag in the same line with the pins.

By holding this in your hand, keeping the pins in the direction of the diagonal, and walking along it till you perceive the reflected flag immediately in the line of the pins, the true position of the perpendicular can be correctly determined.

It is a simple but very useful instrument.

CHAP IV.

THE FIELD BOOK.

THE field book should be of a convenient size for the pocket, having the left page ruled with a central column, and the right page left blank for remarks. The central column should be headed "*Chains*," on either side "*Offsets*," and the right page "*Remarks*."

The central column is intended for all actual lines measured, and, by commencing *from the bottom of the page*, the page becomes a smaller representation of the reality with the line measured from you, and the

offsets, at their respective distances on that line, taken at so many links to the right or to the left, as are actually on the ground. In keeping the field book, it *first* should ever be remembered, that the central column is virtually but one line, representing the chain, the space *within* the column being merely required for the several distances on the chain, whence the offsets are taken; and, *secondly*, that all offsets, read either way, outward *from* the central column, in the same way as they are measured outwards *from* the chain.

To preserve uniformity, as it is more natural to measure from left to right, the place measured *from*, is put on the left of the central column, at the bottom of the line, and the station measured *to*, is put at the top, to the *right*; the points of commencement and termination of the line can thus be immediately seen.

The book should be interleaved with blotting paper, and the entries, if possible, should be made in ink. The pages should also be numbered, before beginning for facility of reference.

If the direction of the line is determined by an angle taken by the theodolite, or the bearing of the line be given by the circumferentor, the angle of the former or the bearing of the latter is placed in the central column, immediately above the starting point, according to the examples below:—

When the line crosses a road, or hedge, &c., make corresponding lines in the field book, as, in the first example of Theodolite Surveying, at the several distances on the line: of 2 links; 1 chain 10 links; 1 chain 26 links; 11·20 chains; 11·40 chains; 13·21 chains; always considering the column as but a line.

In taking "offsets" to corners of fences, houses, &c., mark the relative position of the corner, as to the chain line (see distance 6·79 on line 6·90; and 0·10 on line 7·31, in the example of "CHAIN

SURVEYING") and generally be careful to make the field book, as much as possible, a *fac simile* of the ground itself, with each post, hedge, house, &c., placed on the book, as to the central column, considered always as a line, in the same position as they stand to the chain on the ground. And think not that any time is gained to the Surveyor by hurrying over the notes in the field. *A little care in the field will save much trouble in the office.*

Stations are generally expressed in the field book by the following character Δ , which, in the plan, is represented by a circle in pencil, drawn round the station point, which should be always that of a needle.

I would never recommend the use of letters for stations in Chain Surveying. In the first place, they are soon exhausted; in the second, they in no way assist the memory. The **BASE LINE**, perhaps, had better, when referred to, be termed the *base line AB*, in contradistinction to the secondary lines, which are required in surveys of some extent, and are virtually base lines to their own portions of survey, and may be lettered *CB, EF, &c., &c.*

In all other cases, distinguish the lines by their lengths, and the points upon them, by the distance of those points from the zero end of the lines;—thus, in the *first example* of the method of keeping the field book for chain surveying, “*from 609 on 609, to 0 on 609,*” the line begins at 609 on 609, and runs to 0 on 690—that is, the measured line is a line, connecting the end of the line 609 with the commencement of the line 690.

And again, in the *second example*, “*from 685 on 731, to 574 on 635,*” the point started from, is that point upon the line 731, which is 6 chains 85 links from the zero end of the line, and its termination, a point 574 on another line 635.

In *theodolite* surveying, it is better perhaps to use letters (see example), as the stations are but few, and mostly come within the exception above referred to, letters, in this case, as being usually applied to trigonometrical stations, may be, therefore, more characteristic and distinctive.

Surveyors sometimes take the bearing of the base line at the commencement of a survey, and enter it at once in the field book; this enables them to plan the estate in reference to the meridian line. It must be remembered, that this is but the magnetic bearing, and must be entered as such.

The correction for the variation* must be subsequently determined, and the true bearing of the line inserted in the book.

EXAMPLES OF FIELD NOTES.

(To be read upwards.)

| Chain Surveying. | | Theodolite Surveying. | |
|------------------|--------------------|-----------------------|---------------------------|
| | 6'90 to 574 on 635 | | 20'47 Δ |
| — D — | 6'79 — X | path — | 13'21 — X |
| 10 + 1 | 700 | hedge — | 11'40 — X |
| 10 + 5 | 500 | D — | 11'20 — X |
| 10 + 4 | 400 | | 11'11 Δ |
| 10 + 3 | 300 | | 2'00 41 + 12 ^D |
| 10 + 3 | 200 | H — | 1'26 — X |
| 10 + 2 | 100 | D. of Rd. — | 1'10 — X |
| ^D | | to G.P. 33 | 0'80 |
| 10 + 0 | 0'50 | Road — | 0'02 — X |
| ^D | | between 0 | and 20'47 |
| 7 | 0'00 3 | on 1077 | 123°28' on 20'47 |

EXAMPLE II.

| | | | | |
|-----------------|------|------|-------|---------------|
| Maiden | { - | 7·31 | - x } | Lane |
| | { - | 6·95 | - x | |
| top of bank | - | 6·85 | - x | Δ to 0 |
| 10 + 20 | 6·70 | | | on 690 |
| 10 + 30 | 6·00 | | | |
| 10 + 36 | 5·00 | | | |
| 10 + 70 | 1·00 | | | |
| ✓ 90 | 0·10 | | | |
| from 609 on 690 | | | | |

EXAMPLE. I

| Theodolite Surveying. | | | |
|-----------------------|---------|------|-------|
| path — | 20·47 | — | Δ |
| hedge — | 13·21 | — | X |
| D — | 11·40 | — | X |
| | 11·20 | — | X |
| | 11·11 | — | Δ |
| H — | 2·00 | 41 + | 12 |
| D. of Rd. — | 1·26 | — | X |
| to G.P. 33 | 1·10 | — | X |
| Road — | 0·80 | | |
| between 0 | 0·02 | — | X |
| on 1077 | 123°23' | and | 20·47 |
| at 1077 on 1077 | | on | 20·47 |

EXAMPLE II.

| | | |
|-------------------------------------|------------------|----------------|
| | $82^{\circ} 55'$ | and Δ A |
| | $72^{\circ} 24'$ | and Δ E |
| between Δ C at Δ B | $68^{\circ} 28'$ | and Δ D |

between ΔD
at ΔC | $100^{\circ}20'$
 $14^{\circ}53'$ | and ΔB
and ΔA

| | | |
|-------------------------------------|-------------------|----------------|
| | $180^{\circ} 0'$ | and Δ C |
| | $168^{\circ} 49'$ | and Δ B |
| between ΔE at ΔD | $89^{\circ} 42'$ | and ΔA |

| from Δ A | 9·13 chs. | to Δ B |
|--|-----------|----------------|
| | 151° 59' | and Δ E |
| | 86° 27' | and Δ D |
| between Δ B at Δ A on base line AB | 11° 32' | and Δ C |

EXAMPLE 1.

of this subject, see "chapter on the

Surveying by the Circumferentor.

| | | |
|--------------------------------------|------------|-------------|
| | 17.84 | to 0 on 755 |
| | 6.00 | 25 to fence |
| | 250 | 30 to fence |
| | S.63°45W. | |
| pond { 60+10 { 50+12 | 15.41 | 0 |
| | 12.00 | 20 to fence |
| | 10.00 | 15 to fence |
| | 2.00 | 10 to fence |
| | N.6.15E. | |
| | 8.82 | 4 |
| | 1.00 | 28 |
| | S.75E. | |
| from corner of field near road | 7.55 | 6 to fence |
| | 3.50 | 16 to fence |
| | 0.00 | 3 |
| | S.49°30'E. | |

As it is indispensable, for the proper keeping of the field book, that the common general and local divisions of property should be understood, I have subjoined a few remarks on that subject.

THE BOUNDARIES OF PROPERTY.

The usual boundary of the field, where the ditch is between it and the hedge, is the brow of the ditch, or that edge of the ditch which is furthest from the hedge. This, however, is not always the case, as sometimes it is the stem of the quickset, or the roots of the hedge ; depending upon local custom.

The common allowance from the quick root, for the brow of the ditch, varies, in different places ; being 4½ links, 6, 8, and sometimes as many as 9 links.

When between fields, it is ordinarily 5 links ; when the ditch separates two contiguous properties, 6 ; and adjoining waste lands, moors, commons, roads, &c., generally 7 links. A wall is generally the division line between the properties, on which side soever the ditch may be placed. When there is a boarded fence between two fields, the fence belongs to the occupant of that side where it is clap-boarded, as the nails are considered to be driven home.

The centre of a stream running between two properties, is usually the boundary line.

In most parishes in England, a parish ditch is the boundary ; the course of this ditch having been altered by time and circumstances, will account for the apparent freaks of the division of properties, which is so striking in the map ; running across fields, dividing a pond, and putting one end of a street in one parish, and the other end in another.

It is particularly important in taking the notes of any survey, whenever any of the measured lines cross, or come near any of the division lines of the adjoining property, that these division lines should be noted in the field book. It at once localises the estate surveyed, and is oftentimes of great use to the Surveyor afterwards.

NOTE.—[It were needless to observe, that, whether the contents of the ditch are, or are not, to be taken into account, depends upon the object of the measurements. No one would think of *including* them, in the measurement of a crop of wheat or barley, or of *omitting* them, when the area of each field is required, for the determining of the area of the whole.]

CHAP. V.

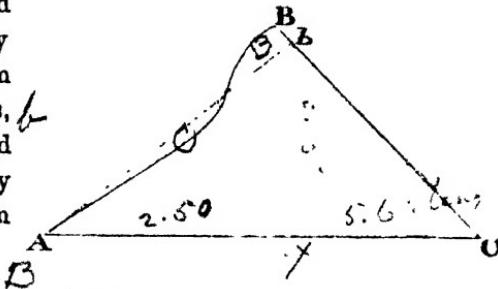
SURVEY OF A SINGLE FIELD.

Triangular.

HAVING given a description of the chain, offset-staff, &c., together with the method of ranging correctly, and the best plan of keeping the field book, we will now proceed to the several plans adopted in actual survey, according to the circumstances.

Let ABC be a triangular field to be surveyed.—The first thing to be ascertained, is, whether a plan is required, or whether the area alone is sufficient.

If only the area, observe whether the sides AB and BC are sufficiently straight and regular to warrant any point b to be taken, from which straight lines drawn to A and C, would approximate sufficiently to the area of the given triangle ABC.

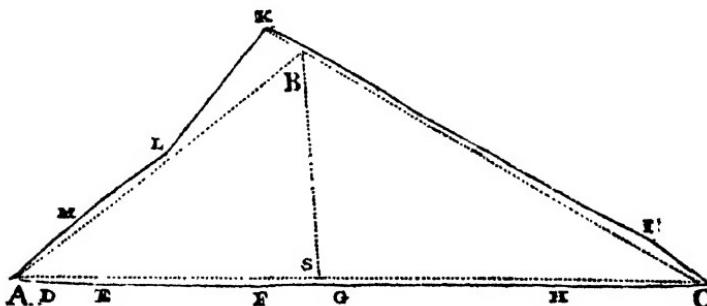


FIELD BOOK.

| OFFSET. | CHAIN. | OFFSET. |
|--------------------------|--------|---------|
| perp. from 260 on 560 | 3·15 | to R. |
| | 5·60 | to C. |
| from Δ A | 2·50 | |

Having carefully selected the point b, it will only be requisite, in measuring along AC, to observe where a right angle from (b') would fall upon AC, note in the field book 250; leave a mark in the ground, and measure to C = 5·60. Draw a line in your book, and measure from 2·50 to b = 3·15; then $\frac{1}{2}(3\cdot15)(5\cdot60) = 8\cdot82$ square chains = area of triangle ABC.

If a plan be required, or the hedges AB and BC be so irregular as to require offsets being taken, it will be requisite, for the sake of the offsets alone, to measure round the whole field along the three sides. Let the field now assume the form in the accompanying diagram:—



Select any points A, B, C, commanding the longest and nearest lines to the several hedges of the field, and place flags at those points A, B, and C.

| ΔS | FIELD NOTES. | ΔB |
|----------------------|--------------|------------------------------|
| From 5.70 on 12.73. | 4.25 | to 8.50 on 9.27 |
| X | 6.80 | 0 + D (to 0 on 12.73) |
| △ | 5.268 | being ΔA |
| from 8.50 on 9.27. | 3.204 | |
| ditch— | 9.27 | crosses 4 + D |
| △ | 8.503 | |
| . | 3.203 | |
| | 1.2014 | |
| from 12.73 on 12.73. | 0.000 | + D |
| D— | 12.73 | \times 0 + D to ΔC |
| | 10.18 | 4 + D \times |
| S △ | 6.156 | |
| | 5.704 | |
| | 4.8215 | |
| | 1.605 | |
| from ΔA | .564 | + D |
| | .000 | 0 + D |

First observe, whether the point A be exactly at the ditch; if it is, put in the centre line 0, and in the right hand offset line 0; proceed till you come opposite 56 links, the first corner or bend D; take the offset (4) to D; this line, connected with A, gives the actual position of the ditch. Had there been no remark at 0, on the line AC, there would have been nothing to show where the ditch went to from the point D; proceed along the line AC, marking down the several lengths, as marked above, with their respective offsets, and selecting some point S, 5.70, to be noted as a station, whence to measure afterwards, a check or tye-line to B; note it Δ (a station) in the left hand column, as the tye-line will be to the left. Having completed the measurement of the line, put *where to* in the right offset column at the top, and draw *one* line over the whole; all *loose* lines, that is, lines not connecting certain points, have *one* line only drawn over them; *fixed* lines have *two*. Measure next from C to B, write from 12.73 on 12.73 on the left offset column in your field book, and observe, as before, what the position of the ditch is; it will, of course, be 0, as before, as the station was at the end of the previous line, and the line was measured up to the edge of the ditch, which is denoted by the ditch crossing,—thus; ditch—(12.73)—X



and mark BA—the direction you are going in—whether to the right or the left, CB being the direction you have just come: BA the way you turn (to the left); proceed, therefore, measuring along CB, and marking the offsets, till you come to B, which enter in your field book $8\cdot50 \Delta$, proceed with the measurement to the hedge, making $9\cdot27$; put down in your centre column $9\cdot27$, offset 2 to right, and draw a line, thus:—

hedge—(9·27)— \times i.e. hedge crosses.

From B measure to A, and as B is $8\cdot50$, write in your book, in the left hand column, from " $\Delta 8\cdot50$ on line $9\cdot27$," and proceed as before to A, which is a point 0, or zero, on line $12\cdot73$, mark this on the right hand column; and, as this completes the triangle, draw two lines above it, instead of one.

Then, from point $5\cdot70$ on line $12\cdot73$, measure carefully the tie-line to Δ B, being $8\cdot50$ on $9\cdot27$, as the whole check of the accuracy of the measurements of the two sides AB, BC depends upon the correctness of SB; and draw two lines over it in the field book, writing *check* line across it.

NOTE.—[The reader's attention had here better be directed to the object, had in view, in the measurement of this line BS; it is not for the sake of determining the correctness of the point S, but that of the lines AB and CB. If either of those lines be made longer than it really is, the point B, on the line BS, correctly measured, would be found in the plotting to fall within the triangle; if shorter, it would fall without; but the position of the point S might be materially altered, without affecting the correctness of the length of BS. Yet the reason of the inutility of the line BS, as a check upon S, becomes the cause of its efficiency, in verifying AB and BC, as, whether S should be $5\cdot90$ or $5\cdot50$, when it is called $5\cdot70$, is not of much moment, as the *length* of BS would scarcely be affected by it.]

It is sometimes necessary and always desirable, that the field notes should be sufficiently distinct, as to show at once, when referred to, on which side of the ditch of a field the hedge is. The black line on the plan is the *ditch* line—the hedge line is seldom noted; to obtain the ditch line, when the hedge is between the measured or chain line, and the brow of the ditch, the offsets would have to be measured through the hedge to the brow of the ditch. To do this at every offset, would be too troublesome; the plan, therefore, generally adopted, is, at the first offset, to measure up to the roots of the hedge (say 12 links), and then from there, to the brow of the ditch ($+ \frac{D}{8}$), and assume this ($+ \frac{D}{8}$), as the average distance, to be added to the hedge offset, throughout the line.

This plan furnishes us at once with a simple method of determining the position of the hedge, thus :—the offset ($12 + D$) shows, that the hedge is *beyond* the field; 12 being *up to* the brow of the ditch, denoted by ($+ D$)—the distance from there to the roots of the hedge not being wanted, as not belonging to the field under measurement. And

^D
the offset ($12 + 8$) shows the hedge to be within the field, as the offset, or distance of the chain from the boundary or brow of the ditch, is not 12 links, but $12 + 8$ or 20; the distances being taken separately for the reasons above.

By adopting this plan, the Surveyor can at once define the position of the hedge. When several fields are measured, and the lines cross from one field to another, it is useful to mark, both where the hedges and the ditches cross.

To find the areas.

In finding the *area* of the field, there are two or three practical methods indifferently adopted.

THE FIRST METHOD.

That of dividing the field into a triangle (whose sides are the measured lines,) and into trapeziums, (whose heights are the measured offsets,) as being the most correct, though certainly the most tedious and troublesome, shall be the first described.

NOTE.—[The example is worked out fully for the use of the student, who may choose out of the three methods for himself.]

EXAMPLE 1.—To find the area of the previous figure from the field notes.

1. To find the area of the triangle ABC.

$$AB = 6\cdot80$$

$$BC = 8\cdot50$$

$$AC = 12\cdot73$$

$$2 \mid 28\ 03$$

$$\text{Area} = \sqrt{S(S-a)(S-b)(S-c)}$$

$$14\cdot01 \quad 14\cdot01 \quad 14\cdot01$$

$$6\cdot80 \quad 8\cdot50 \quad 12\cdot73$$

$$14\cdot01 = S = \text{semi-perimeter.} \quad 7\cdot21 \quad 5\cdot51 \quad 1\cdot28$$

| | | | |
|-----------------|---|------------------|-------------|
| log. 14·01 | = | 1·146438 | 10 26·69 |
| log. 7·21 | = | 0·857935 | A. 2·669 |
| log. 6·51 | = | 0·741152 | |
| log. 11·28 | = | 0·107210 | 4 |
| | 2 | 2·832735 | 2·676 |
| log. 26·69 chs. | = | 1·426368 | 40 |
| | | A. R. P. | 27·040 |
| | | Area of triangle | = 2. 2. 27. |

TO FIND THE AREAS OF THE TRAPEZUMS.

On line AC .

Rule.—Multiply the lengths, by half the sum of the heights, for each area.

On line BC.

| Lengths. | Perpend. Heights. | 1·20 | 5·30 | 2·000 | 7·70 |
|----------|----------------------|----------------|-----------|---------|---------|
| | | ·07 | ·03 | ·085 | ·035 |
| | | ·0840 | ·1590 | 10000 | 3850 |
| 1·20 | 0 | | | 16000 | 2310 |
| | 14 | | | | |
| 200 | 14 | ·0840 | 10 4399 | ·170000 | ·026950 |
| | 3 | ·1590 | ·04399 | | |
| 530 | 3 | ·1700 | 4 | | |
| | 3 | ·0269 | | | |
| ·77 | 3 | 4·399 sq. chs. | ·17596 | | |
| | | | 40 | | |

On-line PD

| | | | | | |
|----------|----------------------|-------------------------------|-----------------------------|-----------------------------|-----------------------------------|
| Lengths. | Perpend. Heights. | 8.200 .405 <u>16000</u> | 2.06 .06 <u>.1236</u> | 1.54 .04 <u>.0816</u> | 10 1.4812 .14812 <u>4</u> |
| 3.20 | <u>17</u> 4 | <u>128000</u> | | | 59248 |
| 2.06 | <u>4</u> 8 | <u>1.296000</u> | | | 40 |
| 1.54 | <u>8</u> 5 | | | | <u>28.69920</u> |

| | | |
|------------------------|--|-------------------|
| ·1236 | | |
| ·0816 | | |
| 1·2960 | | A. R. P. |
| <u>1·4812</u> sq. chs. | | Areas = 0. 0. 23. |

| | |
|---|--|
| A. R. P. | |
| Area of triangle = 2. 2. 27 | |
| Trapeziums on AC = 0. 0. 12·03 | |
| on BC = 0. 0. 7·03 | |
| on BD = 0. 0. 23·69 | |
| Area of field = <u>2. 3. 29·75</u> acres. | |

EXAMPLE 2.—Take, for practice, the field notes of the Fields Nos. 1 and 2, Plan No. 1, at pages 78 and 80, and calculate the areas, checking them by the following method.

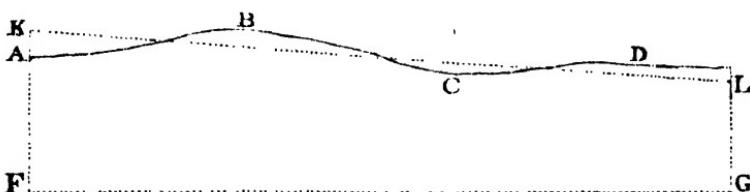
NOTE.—[Though this method is seldom requisite in small surveys, yet it is important that the reader should be acquainted with the best means of ensuring accuracy in cases of importance.]

THE SECOND METHOD.

A common method in practice, in computing the areas of irregular-sided fields, is to have a piece of transparent horn—and by *giving* and *taking*, as it is termed—to draw a straight-sided polygon, equal to the given irregular one, and, by means of the compasses and scales, to measure the lengths of the new lines, and from these lengths to calculate the area. This is a sufficiently close approximation in skilful hands; though I would counsel young beginners not to attempt it at first, until, by computing the area, by means of triangles and offsets, on several occasions, they have had opportunities of testing their own correctness.

EXAMPLE.—Let ABCDE be the irregular outline of a hedge or ditch, by placing along it a straight-sided piece of transparent horn; the position of the line KL can be determined, such that the area KLGF shall be

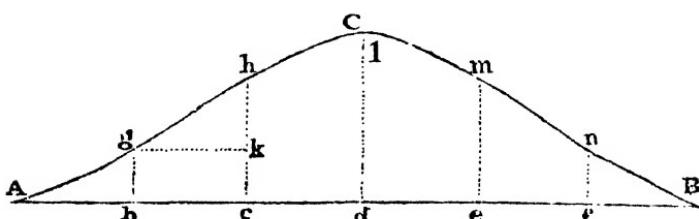
equal to the area of ABCDEFG; so that the pieces, taken in at A and C, shall be equivalent to the pieces given up at B and D.



NOTE.—[I would recommend the beginner to find by this method the areas of the preceding figure, and of the fields in the several figures in the work, comparing them with the calculated areas.]

As it is sometimes desirable to calculate in the field, without plotting, the area of an irregular figure, which has been surveyed by the circumferentor; and, as the space included within the lines of survey can always be calculated separately, without reference to the areas between these subsidiary lines and the natural boundaries, I subjoin an useful practical method of determining this area, when the offsets are supposed to be taken at regular distances.

To approximate to the areas of offsets, let AB be a straight line, near the curvilinear hedge ACB, and let it be required to determine the area, included between the line AB and the hedge.



In measuring from A to B, take at every chain's length, $Ab, bc, cd, &c.$, the offsets $bg, ch, dl, em, &c.$; then the whole area ABC, shall

be equal to the sum of $gb + ch + dl + em + fn$, multiplied into their common distance of one chain, or 66 feet.

For the area of $Abg = \frac{1}{2} Ab \cdot bg = \frac{d}{2} (bg)$, where $d =$ the distance.

$$gbch = bc \left(\frac{bg + ch}{2} \right) = d \left(\frac{bg + ch}{2} \right) = \frac{d}{2} (bg) + \frac{d}{2} ch$$

$$hcdl = cd \left(\frac{ch + dl}{2} \right) = d \left(\frac{ch + dl}{2} \right) = \frac{d}{2} (ch) + \frac{d}{2} dl$$

$$ldem = de \left(\frac{dl + em}{2} \right) = d \left(\frac{dl + em}{2} \right) = \frac{d}{2} (dl) + \frac{d}{2} (em)$$

$$mcfn = ef \left(\frac{em + fn}{2} \right) = d \left(\frac{em + fn}{2} \right) = \frac{d}{2} (em) + \frac{d}{2} (fn)$$

$$nfB = \frac{fB}{2} (fn) = d \left(\frac{fn}{2} \right) = \frac{d}{2} (fn).$$

$$\therefore \text{Area} = d (bg + ch + dl + em + fn).$$

EXAMPLE 1.—Let $d =$ one chain, and $bg, ch, \&c.$, respectively 10, 15, 17, 14, 9 links, what will be the area of the figure?

| | | |
|-----|----------------------------|----------------------------|
| 10 | ·65 sq. chs. = ·065 acres. | |
| 15 | | 4 |
| 17 | | ·260 |
| 14 | | 40 |
| 9 | | Poles <u><u>10·400</u></u> |
| ·65 | | |
| 1 | | |
| ·65 | square chains. | |

EXAMPLE 2.—Given the several offsets 15, 25, 40, 10, 60, 30, 25, 8, 18, 9, 8, 4, 6, 0, taken at one chain's distance. Required the area.

A. R. P.

Ans. 0. 1. 0.

TO MEASURE A FOUR-SIDED FIELD.

This must, in all cases, be divided into two triangles. If the boundaries are irregular, each hedge must be taken by means of a subsidiary line and offsets, and a diagonal line, drawn from the two opposite corners, that are most remote from each other.

If the boundaries are *regular*, the diagonal line alone need be measured, together with the lengths of the perpendiculars to each of the other corners.

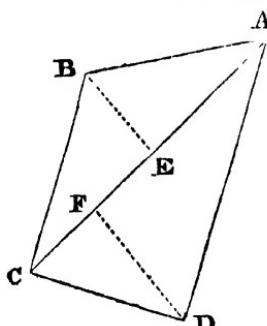
To compute the Area.

Find the area of each of the triangles that compose the trapezium, separately, adopting one or the other of the different methods of computation before enumerated, and their sum is the area required.

Let ABCD be the given trapezium, and first let it be straight-sided; measure AC and BE, and FD

$$\text{then the area will be equal to } \frac{BE + FD}{2}$$

$$\begin{aligned} &\times AC; \text{ for it is equal to } AC \cdot \frac{BE}{2} \\ &+ AC \cdot \frac{FD}{2} = AC \cdot \left(\frac{BE}{2} + \frac{FD}{2} \right) \\ &= AC \cdot \left(\frac{BE + FD}{2} \right) \end{aligned}$$



Now let the sides be *irregular*.—It will be necessary to measure all round for the sake of the offsets; one other measurement ties the whole in, and you have two triangles, ACB and BDC, upon a common base, CB. To prevent errors to young beginners, I would recommend them, in all cases, for some time, taking the tie-line from the base to the vertical angles, till they get into practice; thus, in the last case, in measuring from C to A, when nearly opposite (as well as the eye can tell) to D, take any station F; and, when opposite to B, any station E; after completing the diagonal CA, measure BE, FD; this will at once prove the accuracy of the measurements in the field of the several lines AB, BC, CD, AD, and will, at the same time, guard against any error in the plotting at home.

The following field notes, accompanied with a plan (being Field No. 1, in Plan No. 1,) of a four-sided field, near Maiden Lane, surveyed in the above manner, are given as an example of the method of keeping the notes in the field, and of plotting the work at home.

It were advisable that the student should plot every one of the examples of field notes *himself*, for practice, and compare them with the plans given.

FIELD No. 1, PLATE I.

| | | | | | | |
|-------------------------------|--------|--------------------|--------------|------|---------|-----|
| From 230 on 873 check line | 3·67 | to 480 on 485 | | | | |
| From 490 on 873 check line | 4·71 | to 574 on 635 | 70 40 | D — | 7·24 | — X |
| | | 8·73 to 619 on 624 | | 8 | 7·19 | |
| | 4·90 | △ | | 7 | 600 | |
| from 34 on 485 | 2·30 | | | 5 | 500 | |
| | | | | 6 | 400 | |
| | | | | 6 | 300 | |
| | | 7·16 to 34 on 485 | to gate post | 10 | 2·79 | |
| 10 + 24 | 7·00 | | D + 4 | | 2·00 | |
| 10 + 21 | 600 | | D + 5 | | 1·00 | |
| 10 + 19 | 500 | | | | | |
| to gate post | 22 | 392 | | | | |
| 10 + 11 | 300 | | | | | |
| 10 + 7 | 200 | | | | | |
| 10 + 7 | 1·00 | | | | | |
| to top of bank | 10 + 8 | 0·00 | | D — | 485 | — X |
| from 574 on 635 | | | | + 13 | 480 | △ |
| Maiden { D — | 635 | — X } | Lane | + 28 | 400 | |
| D — | 597 | — X } | | + 32 | 300 | |
| top of bank — | 587 | — X | | D | | |
| 4 | 500 | | 10 + 27 | 200 | | |
| 4 | 400 | | D | | | |
| 2 | 300 | | 10 + 28 | 1·00 | | |
| D + 2 | 200 | | D | | | |
| D + 5 | 100 | | 10 + 10 | 0·34 | △ | |
| to 2d gate post | 12 | 0·08 | 10 + 15 | 0·00 | | |
| | | | D — | | — X | |
| | | | Maiden | | — Lane. | |
| from 719 on 724 | | | | | | |

Commencing on top of bank by the road side, at the south-east corner of the field.

THE PLOTTING OF THE ABOVE NOTES.

First lay off the measured lines, independently of the offsets.

Draw any line AB, indefinitely in pencil, as your guiding line, so placed, as to throw the plan into a favourable position on the paper. Now, for the length of this line, by looking at the field notes you

will find that the whole line is 4·85 ; but there is a Δ at 24, and another at 480 ; 480—24, or 4·56, is the length of this subsidiary line, forming one of the lines of triangulation ; write in pencil 456 against this line.

The next line runs to the right, from 480 on 485, (which 480 from 0 is equivalent to 456 from 24,) and is 724 long ; but the station is at 7·19 ; draw any line, making the supposed angle with AB, and mark 7·19 against it.

The third line also turns to the right, and though continued to 635, across Maiden Lane, has its station point at 5·74 ; mark this line also, in its supposed position.

The fourth line does not say to the right or to the left, because it runs from a station 574 on 635, the end of the last line, to a previous station, 24 on 485, which was the starting point ; this line is 716 long ; mark this distance against it.

The next measured line is from 24 on 485, or the starting point, and runs to another previous station, 619 on 624 ; its total length is 873, and there are two stations upon it, 230 and 490 ; draw this line in its proper position, and mark off the stations, 230 and 490.

The next two lines are check-lines, the one (471) from 490 to 574 on 635 ; the other 3·67 from 230, to 480 on 485 ; mark these also.

Having placed roughly these several lines, with their given lengths, in their supposed position, proceed to plot them off correctly by triangles, marking in every case, on the plan, the direction the line was measured in on the ground, (see plan). AB, of course, is the base of the whole. Upon AB lay off a triangle, whose other two sides are 7·19 and 8·73, laying off upon 8·73 the stations 2·30 and 4·900. Then, to verify the correctness of the work thus far, measure the distance of the station 2·30 from the point B ; this, if the work be correct, should be 3·67, the length of the dark line.

Next, upon the line 873, lay off another triangle, whose sides are 574 and 7·16 ; the distance of the previous station 490, from the vertex of this triangle, should, if the work be right, be found 4·71. The whole field is now plotted.

METHOD OF SURVEYING AND PLOTTING TWO FIELDS TOGETHER.

The field in the previous example having been already surveyed, the adjoining field was added to it,

which is to be plotted from the accompanying field notes.

FIELD No. 2, PLATE II.

| | | | | | | |
|-------------------|------|-----------------|-----------------|------|-------|-----------|
| | | | D — | 7·31 | — X } | Lane. |
| Maiden | | | D — | 6·95 | — X } | |
| from 500 on 868 | 5·35 | to 685 on 731 | top of bank — | 6·85 | — X | △ |
| check line | | | 10 + 20 | 6·70 | | |
| | | | 10 + 36 | 500 | | |
| from 574 on 635 | 8·68 | to 609 on 609 | D | | | |
| | 500 | △ | 10 + 70 | 100 | | |
| | 690 | to 574 on 635 | 90 | 0·10 | | |
| — D — | 6·79 | — X | from 609 on 609 | | | |
| 10 + 1 | 600 | | D 15 | 609 | △ | |
| 10 + 5 | 500 | | 6 + 4 | 5·82 | | |
| 10 + 4 | 400 | | D + 4 | 500 | | |
| 10 + 3 | 300 | | D + 10 | 400 | | |
| 10 + 2 | 100 | | D + 16 | 300 | | |
| to top of bank 10 | 0·50 | 0 to D | D + 22 | 200 | | |
| to top of bank 7 | 0·00 | 3 to D in field | D + 27 | 100 | | |
| from 685 on 731 | | | to gate post 40 | 0·50 | | |
| | | | 3 5 | | | |
| | | | + 21 | .32 | | |
| | | | pond | | | |
| | | | pond + 20 | .12 | | |
| | | | from 719 on 724 | | | produced. |

Having the previous notes of Field No. 1, the following notes were taken for the survey of the adjoining Field No. 2.

Produce the line 7·19 to 609 further, which is a station—then, turning to the right, agreeably to the field book, mark off the distance 685, which is the station point in the next line 731; from this point, 685, the distance to a known corner in the first field, (being 574 on 635), is 690.

Now, because 574 on 635 is a known fixed point, and the line 7·19 of the last survey, is a fixed line, its production is also fixed, and the end 609 is a fixed point; the line joining 5·74 on 635, and this point is, therefore, a fixed line without being measured; upon this base, therefore, describe a triangle, whose other two sides are 685 and 690, and their intersection is also a fixed point. By measuring the diagonal line 869, its distance, determined in position by joining two fixed point, is checked by its measured distance.

Again, by taking the distance 500, upon this diagonal, and measuring the check-line 535, this line, measured from a point in a fixed line, to the intersection of two other lines, becomes a check upon the vertex of the triangle.

Having found the plotted check-lines agree with the measured distances, draw the subsidiary lines carefully in *red ink*, and proceed to lay off the offsets.

TO LAY OFF THE OFFSETS.

Take the first Field.

The offsets, on the line $\text{A}B$, are all to the left, mark off, therefore, the several offsets in their proper places, observing, in the case of the second offset, that $10 + 10$ is ten links up to the hedge, which, with the ditch, is ten links wide. This determines the position of the hedge to be *within* the field; without the 10, or, had it been ($D + 10$), the hedge would be *beyond* the boundary of the field. At $4'85$, the cross ditch of the field intersects the line.

In the next line the offsets are still to the left, and are marked ($D + 5$), showing that, in this case, the hedge is without the field. At $7'19$, there is an offset of 8, to where the edge of the side ditch intersects the side of a pond; at $7'24$ the cross ditch of the field intersects the line as before. The width of the pond 40 links, and its length 70 links, are marked.

In the next line, the offsets are still to the left, and the ($D + 5$) shows, that the hedge, in this case, is beyond the field. As this line, if produced, will cross Maiden Lane, it is produced across for the purpose of determining its width and position. In every case, in addition to the mere measurement of the field, it is advisable to annex such collateral localities as roads, turnpikes, ponds, &c., as may determine the relative position of the field. At $0'08$ there is an offset of 8 links (on the left) to the second gate post. In the following line 716, where the offsets are "still to the left" at 392, there is an offset of 22 to "gate post" only. Observe—that it is usual to take to the *first* gate post, and allowing the average width of gate to be 15 or 16 links, to determine the position of the gate by the direction of the line. It is difficult to fix the position without some fixed rule, such as the above, of always taking the *first* point of the gate, as you come to it, on the line, whence the offset is taken.

The first offset on the line 716 is (10+8), always on the left. This 8 is up to the bank, bounding the lane, and 10 is the distance to its top. This 10 is repeated throughout, the average width having been taken. The other three lines have no offset to them.

OFFSETS OF SECOND FIELD.

The first line is 609, and the first offset is at 1·12 (still on the left), being (pond+20), or, 20 up to the edge of the pond+20 across; the second offset at 1·32, is (35+21), that is, 21 up to the pond +35 across the pond, which, by the accompanying diagram, ends there; the distance 1·32 being taken, in order to show that there was the end of the pond.

The next offset is at 50, that is, 40 to first gate post; at 1·00, the offset is (D+27), that is, 27 to the brow of the ditch, which being beyond the boundary of the field, the width of that and the hedge is not required; the other offsets are regular to the distance 5·82, which

has an offset of (6+4), that is, 4 up to the hedge, and 6 through the hedge, which is now within the field, to the brow of the ditch; the previous offset was (D+4), that is 4 to the brow of the ditch (the hedge being then beyond the field), the ditch therefore *changes* at

this point, denoted in the diagram by | . There is here also a cross

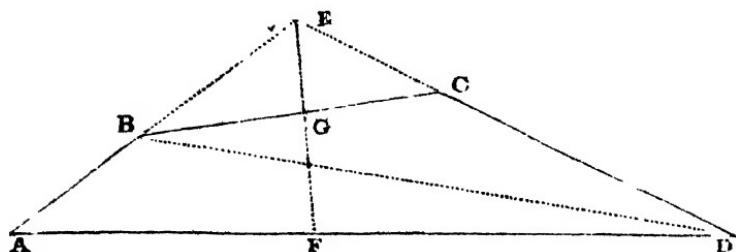
hedge, on the left, which the diagram ——! | shows too.

In the next line, at 1·10, there is an offset, taken to the corner of the field, ·90 links; and the relative position of the sides, to the offset line, is expressed by the diagram in the notes, which should ever be, as much as possible, a ground plan of the locality.

In line 690, the first offset taken is at the station point or 0·00, 7 to the left, and 3 to the right: showing, that this point is between the top of the bank by the road, and the *field* ditch at the bottom; at 50 links, the line touches the ditch, having 0 offset to it.

There are no offsets to the two following lines.

There is a form, however, of a four-sided field, which it would be dangerous to measure in the manner before referred to.



Let ABCD be the field. To measure BD as a diagonal, and to let fall perpendiculars thereon (supposing the field to be *straight sided*), would be troublesome, and might be productive of error; and, to divide the field into two triangles, with BD for their base, subtended, in the one case, by so obtuse an angle as C, and, on the other, by so acute an angle as A, would be equally hazardous. One good tye line, from C to BD, might be taken; but a tye line from A, except upon the production of DB, which might be impracticable, would be useless.

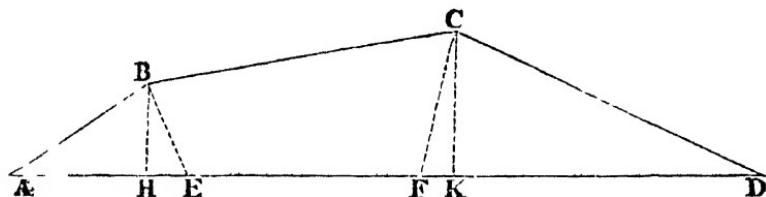
It would be better, according to the circumstances of the locality,—

1st.—Either to produce DC and AB to their intersection at E; and, taking all the requisite measurements, for the computation of the triangle AEC, take, at the same time, those for the area of BEC, and subtract the smaller from the greater, for the required area.

EXAMPLE—Let AD be the base measured, and take as a station, any point F, opposite to E; from A measure along AB, towards E; at the distance AB, in the field book, mark “*fence of field crosses*,” (and also “*out of field*”) and continue the measurement to E. From

E measure towards D, marking (out of field) till you come to C, where, at the distance EC, say fence of field crosses, and proceed to F. From the station at F, towards E, measure to CG, (where fence crosses,) and continue to E ; measure CB.

2nd.—If the hedges be so high or thick, that you cannot range beyond—or, if there be any other local objection, I would prefer the following method :—



In measuring from A to D, take a point E, about the same distance from A as B is, and mark it a Δ ; and at F, take any other point, making FD about equal to CD, marking it in the field book as Δ , and proceed to D; measure the other three sides, which must be done, for the sake of the offsets. Then, from E measure to B; and, from F to C, the points B and C become the vertices of two triangles, ABE, and FCD, whose bases, AE and FD, being given, as well as the lengths of their respective sides, the vertices B and C are given, and therefore the line BC, between them, without being measured, is given also; by measuring the actual distance between B and C, BC becomes a check-line. Other check-lines BH, and CK, to the several triangles upon the base AD, can be taken or not, as the importance of the survey may or may not render it necessary.

THE LINEAR MEASUREMENT OF AN ANGLE BY THE CHAIN.

It must have been observed by the student, that this method of chain surveying is but a practical application of Euclid's problem, (see Problem x, page 5,) "for constructing an angle, equal to a given angle,"

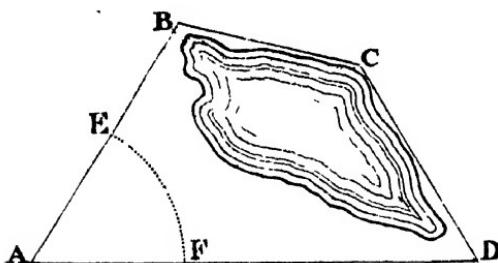
viz., by completing the triangle, and making a second triangle equal to the first.

In *chain surveying*, an angle is not measured by the number of degrees and minutes it contains, but by the length of the base that connects its sides, either completing the field (if it be of a triangular shape), or, as a diagonal, forming the base of another triangle, or of a new system of triangles.

The length and position of this connecting side, or linear measurement of the angle, should not be taken indifferently, but should be so selected, as to connect sides, as nearly equal as possible to each other, and to the line that connects them, forming an equilateral triangle, where possible, or an isosceles.

Again, when practicable, the largest triangle should be taken, though it is not actually necessary, as is shown in the following example.

Let ABCD, be the given angle.



It is not actually necessary, to make AB, AD, the two sides of a triangle, whose base, BD, shall determine the measure of the angle at A. Any points on AB, and AD, may be taken at pleasure, as E, and F. The distance FF, will determine the angle at A, and the position of the sides AE, AF. These lines are but parts of the straight lines AB, AD; and, therefore, AB, AD, are determined ; and the points B, and D, upon them.

There might be many local obstructions to measuring from B to D, and AD might be disproportionately long to AB. By making AF

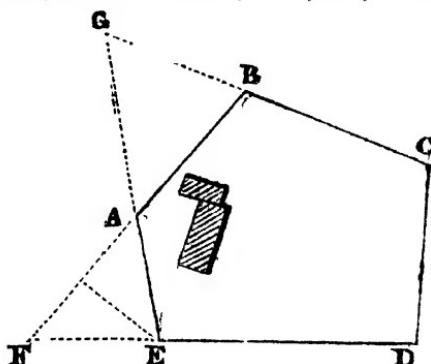
as nearly equal as possible to AE, you obtain a far more favourable measure of the angle at A than BD might be.

THE MEASUREMENT OF AN ANGLE, EXTERNALLY, BY MEANS OF THE OPPOSITE, OR SUPPLEMENTAL, ANGLE.

It sometimes happens, that an angle cannot be measured internally, from local obstructions, such as trees or buildings intervening between the sides that include it. In this case the following plan should be adopted.

Let ABCDE be any enclosure, such as a wood, river, &c., whose angles cannot be measured internally ; or, a farmstead, which, from the position of the buildings, must be measured from without.

Assume the side AE, as the base line, and, for the sake of the offsets, measure all round the enclosure, taking the lengths



of the sides, ED, DC, CB, BA. The nature of the obstructions prevents the measuring of the side AD, in the triangle ADE, which would give the angle AED ; and, also, the length of BE, in the triangle ABE, to obtain the angle BAE. Neither of these angles, therefore, can be obtained in the usual way.

Though the angle BAE, however, cannot be *directly* determined, there are three different angles, from which it can be deduced, viz.:— its own opposite angle, GAF, or either of the supplemental angles, FAE, BAG. Some one or other of these may be free from local obstructions.

First, let the supplemental angle, GAB, be free ; produce the range EA to G, where CB, also produced, intersects it ; measure AG, AB, and GB ; AGB is a triangle upon the base AG ; AE is determined in position, and, therefore, its production AG, is so also ; and GB, and AB, being measured, B becomes determined ; and the line GB and its *Production* BC, and therefore C ; but E is determined ; an imaginary

line, EC, connecting them, becomes so also. This being the base of the triangle CDE, whose two sides, CD and DE, are measured, D becomes determined, and therefore the whole figure is determined, by measuring the supplemental angle GAB, making, in all cases, its sides, BG and AG, the *production* of the sides, CB and EA respectively.

For the purpose of *obtaining the offsets*, it would be necessary to measure every side; there are, therefore, but two, not subsidiary, viz., the supplementary sides, AG, BG, which have to be measured, *purely as such*.

The same plan can be adopted, with any number of angles, modified according to circumstances, and having always regard to the using of as few supplementary lines as a due attention to accuracy will permit.

CHAP. VI.

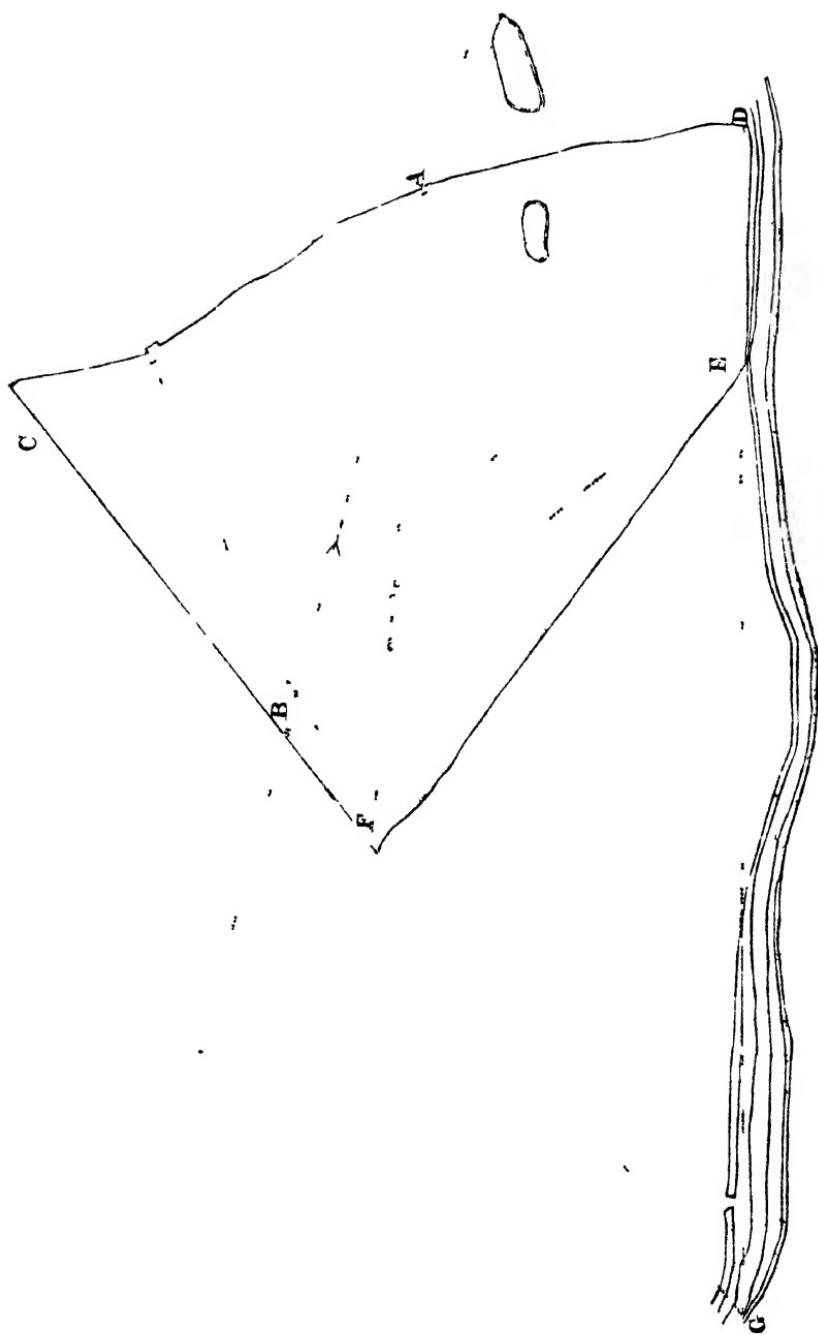
SURVEY OF FIELDS OF MORE THAN FOUR SIDES.

MANY-SIDED fields are of so many various forms, that it is impossible to give any specific rule for all.

The chief point in this, as in most cases, to be secured, is, that as the *whole circumference* of the field *must* be measured for the *sake of the offsets*, a selection should be made of the fewest and the most favourable supplementary or diagonal lines, with the best points for check-lines, to the corners.

The following field notes, of a five-sided field near Highgate, are given as an example, which the student is recommended to plot for himself, and compare with the accompanying plan.

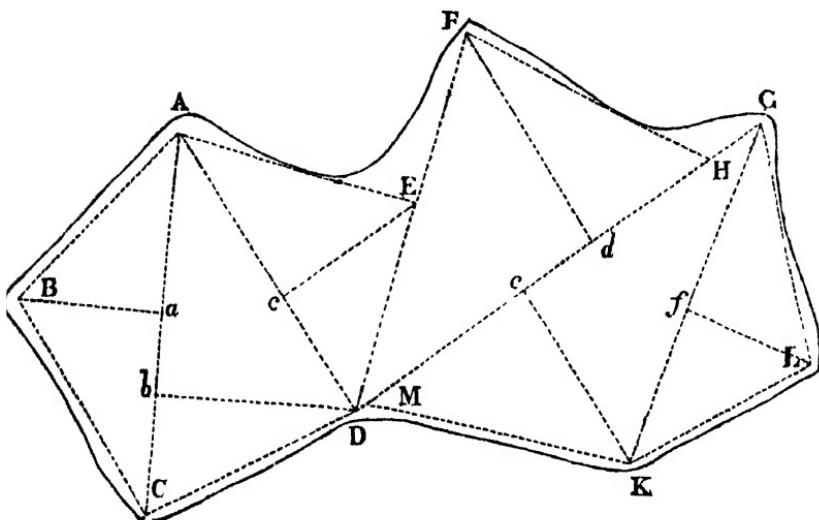
| | | | | |
|------------------------|-------------------------|---|-------|--------------------------------------|
| From 608 on 608 | to 11.97 | | △ | 8.67 110 } gateway 8.43 106 } |
| | 6.80 on 11.97 | | | |
| pond | 6.08 to 2766 | | | 7.00 40 |
| 70 + 76 | 4.55 on 3400 | | | 6.08 24 to stile |
| 70 + 75 | 4.25 | | | 6.00 24 to stile |
| | 4.00 22 | | | 5.00 17 |
| | 3.00 30 | | | 4.80 20 |
| | 2.00 20 | | | 4.00 24 |
| from 456 on 462 | 1.00 21+d | | | 3.00 25 |
| | | | | 2.00 30 |
| | | | | 1.00 22 |
| | | | | 0.00 10+d |
| D— | 4.62 —X2+8+r | H | | |
| △ | 4.56 | | | from 27.66 |
| | 2.00 10 | | | on 34.00 |
| | 1.90 12 | | | to dwarf |
| | 0.26 11 to G.P. | | | pollard } 2 |
| | 0.10 10 to G.P. | | | blazed } 34.00 △ |
| | 0.00 10+8+r | H | | path— 33.60 —X |
| from 1197 on 11.97 | | | | pond |
| | | | | 31.40 40+10 |
| | | | | pond |
| | | | | 31.00 40+60 |
| △ | 11.97 10 > | | | pond |
| | 11.50 18 | | | 30.00 100+60 |
| | 7.00 18 | | | D— 27.80 —X |
| | 6.00 15 | | | 27.66 △ |
| | 5.00 15 | | | 20.00 △ |
| | 3.00 18 | | | path— 17.90 —X |
| | 2.00 20 | | | D— 16.63 —X △ |
| | 1.00 20+d | | | |
| | 0.48 12 to G.P. | | | |
| | 0.32 12 to G.P. | | | |
| | 0.00 12+d | | | |
| from 10.10 on 10.32 | | | BASE- | LINE. |
| D— | 10.32 —Xo+d | | | |
| △ | 10.10 | | | |
| | 8.50 | | | |
| path + 40 | 7.46 to 16.63 on B.L. | | | |
| | 3.00 0 | | | |
| | 2.00 0 | | | |
| | 0.00 2+d | | | |
| from 8.67 on 8.67 | | | | |



I have subjoined, for the practice of the student, a few examples of the best methods that might, under different circumstances, be advantageously adopted.

EXAMPLE 1.

In the survey of many-sided fields, it must be remembered, that the whole field can always be divided into as many triangles (less two) as the figure has sides. Thus in the annexed diagram,



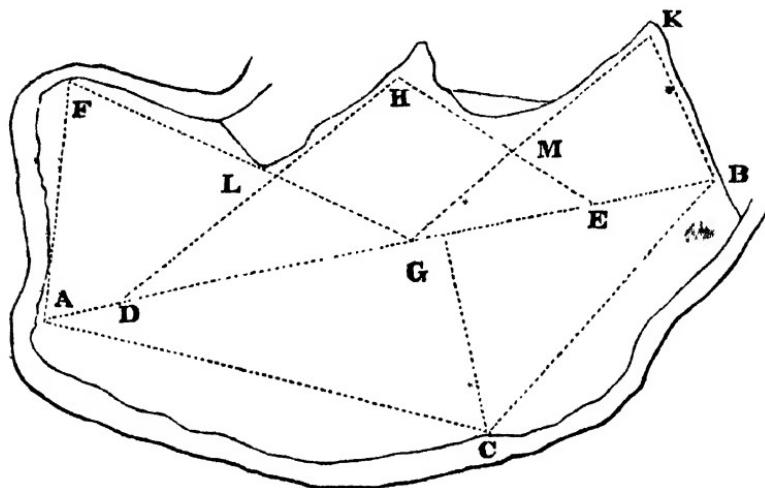
the whole figure ABCKG, by drawing the diagonals, AC, AD, FD, DG, and GK, is divided into triangles.

I would not, in most cases, recommend this method, for it is violating one of the first principles of surveying, viz., that of working from *whole to part*.

Instead of confining the whole field within the limits and error of one triangle, this method not only depends upon a *number* of triangles and the errors of each, but the error of any one of the triangles is not confined to itself, but is carried through the whole. Thus the triangle GKL, (being correct in itself,) depends upon the correctness of the triangle GMK ; the two triangles, GKL and GMK, upon the triangle DFH ; DFH upon AED ; the triangle AED upon ADC ; and ADC, and all the preceding, upon the correctness of the one triangle ABC.

EXAMPLE 2.

In the survey of the Field, in the accompanying diagram, a different arrangement has been adopted.



One base line, AB, has been taken through the centre of the field, and the various triangles have been based upon it, determining the several points required: each point in this arrangement depending solely upon the accuracy of the admeasurement of the triangle to which it belongs.

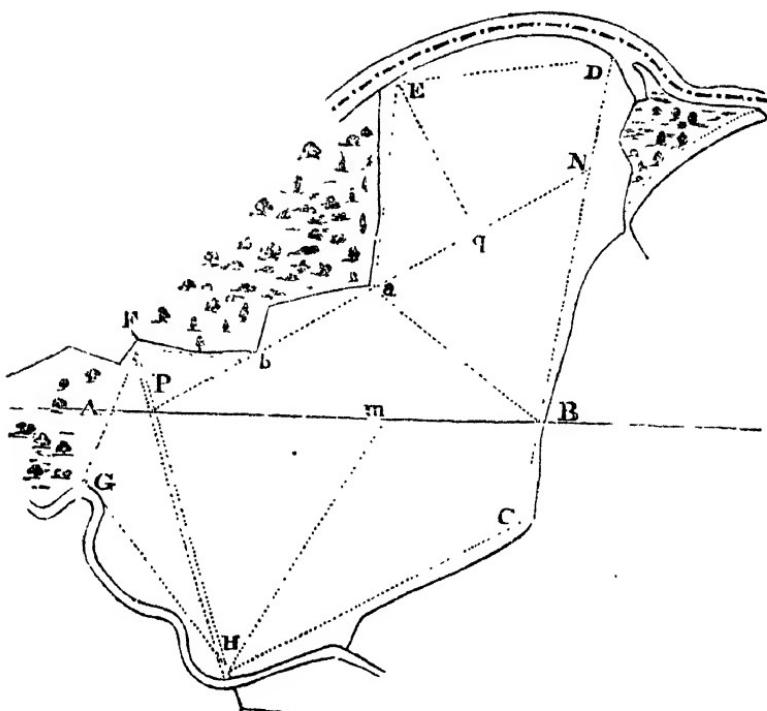
For example, take the point C, by the side of the river—this point is the vertex of the triangle ACB, whose base is the base line AB, and whose accuracy is secured by the check-line GC. The points F, H, K, also, are the vertices of the triangles, AFG, DHE, GKB, whose bases are respectively AG, DE, GB, portions severally of the base line AB. The distances DL, ME, not being required for the determining the vertex H, become check-lines upon all three.

The base line AB, and the check lines GC, LG, ME, are the only supplementary lines required.

EXAMPLE 3.

The next example is that of a very irregular field (selected for practice sake from the centre of a survey),

through which the base line (AB) runs in a certain direction, which direction is required to be known.



Through B draw any *loose* line CBND, so placed as to command the offsets along the whole of that side of the field, selecting upon it such a station N, as to be in the same straight line with $P b a$, produced. The distance of the line PN fixes CD, and PN itself; the other side of the triangle is also fixed, and the point a is a fixed point, and therefore the line aB . By *measuring* aB , the correctness of the whole triangle is determined.

Again, a being a fixed point, as well as D, an imaginary line, joining them, is also fixed, which is the base of the triangle EaD; by measuring the sides aE , ED, the position of E is determined; the line Eq is a check-line in this case; thus far Eq, aB , and aN are the only supplementary lines required.

As b and A are fixed points, measure bF and AF, and you obtain the point F; the only supplementary line required being Pb .

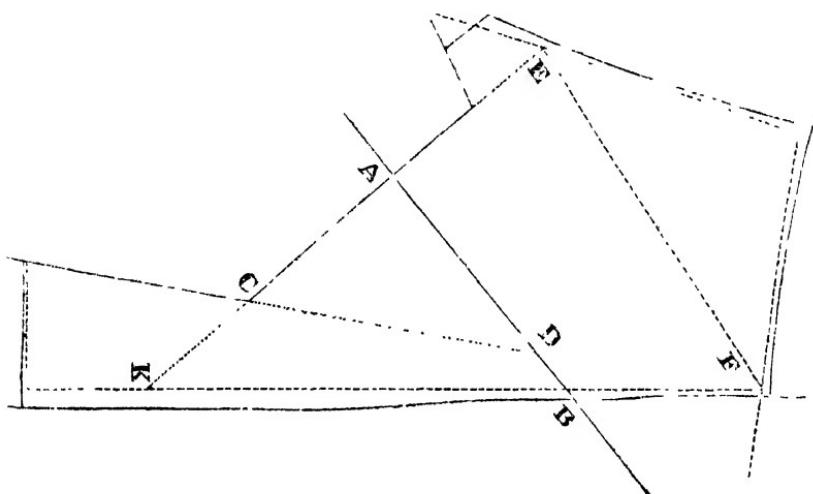
By producing FA to G, G becomes a fixed point; but C is a fixed point; the imaginary line of connection, CG, is also fixed; and the

measurements GH, HC, determine the position of H. All these lines are required for the offsets; by measuring, along the footpath, the line HP, you have a check upon the whole of this latter part of the work.

All the supplementary lines, required for the survey of the whole field, are Pb, aN, Eq, aB, and PH.

EXAMPLE 4.

This example is that of a field under similar circumstances, of a "base line running through it," but the general position of the field is different, and a different arrangement required.



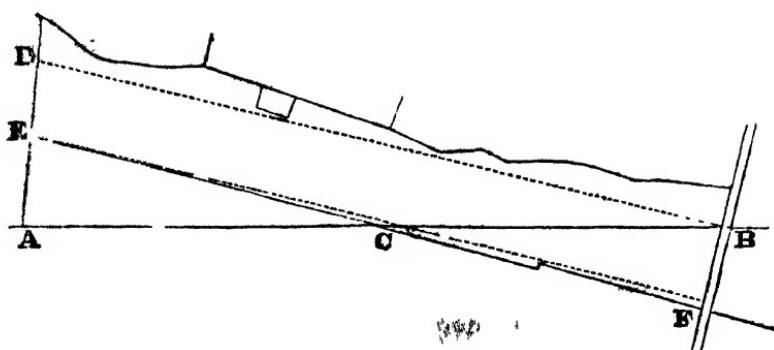
The same principle is still kept in view, of making the sides of the triangles serve the purpose of taking the offsets.

In this case there is but one supplementary line CD, and two check-lines, BF, and CK, or lines of verification to the whole.

EXAMPLE 5.

This example is that of a long narrow slip, crossed at one corner by the base line.

The best method of surveying it, if circumstances will allow, is given in the accompanying diagram.



By measuring CF and FB , on the base CB , CF is determined as to its relative position to CB , for the side BF measures the angle BCF . Produce FC to E ; the point E is determined, and the supplementary line EB . By measuring ED and BD , the other two sides, they both become fixed lines also.

As the accuracy of the whole, however, depends upon the correctness of the measurement of the angle BCF , it would be advisable to produce DE to A , and measure AC , and then obtain the measures of the opposite angle ACE , as a check upon the other.

CHAP. VII.

TO SURVEY SEVERAL FIELDS TOGETHER.

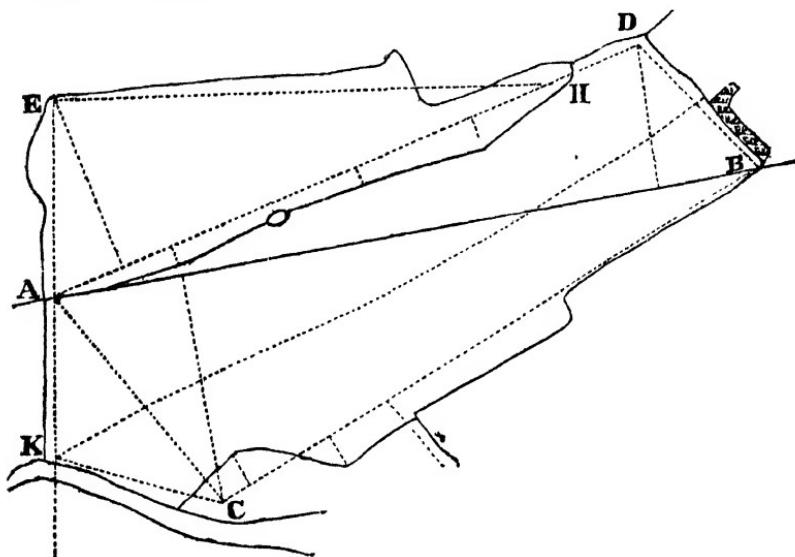
WALK over the ground first, and carefully examine into the relative position of the fields, and the most eligible points, for fixing the main and subordinate triangles, having reference always to making the lines of the triangles *subsidiary* to the measurement of the hedges; at the same time, taking care, that these triangles are nearly equiangular, right angled, or isosceles; and that they are bounded by fixed points, such as posts, stiles, houses, corners of fences, &c. There should be as few as possible supplementary lines: lines

that are merely used in determining the triangles, and do not also serve for taking the offsets.

I subjoin a few examples of this kind, with field notes (of the two last), plans, and descriptions of the mode adopted in making the survey, and the reasons for selecting and arranging the several lines.

EXAMPLE 1.

In this case, there are two contiguous fields irregularly bounded.



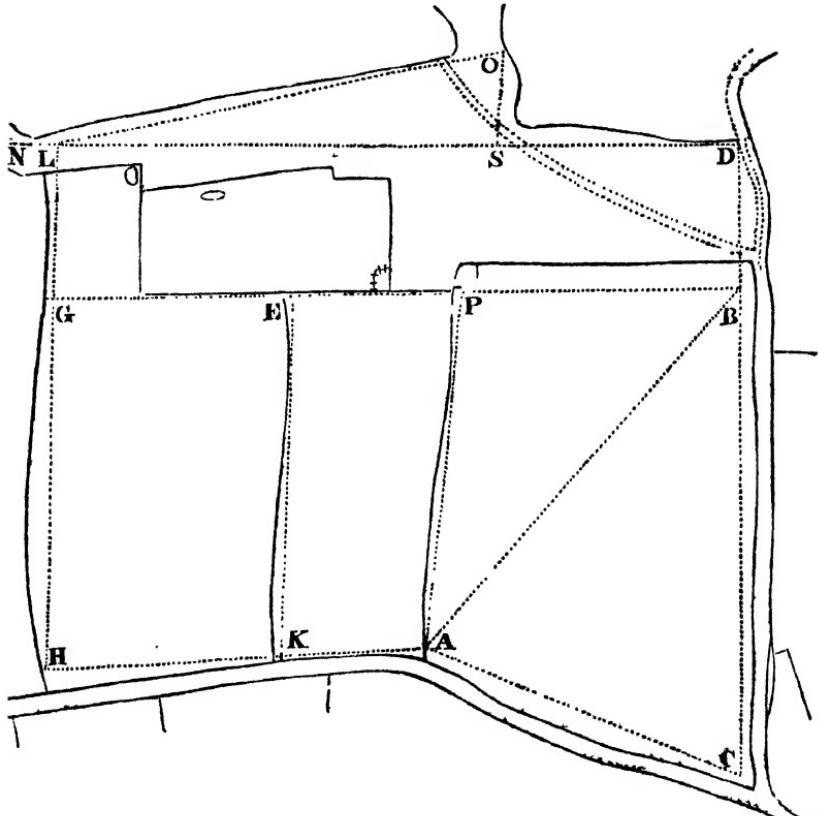
Let the base AB (which may be taken arbitrarily) pass in a diagonal direction through the corners of the larger field.

By basing upon AB, the triangles ACB, ADB, you obtain the position of the three hedges of the larger field, one of which is common to the adjoining field.

By basing upon AH, the triangle AEH, the whole of the second field is measured, and the side EA is a fixed line; its production to K is also fixed; and, as C is fixed, being the vertex of the triangle ACB, the line KC, without measurement, is determined; when measured, *which it must be for the sake of the offsets to the river, its measurement is a base of verification to the whole.*

Check-lines had better be taken in each of the triangles, for the sake of correctness. These, from the absence of offsets, are soon measured, and add very little to the trouble or time of the survey.

EXAMPLE 2.

This is a case of four or five adjoining fields. Take a base line AB; upon AB, describe the triangle ACB; produce CB to D. 

Upon the other side of AB, describe the triangle APB; produce BP to G.

The imaginary line AG, being a fixed line, describe upon it the triangle AHG; produce HG to L; join DL, and produce it to N; and upon LS, a portion of the line DN, describe the triangle LOS. By taking proper offsets along the several lines, all the fields will be surveyed.

The methods above given, the student must bear in mind, have been supposed *free* from those *local obstructions* which will often materially change the arrangement. He must keep in view the difference he will find in laying down a method of survey, *when all the plan is before him, and he has a bird's eye view of the whole, which is supposed to be perfectly flat and free from every obstruction*; and, when, ignorant of the locality, unable to comprehend its content at a glance, he has also to contend with the local difficulties of hills, vallies, woods, rivers, houses, &c.; in fact, of all that constitutes, in all cases, the difference between theory and practice.

NOTE.—[The various methods of overcoming these difficulties will be found fully explained in Part the Second.]

CHAP. VIII.

SURVEY OF A PORTION OF A PARISH.

EXAMPLE.—This example is that of a larger portion of country, comprising some 80 or 100 fields, 6 or 8 roads, a village, river, canal, &c., and a line of rail road running through a corner of it, towards the village. It is an actual survey of a portion of the county of Derby, in the parish of Willington.

A base line AB has been selected, contiguous to an intended line of railway, and running through the whole of the survey, intersecting the road HC, at C; the road GK at D; and the road MO at E, which distances are carefully marked in the field book. Where the base line crosses the hedges also, at the most favourable place for running cross lines along the hedges, stakes must be put in, and the points carefully noted, taking offsets *en route* to any hedges or corners of hedges, fences, or other objects, that may be within an offset distance.

The method actually adopted would be, in order to avoid any needlessly going over the same ground twice, to commence at A, measure AN, NVH, then HC, CV, and the fields within the block VCH, and NC.

From H, the next line measured would be HG, observing carefully where the best stations could be taken for the cross-hedges, on the same side of them, as the stations were selected in the base line.

Then measure GD and HD, observing, in measuring HD, to have the range of the line carefully defined, where the several hedges cross: so as accurately to define the several points in the line CD, where the cross hedge lines, from AB to HG, intersect. By this plan, all these cross-lines are check-lines.

Produce GD, to K, in the same straight line; measure KC, taking notice as before, where the cross hedges come, and on their proper side, and *complete* the block HGKC.

Then measure CL and LK.

Now return to C, and produce HC to F, where it intersects the base line; marking the several points P, Y, and X, upon it, and the several cross hedges.

Then from D produce HD to M; and from M, measure a line in range with EX, which produce to O; join OB; then complete the block GXMD, and the triangular piece KDM.

There now remains but the part adjoining the village.

From P, measure PRS, and join ST; then produce VR to *b*, and join *ba*; the lines *ba*, *aT*, TS, SR, Rb, will tie the whole of the houses in.

This must always be the plan adopted in the survey of a village, or farmhouse, or homestead; to confine

all the areas within one triangle, whose three sides should severally pass through the principal points of the place.

Having given the method, adopted in practice, for saving time in the survey of the plan, we will proceed to explain the nature and use of the several main lines.

The line ME, of the triangle DME, is the measure of the angle MDE; but CH, in the triangle CHD, is the measure of the opposite and equal angle CDH. Therefore the measured and the determined distance, agreeing or disagreeing, of either the side CH or ME, is a proof of the correctness or incorrectness of the angle at the vertex D.

Produce ME, the *fixed* line, to O; any points upon this production are also *fixed*. The point X, which is in a range with the road HP, is fixed; and H, being a fixed point, the lengths of HX is determined.

Its measurement becomes a line of verification to the opposite angle HMX, or (MDE, being supposed correct,) of the supplemental angle DEM: which is the angle that this new line MD makes with the base line. *The line OB, if the nature of the ground will permit its being measured, measures the opposite angle BED, and is another check upon its correctness.*

Again, MX is the measure of the angle MHX; and GD is also the measure of the same angle.

The actual distance of GD, compared with its computed or determined distance, is a check upon the correctness of the length of MX.

Having determined the correctness of these triangles, there can be no error of any moment in the *filling in*.

In fact, all the lines used for the measurement of the offsets to the cross hedges, are only so many additional check-lines to the triangles, or measures of the angles at their vertices.

EK being *determined* by the previous measurements, its measured distance is a check upon the angle CDK, and, therefore, upon the direction of the line KC, relative to the base line AB.

The correctness of the triangle CLK, is secured by the common check-line to its vertex Lc.

The triangle AHC, having in AC a portion of the base line, depends upon the correctness of the measured distances AN and NC.

A being thus a fixed point, as well as H, measure the line NH, and as H has been previously assumed correct, HN is a measure of

the angle NCH, which is the supplemental angle to the two known angles HCD, ACN ; the length VC is a check upon the distances CN and CH.

Now returning to the other parts of the survey, the line HZ, produced to the base line at F, is an additional verification of *the whole of the triangulation*.

To ensure a correct survey of the village, observe that the line MOa passes close to one side of it.

From P, drawing PRS through R, and joining ST, you have *known* lines *close to the village*, on *another side*; producing YR to a point b, such that a line ba shall pass close to the third side of the village, you surround the whole with a fixed triangle. All errors must be confined *within* this limit; and all lines, for the measurement of the streets or lanes, carried through to either of the sides of this triangle, are, as in the case of the cross hedge-lines, in the first part of the survey, virtually but so many corroborative checks of its accuracy.

CHAP. IX.

SURVEY OF FIELDS NEAR MAIDEN LANE.

PLATE No. 3 is a plan of the point of junction of Maiden Lane, with the Junction Holloway Road, and of six or seven fields in one of the corners.

The extent of the survey was settled in the first instance, and the whole of the ground carefully examined, and the arrangement of the lines, as much as circumstances might afterwards allow, predetermined.

The plan does not take in the whole of the field notes, as the roads surveyed extended much further than the page would admit of being introduced ; the notes are, however, retained, as the student will not, of course, be under similar restrictions.

FIELD NOTES OF SURVEY.

| | | | | |
|----------|--------------------|-----------------|----------|--------------------------|
| | | paling— | 6·09 | —X 13— |
| 6·34 | to 16·61 on 1728 | D— | 5·95 | X |
| 6·33 | 15+p+21+15 | | 5·90 | 2+18 ✓ |
| 6·17 | 9+p+6+15 | | 5·83 | △ |
| | D | | 5·80 | 1+d |
| 5·95 | 6+p+9+16 | △ | 6·44 | |
| | D | path— | 5·39 | —X |
| 5·70 | 2+p+24+15 | | 5·00 | 2+d |
| | D | | | |
| 5·44 | 30+15 | | 4·48 | to 487 on 10·67 |
| p+p— | 5·38 | | 4·47 | 0+d |
| p+5 | 4·90 | pond— | 0·61 | —X |
| | 28 to end of fence | | 13 | 0·60 33 |
| 4·51 | 12+waterfall | pond— | 0·31 | —X |
| p+10 | 4·38 | pond | | |
| | 8+fence+d | | 12+4 | 0·30 |
| 4·23 | 6+20 | from 578 | | |
| D— | 4·22 | | | |
| D | D | on 578 | | |
| 13 | 4·16 | | 579 | to off. 23 at 8·20 on |
| | 20 | | 4 | 19·55 (page 107) |
| D— | 4·10 | | 570 | |
| D— | 400 | | 8 | |
| D | + d | | 500 | |
| 3 | 3·90 | | +21·55 | |
| p+11(d— | 3·70 | | D+6 0·00 | |
| | + | from 0·13 | | |
| 326 | 20+15 | | on 734 | |
| p+5 | 297 | | | |
| | 20+20 | | | |
| 2·32 | 20+15 | | 7·34 | to off. 21 at 483 on 968 |
| path+10 | 200 | 30+15 | | |
| | 1·44 | 25+15 | 700 | 2+d |
| | 1·23 | 33+20 | 4·52 | 8 to G. P. |
| | D | | 4·36 | 10 to G. P. |
| | 0·94 | 28+15 to paling | 1·56 | 17 to 1031 on 1728 |
| path+9 | 0·70 | | 0·13 | |
| from 544 | | | 0·12 | 18+d |
| on 609 | | | from 578 | |
| | 3·54 | on 578 | | |
| | to 583 on 609 | | | |
| 327 | 3 | | | |
| 300 | 4 | | 5·78 | to off. 98 at 908 on |
| 2·62 | 10 to paling | | | 1728 (page 105) |
| 2·50 | 10+p+fence+8 | | 5·75 | 29+ |
| 2·39 | 58+26 to G.P. | | 5·60 | 26+pond |
| 209 | 88 to G.P. | | 5·43 | 32+pond (35) |
| 205 | 60 | | 5·25 | 40 to G. P. |
| 1·93 | 23+18 | | 300 | 18 |
| path— | 1·68 | | 2·50 | 14 |
| | 4+p+22+fence+ | | 2·10 | 10 |
| 1·65 | —+ +path | | 0·72 | 4 |
| from 860 | 1·00 | | 0·55 | 0 |
| on 1067 | 9+10+3 to paling | | 0·25 | 6 |
| | | | 0·13 | 12 |
| | | from 616 | | |
| | | on 1306 | | |

| | | | |
|--------------------------|-------|-------|---------------------|
| | | 13·06 | to offset 14 at 147 |
| 10+30 | 13·02 | | on 1955 |
| D | | | |
| 10+76 | 6·83 | | |
| | 6·16 | △ | |
| D | | | |
| 10+73 | 6·02 | 4+D | |
| — D — | 6·02 | — X | |
| | 0·08 | 8+D | |
| <u>from 1063 on 1067</u> | | | |

| | | | |
|---|--------|---------------|---|
|  | pond | | |
| | 46+p+0 | 10·67 | |
| | 15 | 10·63 | △ |
| | | 10·50 | |
| stile | 10·46 | — X 0 to post | |
| 8 | 10·00 | 6 | |
| 2 | 9·36 | | |
| P | | P | |
| 3 | 9·00 | 8 | |
| path+path | 8·74 | — X | |
| to paling 10+6 | 8·61 | | |
| △ | 8·60 | | |
| D — | 4·88 | — X | |
| | 487 | △ | |
| <u>from 1443 on 1728</u> | | | |

| | | | |
|----------------------------|------|------------------------|--|
| D+7+44+0 | 9·68 | to offset 26 at 16 ch. | |
| haystack +2 | 9·35 | on 19·55 | |
| 46 | 905 | | |
| 30 | 900 | | |
| 7 | 800 | | |
| ↑ to fence 21 | 600 | | |
| D+12 | 4·83 | | |
| 16 | 479 | | |
| 18 | 400 | | |
| 5 | 300 | | |
| 0 | 200 | | |
| D+3 | 1·46 | | |
| | 1·00 | | |
| <u>from 17·28 on 17·28</u> | | | |
| page 105 | | | |

| | | |
|-----------------------------------|----------|----------------------------|
| | | to offset 27 at 710 * |
| | | on 12·30 (p. 104) |
| 5·56 | 18+13 | * |
| 541 | 32 | |
| 400 | 26 | |
| 300 | 22 | |
| 2·70 | 8+oval | |
| 2·40 | | |
| 1·00 | 13+hedge | |
| from 765 on 11·60 page 104 | | |
| △ | 4·06 | to 100 on 1160 |
| | 4·05 | 11+p+15 |
| | 3·72 | 27 |
| | 3·26 | 7+ |
| | 2·95 | 3+p+26 |
| | 1·18 | 24 |
| | 0·93 | —x |
| path— from 574 on 577 | | |
| | ↑ | |
| fence— | 5·77 | —x |
| • | 5·74 | |
| △ | 5·67 | 3+16 ^D on fence |
| p— | 5·58 | —x 22+16 ^D |
| | 500 | 2+18 |
| (30) D— | 4·50 | 4+16 |
| D | 30 D— | 412 10+12—x |
| | | 400 12+14 |
| | | 300 9+16 |
| | | 256 30+15 |
| | | 200 41+14 |
| | | 1·00 50+12 |
| | | 0·95 342+0 |
| | | 0·80 21+18 |
| | | ·57 —x |
| p — | 0·30 | 11+10+8+12 ^P |
| D— | 0·29 | —x ^D |

| | | | |
|--------------------|-------|-----------------|----------------------|
| | | | stile 16+12+13+15 |
| from 765 on 1160 | | | |
| | | | |
| 16+3 | 11.60 | to 1728 on 1728 | |
| | 11.56 | 14+p | |
| 10+6 | 11.00 | 17+p | |
| 12+7 | 10.50 | 15+p | |
| D | | | |
| 12+10 | 10.00 | 8+2 path | |
| D | | | |
| 12+13+p | 9.53 | -x | |
| 10+11+2+7 | 900 | | |
| 12+23+p+26 | 8.00 | | |
| 10+23+p+26 | 7.90 | | |
| D | | | |
| to post 20+18+p | 7.70 | | |
| +25 | 765 | △ | |
| D | | | |
| 737 | -x | | |
| 720 | 27+D | | |
| 37 | 600 | | |
| +path+35 | 400 | | |
| +path+21 | 200 | | |
| △ | 1.00 | | |
| to post stile 16 | 0.76 | | |
| stile | 0.69 | -x | |
| 16+6 | 0.65 | | |
| R | | | |
| to g. p. 21+11 | 0.36 | | |
| to g. p. 41 | 0.17 | | |
| from 1230 on 12.30 | | | |
| lamp | △ | post | |
| | 12.30 | | |
| 76 | 12.24 | | |
| | 11.76 | 30+22 | |
| | 11.70 | 4 to pump | |
| | 10.92 | 12+20 | |
| | 10.50 | 16+13 | |
| | 9.32 | 20+9 | |
| | | | Shrubbery |
| H+9+69 | 7.77 | | |
| | 7.21 | 25+9 | |
| | 7.10 | 27 | |
| | | | |

| | | | |
|------------|-------|------|-----------------|
| | | 5.00 | R 5+30+H |
| 4+15+10+34 | R | 3.00 | 10+28+H |
| 11+10+25 | | 1.00 | R H 17+20+10 |
| | 46 | 0.58 | |
| | 11+14 | 0.51 | |
| | 34+12 | 0.14 | |

from 1026 on 1026
along High Road to Kentish Town

| | | |
|----------------------|-------|---------------------|
| to 2 o. p. 0 | 10.26 | |
| *to turnpike g. p. 7 | 10.09 | *offset 50 at 19.15 |
| to fence 15 | 9.82 | on 19.15 |
| to fence 10 | 9.78 | |
| hedge— | 9.73 | —X |
| | 9.64 | 18 to G. P. |
| | 9.58 | 15 18 |
| \ fence— | 9.49 | —X |
| | 9.46 | 2 |
| | 9.34 | 20 |

from 1728 on 1728

| | | |
|-----------|-------|-------------------------------|
| | △ | 0 |
| | 17.28 | 0 to 2 brick pier |
| | 17.14 | 11 to pier of small bridge |
| path— | 17.00 | —X |
| | 16.93 | 8 to further post |
| | 16.86 | 0 + stile |
| to tree 0 | 10.32 | |
| D— | 10.31 | —X |
| | 9.08 | 90 to G. P. |
| to tree 0 | 8.39 | |
| D— | 8.35 | —X |

from offsets 14 at
147 on 1955

| | | |
|-------------------|------|-----------------|
| | 7.53 | to 2166 on 2624 |
| | 300 | 28 to 2624 |
| from 300 on 23.46 | | |

| | |
|---------|----------------------------|
| | Holloway & Highgate Road X |
| | 23.46 0 to sign post |
| | 23.32 5 to G.P. |
| | 22.00 5 to garden gate |
| R | 21.00 10 to G.P. |
| 10+44+3 | 20.00 2+8+2 |
| Rd. — | 19.00 —X |
| | 17.90 19+14+28 |
| | 17.64 19+15+28 |
| | 17.44 20 to paling |
| | 16.66 |
| | P |
| | 16.37 7+5+8+2 |
| | R P |
| | 14.60 12+6—8+4 to G.P. |
| to G.P. | 20+30 13.95 |
| | 24+25 12.00 |
| | 10.00 27+4+8+12 / |
| | CHURCH LANE |
| | 9.50 26+4+10 |
| 25+15 | 8.92 |
| | 5.34 40+4+12 to G.P. |
| | 4.42 40+4+11 to G.P. |
| 15+4 | 3.70 —X |
| Road — | 3.00 Δ |
| | R P |
| | 1.00 7+44+4+15 |

from 2624 on 2624

NOTE.—[The two following lines not in the engraving.]

| | | |
|--------------------|-------------|--------------------|
| | 12.58 | to 159 on 159 |
| | 12.00 | 20 |
| △ | 10.16 | 40 |
| | 6.50 | 76+D |
| | 2.00 | 23+57 |
| fence — | .70 | —+ |
| to fence 4+hedge — | 0.90 | —X 90 |
| from 2166 on 2624 | | |
| | △ | |
| | 26.24 | R |
| 15+0 | 25.55 | 40+5+13 |
| R — | 25.54 | —X |
| R | | R |
| 20+16 | 24.00 | 20+5+14 |
| R | | R |
| 18+35 | 22.52 | 3+6+15 |
| Road — | 22.35 | —X |
| to G.P. | 20+36 22.34 | [from gate. |
| △ | 21.66 | 0 to 9th lamp-post |

| | | | | |
|---------------------|-----------|------------|-------------------------|----------|
| | | | | |
| — | 4+20+36+6 | 21.46 | 14 to — | on fence |
| to G. P. | 20+40+5 | 21.27 | | |
| | | 20.62 | | |
| | | 20.00 | 8+5 | |
| | | 19.09 | 16 to G.P. | |
| | 20+35+4 | 18.93 | 2+13 to G. P. | |
| R | | | | |
| 15+30+10 | 18.00 | 2+11 | | |
| Road— | 16.20 | —X | | |
| 12+30 | 15.00 | 3+12+5 | | |
| 14+20 | 13.00 | 13+4+13 | | |
| 14+10 | 10.00 | 24+5+13 | | |
| 10+9 | 9.00 | 28+4+12 | | |
| 12+7 | 8.00 | 30+6+13 | | |
| R | | R | | |
| 20+6 | 7.00 | 30+6+13 | | |
| 16+11 | 6.00 | 26+5+12 | | |
| R | | R R | | |
| 15+15 | 5.00 | 20+5+10 | | |
| R | | R R | | |
| 17+20 | 4.00 | 19+4+10 | | |
| R | | R R | | |
| D+10+26 | 3.00 | 10+10+10+D | | |
| Road— | 2.32 | —X | | |
| R | | P | | |
| D+13+30+3 | 2.00 | 10+13+D | | |
| R | | | | |
| D+28+30+10 | 1.57 | | | |
| path— | 1.56 | —X | | |
| — gate fence— | .82 | —X | 12 to stile | |
| from 19.55 on 19.55 | | | Along the Junction Hol- | |
| | | | loway Road. | |
| from 21.7 on 19.55 | 3.73 | | to 638 on 663 | |
| | | | | |
| | △ | 19.55 | 0 | |
| | | 19.26 | 25 | |
| | | 19.15 | 50 | |
| post by hedge | 47 | 19.07 | | |
| | | 19.00 | 40+20 | |
| | | 18.72 | 37 | |
| | | 18.52 | 18 | |

| | | | | |
|--|-----------------|--------|-----------------|---------------------|
| | to G. P. | 30 | 18.42 | |
| | to post | 15+20 | 17.84 | 10+10 to stone |
| | | 17+15 | 16.00 | 26 to G. P. |
| | | | 15.86 | 15+10 to G. P. |
| | | 11+15 | 15.47 | Δ 24 to fence |
| | | 7+13 | 14.00 | 20+6 |
| | | | 12.24 | 20+8 to G. P. |
| | | | 12.08 | 20+6 to G. P. |
| | | | 8.20 | 10+18 to ↑ on fence |
| | | 3+28 | 600 | |
| | | 3+25 | 500 | 10+3 |
| | | 20 | 3.00 | 10+3 |
| | | | 2.17 | Δ |
| | | 2+25 | 2.00 | 10+2 |
| | | R | | R |
| | | D+3+27 | 1.70 | 10+2+d |
| | | | 1.47 | 14 to ↑ on fence |
| | | | 1.18 | 22 to fence |
| | | D+4+18 | 1.00 | 16+6+d (20) |
| | | — | 0.00 | |
| | from 159 on 159 | | | |
| | from 159 on 159 | 2.14 | to 5.74 on 6.63 | |
| | | | R | |
| | D+0 | 1.59 | Δ 30+5 | |
| | — D | | | |
| | 6+12 | 1.53 | | |
| | to G.P. 10 | 1.31 | | |
| | to G.P. 12 | 1.15 | R | |
| | 5+8 | 1.00 | 23+5 | |
| | from 638 on 663 | | | |
| | check-line. | | | |
| | from 116 on 330 | 0.32 | to 518 on 544 | |
| | | 3.30 | to 215 on 663 | |
| | from 397 on 544 | 1.16 | Δ | |
| | hedge — | 6.63 | — X | |
| | R | | | |
| | 3+28 | 639 | 0+7 | |
| | Δ road — | 6.38 | — X | |
| | R | | | |
| | 16 | 600 | 10+6 | |
| | Δ | 574 | | |
| | 5 | 500 | 26+8 | |
| | D+0 | 460 | 26+8+d | |

| | | |
|-----------------|------|----------------------|
| 3+8 | 4'00 | 24+10 |
| 4+10 | 300 | 20+8 |
| 4+16 | 2'60 | 15+10 |
| △ | 2'15 | |
| R | | R |
| D+10+20 | 2'00 | 8+7+D |
| road | 1'00 | —X |
| R | | |
| a+33+0+3 | 0'55 | 6+D |
| from 518 on 544 | | |
| hedge of road | 5'44 | —X |
| | 5'34 | 7 to G. P. |
| △ | 5'18 | 15 to G. P. |
| R | | |
| 5+30+6 | 5'00 | 20+D |
| R | | |
| D+8+30 | 4'61 | |
| road | 4'60 | —X |
| 3+18 | 4'00 | 12+30 |
| △ | 3'97 | |
| 3+16 | 3'90 | |
| 8+15 | 3'40 | 20+30 |
| | 3'00 | 15+25 |
| to G.P. 17+18 | 2'96 | |
| to G.P. 20+20 | 2'80 | |
| 30+20 | 2'00 | 15+20 |
| | 1'01 | △ |
| 20+13 | 1'00 | 18+10 |
| R | | R |
| D+25+10 | .800 | 20+15 |
| | R | |
| D+10+0 | .00 | 33+20+D |
| from 23·41 | | being old sign post. |
| on 2341 | | in previous survey. |

NOTE.—[The commencement of this line will not, from want of room, be found inserted in the plan.]

For the assistance of the student, we will briefly go over the different selections of the lines, and the arrangement of the notes, making such special comments upon any local obstructions that may have occurred, as may enable the student to meet the same under similar circumstances.

The first line commences at a point 23·41, or 23·41, which is the end of an old line; this point is a sign post, selected as of certain reference and known position. This position is defined at starting, being at 0·00, close by the road side—that is, having the whole of the road (33) on the right side, and 0 (of the road) on the left; the 10 links on the left, and the 20 links on the right, are the width of the grass, common in country lanes, between the road and the hedge.

The station, 101, is a point required for the purpose of connecting this present new line with the line above referred to (23·41). This is, of course, not wanted for our present purpose. At 280, 20+20 to G. P. (or gate-post), at 29 18+17 to G. P. Reference is seldom made to a gateway *twice*. It is usual to mark the position of the *first* post of the gate you come to.

The station 397 is wanted for the purpose of measuring the bend in the road, the remaining distance being taken as one of the sides of the triangle. The distance 215, on the next line, being taken as the second side, and the base of the triangle being the distance 3·30, which will be found immediately after the next line, 663, viz., from 397 on 544 (330) to 215 on 663. There is a *check-line* to this, for the sake of accuracy, from 116 or 330 (·32) to 518 on 544. This gives the direction (relatively) of the line 6·63. At the end of this, the lane again bends to the left, for a distance of 1·59 chains. The position of this line is also fixed by the measured base of 2·14 chains; from the end of 159, to a point 571, on the former line 663. The road now turns quickly to the right, and runs straight for nearly a quarter of a mile, to the turnpike (19 chains 55 links). This sharp turn in the road is measured by the subsequent base line of 3 chains 73 links, from 217 on the new long line, to the station 638 on the second line 6·63.

It is upon this long line, 19 chains 55 links, that all the triangulation is based, and the relative positions of the several roads in the plan determined. The small triangles, used for the windings of the

lane, which came first in the field notes, should be plotted last in the plan. The surveying of a winding road, by means of the chain, is very inaccurate, and should, where possible, be avoided. It, however, seldom happens, that when extreme accuracy in the delineation of a road is required, that this mode of measurement becomes compulsory. One average line, through the whole of its windings, can mostly be taken, when desirable; and when an approximation, as to position (which is frequently the case), is all that is wanted, the present method is sufficiently correct.

In the taking of the offsets upon this base line 19·55, great care is requisite in selecting the most suitable. It often happens, in practice, that stations for the side triangles cannot be taken at the base line itself, but an offset points near to it, such as a post in a fence, a gate post, the corners of a fence, or any other fixed point that can at any time be referred to. For instance, the first station here selected is that of a broad arrow on the fence (offset 14 at 147), for the purpose of measuring the cross hedge.

The next is that at an offset of 23 at 8·20 chains. The third station is that of the offset 24, at 15 chains 47 links.

At 16·00, then, is the common offset of 26 to gate post; at 17·84, 35 on left to a post at the corner of the field; at 1842, 30 to first left hand gate post; at 18·52, 18 links to a post in the corner of the field to the right; at 1872, 37, on the right, to where a small garden enclosure commences; at 19·00, 40 + 20, *i.e.*, 40 up to the enclosure + 20 to the corner; at 19·07 to stile post on left, by side of hedge; at 19·15, 50 to first right hand turnpike gate post; at 19·26, 25 links to second gate post on right; at 19 55, to the further corner of turnpike.

From this corner you turn to the left, on the high junction Hol. loway Road, for a distance of 26 chains 24 links.

The *direction* of this line is obtained by measuring the line 12 chains 58 links, from the station 21·66, on the line 2624, to the end of the line 159.

The number of the offsets on this line will depend altogether upon the degree of nicety required. At 1·57 the offsets on the left hand are

R
10 + 30 + 21 and n, that is 10 up to the road, which is 30 links wide, beyond which there is a grass plot of 28, up to the ditch. At 2·00, the line is on the right 10 links from the path, and on the left 3 links from the road. At 16·20 the road X, that is, the line

crosses the road, the position of the road in the previous or subsequent offset point will define the side of the road; for instance, at the previous offset, 15 chains, the line is only 3 links from the road; at the subsequent offset there is a space of 10 links between the line and the road on the left. The side of the road, therefore, crossed at 16·20, must be the right side of the road, and you cross out of it. The station selected at 21·66 is a lamp post, being the ninth from the gate. At 22·38, the line crosses the road again, and, as at the next distance you have road on each side of the line, it now crosses *into* the road. At 25·54, the line crosses a side of the road again, and, as in the next station 25·55, the whole of the road is on the right, there being no portion of it (o) on the left, the line crosses the right side, *out* of the road. At 26·24 the line ends. The road, now taking a new direction, another line of 28 chains 46 links is taken, making an angle with the preceding, which angle is measured by the line 7·53 chains, drawn from 309 on 28·46 to 21·61, on 56·24.

There are no particular observations needed on this line; at 9 chains 50 links there is a lane, called Church Lane, to the right; the position of its corners, and their distances from the line, are carefully defined. At 16·37, and at 16·66, on either side, the hedge ceases, and the wood fence begins. Between 17·64 and 17·90 there is a house on the right, 62 links off the line; at 23·46 the station is a sign-post, at the corner, where the Junction Road comes out into the high Holloway Road.

This completes the Holloway side of Maiden Lane. We now return to the Kentish Town side, to the survey of the fields.

From offset 14, at 147 on 1955, turning to the right, a line is measured to the second brick pier of a culvert, being a distance of 17·28; on this there is an offset to a post in the corner of the hedge of the first field, viz., at 9 09, 90 to G.P.

From 17·28 another line is again measured, of 10 chains 26 links, to second gate post of turnpike, which is a known point of offset upon the line 19·55.

This completes the first triangle, which passes obliquely through three or four fields

Again, from this point 10·26, turning to the right, along the same Junction Holloway Road, but in a *contrary* direction, measure 12 chains 30 links, to a lamp post.

From the end of this line measure 11·60 to 17·28 on 17·28, to the second pier of culvert, above referred to. In this line we meet

with a path, running through the field, the position of which it is necessary to define.

Instead of saying, at 200 distances, 20 links to path, &c., I would recommend a plan (which I have adopted in this instance) of $21 + \text{path}$, or, as at 7·70, of $25 + p + 18 + 20$: the intersection of p , shows the position of it, without interfering with the system and simplicity of the arrangement.

Upon this line 11·60, there is a third triangle based, for the purpose of obtaining the position of the boundaries of the field notes; the sides of this triangle are respectively 5 chains 74 links, and 4 chains 6 links.

On this line, 577, the first offsets taken are at 30 links, being 16 links after the stile, which is 12 links wide, and 13 links from the ditch, which is 15 links across.

At 4·12, ditch crosses—and, from being on the right side of the line, runs 30 links to the left, forming, as the following offset at 4·50 shows, a small pond, whose length on the line is 31 links, and width 30 links to the left. The offsets, now, go on regularly to the end.

In the next line, 406, which brings us back again, close upon the high road, at the distance of 1·18, there is an offset of 24 to the palings; at 2·95 ($3 + p + 20$), that is, 3 to the path, which is 20 links off from the corner of a small enclosure; at 3·26, the offset of 7 links is taken to the widest part of the oval; at 3·72, you come to the end of it, which is 27 links off from the line; the position of the main fence is fixed by the two offsets, 20 and 27, and the beginning and end of the enclosure by their distances on the line; the size of the oval is known by the distance 7. At 4·05 there is an offset of 11 to the path + 15 to the corner of a garden, on the other side of the field.

The offsets are now carried up to where the triangulation was left off. Let us now, therefore, proceed with the triangulation.

The next line we come to is the line 556, which, turning to the right, runs from a known point, 765 on 1160, alongside of a thick quickset ornamental hedge, to another known point, viz., an offset 27 at 710 on 12·30; this line, therefore, acts as a *check-line*. The first offset, at 100, shows the position of the hedge; the second distance, 246, the commencement of an oval enclosure; the next, 270, shows that, at its widest part, it touches the line; at 3·00 the oval ends, and is 22 links off the line.

At 541, another half-oval begins, which, at its widest part, at the end of the line, is 18 links off, its conjugate axis being 13 links ; this point, which is 31 links from the present line, is the offset point 27, on the old line 12·30. This line completes the Field No. 6.

The next line runs from the brick pier at the corner of No. 5, along the boundary hedge, between Field No. 5 and Fields Nos. 1 and 4. It connects the point 17·28 at the pier, with another known point, at the other end of the field near Maiden Lane, being offset 26 at 16 chains, on the line 19·55. This line finishes Field No. 5.

The following line, 10·46, is a loose line, being one of the two sides of another triangle, based upon a previous line 17·28, and selected for the purpose of surveying fields Nos. 2, 3, and 4. It runs to a stile post at the corner of field No. 3.

The other side of the triangle is the line 13·06, which extends from the same stile post to a known point, viz., the offset 14 at 147, on the line 19·55, being the corner, on Maiden Lane, of field No. 2.

On the line 10·67, there are two stations taken, 487 and 8·60 ; the object of them will be explained presently. The offsets on this line are as usual, except at the end, where they differ somewhat ; for example, at 10·46, the line touches the stile post on the line, denoted by (0+post), and stile crosses at 10·50—there is an offset at 15 links, to the angle of the adjoining property. At 10·07 there is an offset to another angle in the fence.

On the line 13·06, the chief thing to be observed, is, that at 6·02 the line crosses the ditch between fields Nos. 2 and 3, and that the corner of field No. 2 is (10+73) links, to the left of the line, where it crosses, while the corner of field No. 3, is 4 links to the right. The offsets of $73+10$, $67+10$, denote, that on this side of the field the hedge is *within* the boundary of the field, as was explained above. There is but one Δ on this line, viz., 6·16.

The next line measured, 578, is from this Δ , 6·16, to a previous known point, viz., offset 90 at 908 on 1728. This line is a check upon the line 10·67, and at the same time it gives the position of the boundary between fields Nos. 3 and 2.

From 578, along the ditch, between 4 and 1, the line 734 is measured to a known offset, viz., 21 at 483 on 968. This line is a check upon the measured lengths of fields Nos. 1 and 2, as to their

sides adjoining Maiden Lane. At 156 the line is 17 links distant, from a previous point 1031, on a fixed line 1728. There are, therefore, *two lines* drawn half way across to the central, to show that it is not a *loose* line, and the *halfway* denoting that the line, in its fixed direction, is produced further.

Next, from a point on the last line (0·13), a line is measured along the boundary, between 1 and 2, to a known offset on Maiden Lane, being offset 23 at 8·20 on 19·55.

Then, from the end of 578, a line is taken between fields 3 and 4 to a previously known point (487 on 1067) and produced to 609 (denoted by the two lines half way across).

Upon this line there is a Δ (544), selected for the purpose of obtaining the winding of the old flat ditch, that runs at the bottom of the field No. 4, and another Δ at 5·83, for the corresponding boundary of No. 3.

The next line, 354, is a line connecting a point, 860 on 1067, with this last Δ 5·83, having offsets to the fence on the right.

And the last line is a line measured from the first of the Δ 's on the line 609, (1544), to a known point on the line 1728.

The offsets, on this and the preceding line, are many and somewhat complicated, but certainly intelligible (with a little trouble) to the student, who has made himself master of the preceding explanations.

The Areas of the Fields in the Survey, Plate III, are respectively as follows :—

| | A. | R. | P. |
|----------------|-----|----|-----------|
| No. 1 contains | 4. | 3. | 0. |
| 2 | 4. | 2. | 25. |
| 3 | 3. | 0. | 32. |
| 4 | 3. | 1. | 28. |
| 5 | 5. | 3. | 0. |
| 6 | 2. | 3. | 8. |
| 7 | 11. | 2. | 28. |
| Total area | 35. | 2. | <u>1.</u> |

CHAP. X.

REDUCTION OF CUSTOMARY TO STATUTE MEASURE,
and vice versa.

THE statute length of the perch is 16 and a half feet, but it is different in various counties of England.

In Devonshire and Somersetshire, the customary perch, that is, the local measure of the perch, is less—being but 15 feet.

In Cornwall, it is more, 18 feet; while in Lancashire, it increases to 21; and in Staffordshire and Cheshire it is as much as 24 feet.

This is a *lineal* difference. There is, also, in some counties of England, a *superficial* difference in the measure of an acre; an acre, in Wiltshire, containing only 120 square statute perches, instead of 160.

The Wiltshire customary acre is, therefore, one quarter less than the statute acre, and the rood one quarter less than the statute rood.

As property is frequently bought and sold by the customary measure of the county wherein it lies, the Surveyor is often called upon to reduce it from one to the other.

DIFFERENT VALUES OF THE ACRE.

The number of (statute) square yards in an acre will, of course, vary with the length of the customary perch of the county.—(An acre consisting of ten square chains or of 160 square perches.)

In the statute acre, a square perch is 272·25 square feet, and the acre, therefore, is equal to

$$\begin{aligned} 272\cdot25 \times 160 &= 43560 \text{ square feet,} \\ &= 4880 \text{ square yards.} \end{aligned}$$

In the acre of Devonshire or Somersetshire, as the square perch contains 15×15 square feet, or 225 square feet,

$$\begin{aligned} \text{the number of sq. feet} &= 225 \times 160 = 36000 \\ \text{and of yards} &= 4000 \end{aligned}$$

In Cornwall, where the perch is 18 feet,

$$\begin{aligned} 18 \times 18 &= 324 \times 160 \text{ feet} = 51840 \text{ sq. feet,} \\ &\quad \text{or } 5760 \text{ sq. yards.} \end{aligned}$$

The Lancashire perch is 21 feet long; the square perch, therefore, must contain $21 \times 21 = 441$ square feet, which will make the acre to contain 70,560 square feet, or 7840 square yards.

The customary acre in Cheshire and Staffordshire is the largest of the whole, each perch being 24 feet; the acre will consist of $24 \times 24 \times 160$ square feet, which is equal to 92160 square feet, or 10240 square yards; while the Wiltshire acre consists only of $\frac{1}{4}$ the statute acre, or 3630 square yards.

By dividing the square feet, in a customary perch, by that in the statute perch, you will obtain the measure, the customary is of the statute acre, taken as an unit; and, by multiplying statute square links by this number, you can at once bring the statute acre into the corresponding customary form:—

$\frac{225}{272.25} = .826447$; the measure of the Devonshire acre in terms of the statute acre.

$\frac{324}{272.25} = 1.19$, that of Cornwall; $\frac{441}{272.25} = 1.62$, of Lancashire.*

$\frac{576}{272.25} = 2.1157$, that of Cheshire.

$\frac{120}{160} = \frac{3}{4} = 0.75$, that of the Wiltshire acre.

DIFFERENT LENGTHS OF THE CHAIN.

Because the area of any acre $A = 10x^2$, when x equals a chain's length in feet; therefore $x = \sqrt{\frac{A}{10}}$ in sq. feet, where A is the area in sq. feet of the given acre, whether statute or customary:—

The Devonshire acre contains 36000 square feet;

$$\text{the chain } x = \sqrt{\frac{36000}{10}} = \sqrt{3600} = 60 \text{ feet.}$$

The Statute acre contains 43560 sq. feet.

$$x = \sqrt{4356} = 66 \text{ feet.}$$

The Cornwall acre contains 51840 sq. feet.

$$x = \sqrt{5184} = 72 \text{ feet.}$$

The Lancashire acre contains 70560 sq. feet.

$$x = \sqrt{7056} = 84 \text{ feet.}$$

The Cheshire and Staffordshire acre = 92160 sq. feet.

$$x = \sqrt{9216} = 96 \text{ feet.}$$

* This is the same as the Irish perch.

As the chain is divided into 100 links, whatever may be the length of the link, and, as in every case, an acre equals 10 square chains, the area of an estate in any county can be *at once* found in the customary measure of that county, by using the proper length of chain, without first calculating it, by statute measure, and then having the trouble to bring it back again to customary.

If a chain be divided into 100 links, or a in feet = 100 links ; then each link = $\frac{a}{100}$ feet = the decimal of the number of feet.

Feet. Inches.

The statute link, therefore, = .66 or 7.29 in length.

The Devonshire link = .60 or 7.20.

The Cornwall link = .72 or 8.64.

The Lancashire link = .84 or 10.08.

The Staffordshire link = .96 or 11.52.

To reduce Statute Measure to Customary, or one Customary to another.

Rule 1.—Bring the acres, roods, &c., in every case, to square perches ; multiply these by the number of square feet in the given perch to bring them into square feet (a foot being the common unit of measurement of both statute and customary measure), and divide by the number of square feet in the required perch ; this will bring it into perches ; raise these perches to roods and acres and the result is the area in acres, roods, and perches, of the customary measure required.

Or *Rule 2.*—Multiply the given area in acres by the measure, the statute perch is of the customary perch, and the product will be in customary acres, which reduce to their proper value.

Or *Rule 3.*—Bring the given area into acres and decimals, and divide by the measure the customary is of the statute acre (considered as an unit.)

EXAMPLE 1. (*by the 1st rule.*)—Reduce 25 acres, 2 roods, 26 perches, statute measure, to the customary measure (Derbyshire) of 15 feet to a perch.

| | | |
|-----|----|-----|
| A. | R. | P. |
| 25. | 2. | 24. |

| | | |
|---|--|--|
| 4 | | |
|---|--|--|

| | | |
|-----|--|--|
| 102 | | |
|-----|--|--|

| | | |
|----|--|--|
| 40 | | |
|----|--|--|

| | |
|-------------|-----------------|
| <u>4096</u> | square perches. |
|-------------|-----------------|

The square feet in a statute perch = $272\cdot25 \times 4096 = 1115136$ square feet.

$$15 \times 15 = \frac{1115166}{225} = 4956 \text{ customary perches.}$$

$$\begin{array}{r} 40 \\ \hline 4 | 4956 \\ \hline 4 \quad 123-36 \end{array}$$

30. 3. 36. Derbyshire measure.

EXAMPLE 2. (*by the 2nd rule.*)—How many Cornwall customary acres are there in 45 acres, 2 roods, and 27 perches, statute measure?

$\frac{272\cdot25}{324} = .84$ = the measure, the statute perch is of the Cornwall perch, considered as an unit.

$$\begin{array}{r} 40 | 27\cdot00 \\ 4 | 2\cdot675 \\ \hline 45\cdot66875 \times .84 = 38\cdot36175 \\ \hline 4 \\ 1\cdot44700 \\ \hline 40 \\ \hline 17\cdot88000 \end{array}$$

A. R. P.
Ans. 38. 1. 17.

EXAMPLE 3. (*by the 3d rule.*)—How many acres, in the county of Stafford, are required to make a farm of 100 statute acres?

The Staffordshire measure of an acre is 2.1157.

$$\begin{array}{r} 100\cdot000 \text{ acres} = 47\cdot426 \text{ Staffordshire acres.} \\ 2\cdot1157 \\ \hline 4 \\ 1\cdot704 \\ \hline 40 \\ \hline 28\cdot160 \end{array}$$

A. R. P.
Ans. 47. 1. 28.

To bring Customary into Statute Measure.

Reverse each of the preceding rules.

EXAMPLE 1.—How many statute acres are there in 28 acres, 3 roods, and 15 perches, of Devonshire measure?

$$\begin{array}{r} 4 | 15\cdot000 \\ 4 | 3\cdot375 \\ \hline 28\cdot84375 \text{ Devonshire acres.} \end{array}$$

The measure of the Devonshire acre = $.826447 \times 28\cdot84375 = 23\cdot8378$

$$\begin{array}{r} 4 \\ 3\cdot3512 \\ \hline 40 \\ \hline 14\cdot0480 \end{array}$$

A. R. P.
Ans. 23. 3. 14.

EXAMPLE 2.—A farm in Cornwall consists of 100 acres, of local measurement, how many statute acres are there ?

100 acres.

Cornwall acre measure = 1.19

Ans. $\frac{100}{1.19}$ acres of statute measure.

EXAMPLE 3.—In 30 acres, 3 roods, 36 perches, Derbyshire measure, how many statute acres ?

A. R. P.

Ans. 25. 2. 16.

EXAMPLE 4.—A gentleman, wishing to purchase a farm in Lancashire, which is 486 acres and 2 roods, of the statute measure, is desirous of knowing how many acres of customary measure there are in it ?

Ans. 300 acres.

EXAMPLE 5.—In Cheshire, there is a farm of 240 acres 2 roods, of statute measure ; a recent purchaser wishes, by the purchase of a portion of the adjoining land, to increase his farm to 350 customary acres : what will it cost him to do so, at the rate of £40 per statute acre ?

Ans. £20,000.

EXAMPLE 6.—A nobleman wishing to farm 400 acres of customary measure, in the county of Wiltshire, is desirous of knowing what it will cost him, at the rate of £30 per statute acre ?

Ans. £9,000.

MISCELLANEOUS EXAMPLES.

How many acres of Lancashire measure are there in 250 statute acres ?

A. R. P.

Ans. 154. 1. 11.

How many statute acres will make up a farm of 300. 2. 30. acres, of Wiltshire customary measure ?

A. R. P.

Ans. 225. 2. 2.

The base of a triangular field measures 6 chains 25 links, of a Cheshire chain, and the perpendicular, 4.84. How many statute acres are there in it ?

A. R. P.

Ans. 3. 0. 32.

A rectangular field measures, by a Lancashire chain, 12 chains 45 links, and its perpendicular breadth, 8.20 links. How many acres would a farm of the same size contain in the county of Devonshire ?

Scotch Measure.

The acre in Scotland consists, as in England, of 10 square chains, (each chain divided into 100 links,) and is reckoned in acres, roods, and *falls*, which are equivalent to the English perches; 40 falls making one rood, and 4 roods one acre. The Scotch chain, however, is 8 feet longer than the English, being 74 feet instead of 66.

The acre being 10 sq. chains $= 10 \times 74^2 = 54760$ sq. feet.

The chain or 100 links $= 74$ feet, therefore 1 link equals .74 feet $= 8\cdot88$ inches.

$$\begin{array}{r} \text{As 10 sq. chains } = 160 \text{ square perches} \\ \frac{54760 \text{ feet}}{160} = \text{one square perch} \end{array}$$

therefore one square perch or fall $= 342\cdot25$ sq. feet.

As the Scotch fall $= 342\cdot25$ sq. feet,
and the English perch $= \underline{272\cdot25}$ sq. feet,
the excess of the Scotch fall $= 70$ feet sq. feet.

And the measure of the Scotch acre, in terms of the English

$$\text{statute acre } = \frac{342\cdot25}{272\cdot25} = 1\cdot2571.$$

To bring English Statute Measure into Scotch.

Rule 1.—Reduce the given area into English perches, and then into square feet, by multiplying by 272·25, the number of square feet in an English statute perch; divide this product by the number of square feet (342·25) there are in a Scotch fall, and you obtain the area in terms of Scotch falls, which bring back to their proper quantities in roods and acres.

Rule 2.—Bring the given area into decimals of an acre; divide this by 1·2571, which is the measure of a Scotch acre, when the English acre is unity, and you obtain the decimals of a Scotch

acre, which multiply by 4 and by 40, to bring to their proper quantities.

A. R. P.

EXAMPLE 1.—Reduce 32. 3. 25. English statute measure, into Scotch measure.

By the 1st Rule,

$$\begin{array}{r} 32. 3. 25 \\ \hline 4 \\ \hline 131 \\ \hline 40 \\ \hline 5265 \end{array}$$

square perches

$$272.25 \times 5265 = 1433396 \text{ square feet in the given area}$$

$$\text{sq. ft. in a fall} = \frac{1433396}{342.25} = 4188 \text{ sq. falls}$$

$$\begin{array}{r} 40 | 4188 \\ 4 | 104.28 \\ \hline 26.0.28 \end{array}$$

A. R. P.
Ans. 26. 0. 28.

By the 2nd Rule,

$$\begin{array}{r} 40 | 25.0000 \\ 4 | 3.6550 \\ \hline 32.93625 \end{array}$$

$$\text{the measure of a Scotch acre} = \frac{32.93625}{1.2571} = 26.175$$

$$\begin{array}{r} 4 \\ \hline .704 \end{array}$$

A. R. P.
Ans. 26. 0. 28. the same as before 25.360

EXAMPLE 2.—Bring 20. 3. 39 acres, English, into Scotch measure.

How many Scotch acres are there in 100 English acres, and by how much does the Scotch exceed the English acre?

THE IRISH MEASURE is the same as the Lancashire; the chain is 84 feet long, and the acres are reckoned in acres, rods, and square perches, as in England.

The perch is 21 feet instead of 16 $\frac{1}{2}$ feet; the square perch, therefore, contains 441 square feet, and the acre 70,360 square feet or 7840 square yards.

$$\frac{441}{272.25} = 1.62 \text{ the measure of an Irish acre.}$$

As 85 feet equals 1 chain, 1 link equals .84 feet. = 10.08 inches.

The Irish chain being equal to 84 feet

and the English only to 66

In each Irish chain there is an excess of 18 feet, multiply by number of chains in a mile 80

$$3 \boxed{1440}$$

The Irish exceeds the English mile by 480 yards

English mile contains 1760 yards

Irish mile = 2240 yards

$$\frac{2240}{1760} \div 160 = \frac{14}{91} \text{ or the Irish is to the English mile, as 14 is to 11.}$$

The same rules must be adopted in this case as in that of English and Scotch measure, in the reduction of statute English to customary, and customary to statute.

EXAMPLE 1.—How many English statute acres are there in 25. 3. 19. Irish acres ?

EXAMPLE 2.—How many Irish acres must be taken to make up a farm of 100 statute acres ?

EXAMPLE 3.—A Gentleman has an estate in a rectangular form, $2\frac{1}{2}$ miles Irish one way, and $\frac{3}{4}$ the other. How many English acres are there in it, and what would be the periphery in Scotch miles of the same sized estate in Scotland ?

EXAMPLE 4.—There is an Irish farm of 120 acres, and a Scotch one of 150 acres, the former worth £10 per acre, the latter 15£. For what quantity of land, at £25 per English acre in the county of Cheshire, could they be exchanged without loss ?

GAD MEASURE.

In some places in the Country, measurements are taken by the gad pole, a staff of 8, 9, or 10 feet, indifferently divided; either into feet or into tenths.

The square gad is the space inclosed within the length and breadth of a gad.

Acres are reckoned in square gads and square feet, or in square gads and decimals.

The 8-feet gad square is 64 square feet; the 9-feet gad, 81 feet; and the 10-feet, 100.

To ascertain, in gad, the areas of lands, measured by this method, bring the length and breadth into feet or tenths, multiply these together, the product is either square feet or square tenths, or, using

the 10 as decimals, square gads; the latter is complete, the former is to be divided by the number of square feet in the given gad pole.

To bring areas in gads, and feet of customary measure, into the statute measure of acres, roods, and perches.

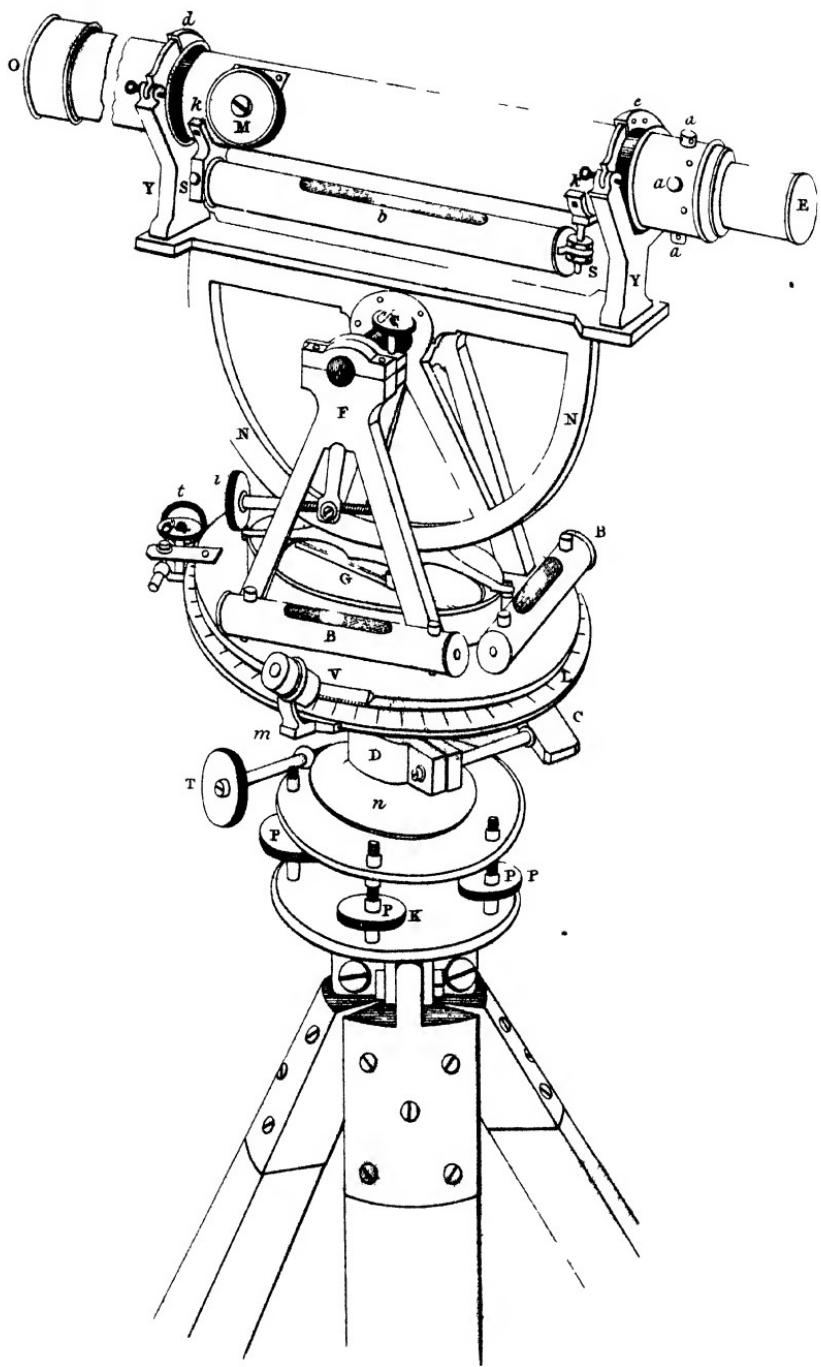
~ Bring the areas into square customary feet, divide this quantity by the square feet in a statute perch, 272·55, and the quotient is the number of statute perches in the given area, which divide by 40 and by 4, as usual, to bring it into roods and acres.

EXAMPLE 1.—A triangular field measures 28,(10 feet) gads and 5 feet, in its longest side, and in its perpendicular, 19 gads and 7 feet—What is the area in square gads, and also in statute measure?

EXAMPLE 2.—How many square gads (9 feet) are there in a rectangular field, whose length is 10·25 Irish chains, and breadth 6·29.

Simm's 5 Inch Theodolite

Plate 4



LAND SURVEYING.

Part the Second.

THE THEODOLITE.

Its Description, &c.

THE Theodolite is the most useful instrument, that has been invented, for taking horizontal and vertical angles, as, by the nature of its construction, it is not necessary, that, in the former, the objects should be in the same horizontal plane; or, in the latter, in the same vertical plane.

This instrument stands upon three legs, and consists of three divisions, and has three motions.

1st.—*The absolute horizontal motion* of the whole instrument moving upon its axis, with clamp-screw (C) to fix it, and tangent-screw (T) for fine adjustment.

2nd.—*The relative motion*. (as to the lower) of the *upper* of the two horizontal circles, to which the vernier (V) is attached, with its clamp-screw (c), and adjusting-screw (t).

These two motions are for taking horizontal angles.

3rd.—The relative motion of the vertical circle, which has also, as well as the other two, its clamp-screw (c), and fine adjusting-screw (i).

Detail of the first motion, which must be perfectly horizontal.

The lower (K) of the two parallel *Plates* is screwed tightly down to the legs of the instrument. The axis of the whole instrument passes right through to this plate. The centering, at the other end, is fixed to the upper of the two parallel *circles*—the upper one, called the *vernier circle*, from having the vernier attached to it; the lower, the graduated or horizontal limb (L), having its whole circumference graduated into degrees and half degrees. The lower circle has a distinct motion from the upper, working by means of a collar attached to it, upon the centering of the upper. Upon this is again fixed the collar (D) of the large clamp-screw (of the first division), which is attached as well as the tangent-screw to the upper of the parallel plates, which, connected with a ball, works in the socket of the lower plate, and has a double relative motion.

The upper of the parallel *plates* is made *instrumentally* parallel to the lower graduated circle; and the upper circle, (when in correct adjustment), is also parallel to them both.

Upon the upper of these circles are two small spirit levels, B, D, at right angles to each other.

By means of two pairs of conjugate screws, (PPP), which alter the relative position of the plates, the upper one can be always made level, as will be immediately seen, by the two bubbles, in the levels, being in the centre of the tube; and, once set level, the instrument, when in adjustment, will be level in any position.

By tightening the clamp-collar, and using the tangent-screw, the finest adjustment can be obtained.

The second motion.—Unclamp the horizontal circles, and the upper will move independently of the lower, or body of the instrument, with which it is connected. This motion of the upper circle, or vernier plate, as well as that of the two (or virtually the motion of the lower), will be perfectly level, if the instrument be correct and in adjustment; and the bubbles, now, as then, will be, in every position, in the centre of the tubes. This is, however, the weakest part of the instrument. The upper circle is very thin, and having an unequal and constant pressure exerted in one place, the bubbles, which, when the two circles are clamped together, are in perfect adjustment, are often very far from being so, when the upper plate is no longer held down by the clamp-screw (*c*).

The clamp (*c*), and tangent-screw (*t*), are placed along-side these circles, and have the same office as those of the first motion.

The third motion.—In this, which is a vertical motion, a graduated circle (*N*) is made to move instrumentally at right angles, with the horizontal motion plane of the instrument.

This circle moves upon its axis (*A*), which, passing through the common centre of the instrument, is supported by two shoulders or supports (*FFP*), at right angles with the vernier plate, to which this axis is made parallel.

This, like the horizontal circle, is graduated to half degrees, and, like that, by means of a vernier (*v*), supported by the compass box, is capable of being read off to minutes.

Attached to this vertical arc, above it, is placed a telescope, supported on two *Ys* or arms, in the form

of the letter Y. These Ys are virtually tangents to the tube of the telescope, which is generally furnished with bell-metal collars, ground truly cylindrical, to rest upon them. These are kept in by clips (*de*), fastened by a pin.

A long spirit-level (*b*) is fixed beneath, to the telescope, parallel to it. The upper part of the inside of the tube is ground truly circular, so that when it is nearly filled with alcohol, or any other spirit, and its axis in a level position, the fluid may recede from the centre, which will then be its upper part.

There is also a compass box (*G*), with a magnetic needle. This box is generally placed over the two circular plates, and under the vertical arc. It is occasionally used to find the bearing of any line, though it is not correct enough for that purpose. It is really only calculated for ascertaining, *roughly*, the general position, as to the north and south points of the estate.

The Working of the Theodolite.

Adjust, first, the parallel screws (*P P*), so as to have, as nearly equal lengths of the worm, as possible, above the upper plate.

Extend the three legs, approaching or extending each, until the bubbles in the two levels (*B B*) are nearly central, and the plummet, suspended from a hook under the body of the instrument, hangs freely above the centre of the station. The better plan is to move only one leg, which is, of itself, capable of a double motion.

Press the legs firmly in the ground, unclamp the

whole instrument by means of the large clamp-screw (C), observing to keep the other motions clamped.

It must now be remembered, that the two levels, on the horizontal plate, are conjugate, *i. e.*, at right angles; and, that the opposite screws, also, are conjugate, each pair of them.

Set one level in a plane, vertically parallel to a plane, passing through any one of the screws; and the other level and the other pair of screws, will be also in parallel vertical planes, conjugate to the former.

If both the bubbles of the levels, thus placed, are not in the centre, take the bubble that is not level, and, loosening one of the corresponding conjugate screws, tighten the other, until the bubble be accurately adjusted. Then loosen and tighten the other pair in the same way, till the same result be obtained. This will probably throw the first out; repeat the process to each, until both bubbles are level.

Having these plates (K k) level, clamp the whole instrument, and, unclamping the parallel circles, set the broad arrow of the vernier, which is in the upper plate, to 360° , or zero, of the larger circle, and clamp it. This must be done by the magnifying or reading glass (m), attached to the horizontal circles.

This large circle is divided into 360, and then again, subdivided into half degrees, which are numbered from left to right, and, by means of the vernier, read off to minutes.

Again, unclamp the large clamp-screw, and turn the whole instrument towards the left of the stations, between which, you are desirous of taking the angle, until you can cut the object as accurately with the intersection of the cross wires of the telescope, as can

be done by the hand. Clamp the screw (C), and slowly turning the milled-head tangent-screw (T), you can obtain all possible degree of accuracy.

Now, as the zero points of both upper and lower circles are together in the present position of the telescope, and as the lower circle is graduated from left to right, by separating the upper circle, and turning it round, till the centre of the cross wires of the telescope, which is attached to it, cut exactly the centre of the object at the second station, you obtain the angle between the two, determined by the position of the vernier and the length of the arc of the circle it has described. This can at once be read off from the plate by the broad arrow of the vernier, which will stand exactly above the number of degrees and minutes of the angle, measured between the two given objects.

When the cross wires, therefore, nearly cover the object, clamp the plates, and use the tangent-screw (*t*); and, with the magnifying glass (*m*), read off the angle, by means of the vernier. (For a description of the vernier, see page 137.)

The angle, thus read off, should always, if great accuracy be required, be read off by each of the two verniers. The common 5-inch theodolite is furnished with two, and the larger instruments have three, equidistant from each other, so that the *mean* of the readings, taken at different points on the circumference, should correct the errors of eccentricity or graduation.

In extensive surveys, other securities are adopted against these errors.

Instead of fixing the broad arrow of the vernier at the zero point of the horizontal limb, at starting,

the telescope is directed to the first station, with the broad arrow indifferently placed upon the lower plate, and its position carefully read off by the several verniers, and the *mean* taken. The difference between this mean, and that of the reading of the second station, is the measure of the required angle.

In incorrect graduation, this is perhaps the best check that can be used.

As an additional check, these angles are often repeated; that is—the angle is not taken again, by separating the upper plate and bringing the vernier back to zero, and then taking it a second time—but, without detaching the two plates after the last observation, turning the whole instrument bodily round to the first station, and, then unclamping the vernier plate, and turning it round to the second station.

The difference between this and the first reading, before starting, will be double the mean angle. Keep the two plates still together, and turn the whole round, repeating the process as before.

The difference between this third reading, and the reading at starting, will be three times the angle required.

It is requisite that the verniers should be separately marked, as A, B, C.

TO TAKE A VERTICAL ANGLE, OR AN ANGLE OF ELEVATION OR DEPRESSION.

First set the whole instrument level, as was explained before, by means of the bubbles on the vernier plate. Then bring the bubble of the telescope level (*b*) to the centre of the tube, observing whether, at the

same time, the zero point of the circular arc coincides with the zero of the vernier. This must be carefully examined by the microscope.

The instrument being thus perfectly level, when the zero point of the circle and the broad arrow are together, raise or depress the telescope, till you distinctly cut the required object with the horizontal wire, or the common intersection of the three wires. The changed relative position of the broad arrow, will give the required angle, which will be an angle of depression, if the broad arrow be found between the zero of the plate and the object-glass of the telescope, and of elevation, if beyond them.

It will be requisite, for particular accuracy, to invert the telescope in its Ys, and read off the same angle from the other end ; half the difference of these two will be the angle of error, of the vernier.

A D J U S T M E N T S.

THE TELESCOPE.

The accuracy of this instrument, in its application to the purpose of taking angles, depends altogether upon the correctness of the line of collimation. The optical axis of the telescope, which is an imaginary line, joining the centre of the object-glass and eye-glass, should pass through the point of intersection of the cross wires.

These wires are attached to a broad flange of an inner tube, within the tube of the telescope, near the eye-piece, with which it is connected by two pairs of conjugate capstan-headed screws (*a a a*), so as to admit of a double relative motion.

I.—TO ASCERTAIN, WHETHER THE LINE OF COLLIMATION
IS IN ADJUSTMENT.

Place the telescope within the Ys, and, having found some point clearly defined, which is cut by the intersection of the cross wires, turn the telescope round on its axis, and observe, whether, during its whole revolution, the centre of the wires remains the same, always covering the same point. If it does, it is in adjustment; if not, turn the telescope round on its axis, and correct for half the error, by means of the small capstan-headed screws (*a a a*), loosening one and tightening the other.

II.—WHETHER THE AXIS OF THE LEVEL IS PARALLEL
TO THE AXIS OF THE TELESCOPE.

Place the telescope on the Ys, and unclamping the large clamp-screw, set the telescope over one pair of conjugate screws, and loosen and tighten them till the level, attached to the telescope, is made perfectly level; or it may be made perfectly level by the vertical tangent-screw.

Then reverse the telescope in the Ys, if the level remains the same, this also is in adjustment; if not, correct for one half the error by the capstan-headed adjusting-screw, at the end of the level (S); and the other half by the vertical tangent-screw.

There is also a side adjustment required.

The level may not always be immediately under the telescope, but a little to the right or to the left; this must not affect the position of the bubbles, or a lateral adjustment, similar to the vertical one, is indispensable, by means of the capstan-headed screw, at the other end of the level (S).

III.—HORIZONTAL.

To mak the axis of the bubble on the vernier plate, parallel to that plate.

Let one bubble be over one pair of the circular plate screws, then the other bubble will be over the conjugate pair; make both bubbles level, turn them half round the circumference, and if the bubbles deviate from the centre, correct one half the error, by the small milled-headed screws above the levels: and the other half error, by the circular plate screws; repeat this, till the bubbles are level, in every position, throughout a whole revolution of the circumference.

IV.—HORIZONTAL.

Whether, after having duly corrected for the third adjustment, or made the "axis of the bubbles, on the vernier plate, parallel to that plate," the bubbles will remain perfectly level, during a whole revolution of the instrument upon the common axis.

Clamp the two circular plates, and unclamp the large clamp-screw; set the bubbles perfectly level, as before; when, immediately over each pair of conjugate-screws, reverse them; if they continue level, they are in adjustment; if not, the two circular plates are moving upon different axis, and are not parallel to each other. This imperfection can only be well remedied by an instrument maker.

V.—VERTICAL.

Whether the vertical arc moves in a truly vertical plane.

Set the vertical plate, or upper horizontal circle, perfectly level.

Direct the telescope to some well-defined angle of a building ; or, should there be no building convenient, suspend a string, with a plummet attached, from the top of a high pole, and, taking care that the intersection of the wires exactly cut the string, near the plummet, raise the vertical arc, observing whether the cross wires, throughout the whole of the vertical motion of the telescope, cover the vertical string ; if it does, this also is in adjustment.

As it is seldom found that two objects, whose horizontal angle is required, are exactly in the same horizontal plane, this adjustment becomes a very important one, and requires great care. Considerable error has resulted from neglect of it.

VI.—VERTICAL.

Whether the vertical vernier is in adjustment, or perfectly central.

Direct the telescope to some point of elevation, and note the angle. Reverse the telescope in its Ys, and raising the telescope to the same object, read off the same angle ; if these angles are the same, the vernier is in adjustment ; if not, correct the vernier for the error, by means of the small screw, fastening the vernier to the vertical plate, which can be loosened, and half the difference of these two angles will be the angle of error ; or, which is better, add this angle of error to every angle of elevation, when you use the end that reads off the smaller angle, and subtract the same, from that of depression, under the same circumstances.

When you read with the larger angle, subtract this angle of error from the angle of elevation, and add it for the angle of depression.

PARALLAX.

Is an error occasioned by the focus of the eye-glass not being at once, with the focus of the object-glass, in the field of the cross wires.

The existence of parallax is determined by moving the eye about, when looking through the telescope, observing whether the cross wires change their position, and are flittering and undefined.

To correct this error, first adjust the eye-glass, by means of the moveable eye-glass tube, till you can perceive the cross wire clearly defined, and sharply marked against any white object.

Then, by moving the milled-head screw (M), at the object-end of the telescope, until you obtain the proper focus, according to the distance of the object, you are enabled at once to see clearly the object, and the intersection of the wires, clearly and sharply defined before it.

The existence of parallax is very inconvenient, and where disregarded, has frequently been productive of serious error. It will not always be found sufficient to set the eye-glass first, and the object-glass afterwards. The setting of the object-glass, by introducing more distinct rays of light, will affect the focus of the eye-glass, and produce parallax or indistinctness of the wires, when there was none before. The eye-piece must, in this case, be adjusted again.

Generally, when once set for the day, there is no occasion for altering the *eye-glass*, but the *object-glass* will, of course, have to be altered at every change of distance of the object.

THE VERNIER.

The vernier is a contrivance for subdividing, to any extent, the smallest division in a graduated scale, varying according to the scale to be subdivided, and the extent of subdivision, but having the same principle in all cases.

The following explanation of that of the 5-inch theodolite, of Simms, which is given in the plate, and which is a very convenient and accurate little instrument, will serve for all.

In this, the lower circle is divided into half degrees or 30 minutes; and the vernier so arranged as to read off to one minute.

Twenty-nine divisions of the graduated scale, which is twenty-nine half degrees of the lower circle, are taken; and are divided, upon the vernier, into thirty divisions: now, as thirty divisions are compressed into the space of twenty-nine, each of these thirty divisions is one-thirtieth less than those of the twenty-nine: or, as the whole arc, in the graduated scale, is equal to 29 half degrees, or 870 minutes, and these are subdivided into 30 divisions, in the vernier scale, each of these subdivisions is equal to 29 minutes—therefore, if the zero point of the limb correspond to the broad arrow of the vernier, the first division line of the vernier is one minute to the right of the corresponding division on the limb; the second division on the vernier, two minutes to the right; the third, three minutes; and, so on, till the thirty divisions of the vernier, exactly coinciding with the twenty-ninth division, is just 30 minutes to the right of the thirtieth, or corresponding division in the limb, and therefore corresponds to the twenty-ninth division.

If the vernier, therefore, be moved, till its first division corresponds to the first division of the limb, the broad arrow of the vernier will be removed one minute from the zero of the limb. If the second division of the vernier be made to correspond with the second division of the limb, the broad arrow of the vernier will be two minutes removed from the zero of the limb. If the third corresponding divisions, coincide, the zero's are three minutes removed, and so on.

Hence, *to set the instrument* at any angle, of any number of minutes, between 0 and 30, and 30 and 60, move the vernier, until the broad arrow becomes in such a position, *within the required half degree*, as, that the number of the line, on the vernier, coinciding with the same numbered line on the limb, shall correspond to the number of minutes required. And, to *ascertain the number of degrees and minutes*, that there are in a given angle, observe where the broad arrow of the vernier is; if, between a full degree and a half degree, so many degrees and as many minutes, as are denoted by the number of the first division line of the vernier, (reading onwards as the degrees number,) that coincides with the corresponding division in the limb; or, if between a half degree and a whole one, so many degrees and 30 minutes, *plus* the broken number of minutes, as denoted by the coincidence of the corresponding lines of the vernier and limb.

The principle of the above subdivision of the vernier of Simms's theodolite, is very simple, and is universally applicable to any subdivision; viz., divide the value of the division, in the graduated scale, by the number of divisions in the vernier, and the quotient will be the value of each of these subdivisions.

Thus, in the theodolite the graduated division is 30 minutes ; the number of vernier divisions 30 ; the instrument (29 being subdivided into 30) reads to one minute.

In the sextant, the graduated division is 10 minutes, or 600 seconds ; the vernier subdivisions are 60 ; the reading of the instrument is to 10 seconds, (59 of these 10 minutes are subdivided into 60.)

In the circumferentor, in this country, the common graduation of the plate is to one degree, or 60 minutes ; the vernier subdivisions are 20, making the reading of the instrument only 3 seconds, (19 of these divisions being subdivided into 20) ; in this case, nine and a half on each side of the zero being divided into 10.

Hence, the value of the graduated division being given, and the extent of subdivision of reading required, to ascertain the number of requisite subdivisions.

By the rule above, where $x + 1$ = the required number of the subdivisions in the vernier, and x that of the graduated arc ; V , the value of the graduated division ; and v , that of the required subdivision, we have

$$\frac{V}{x+1} = v, \text{ or } x = \frac{V-v}{v}$$

thus in the sextant $V=10$ minutes or 600 seconds ; $v=10$ seconds

$$x = \frac{600-10}{10} = \frac{590}{10} = 59 \text{ divisions.}$$

Again, if, in the circumferentor, $V = 1$ degree or 60 minutes ; and $v = 3$ minutes,

$$x = \frac{60-3}{3} = \frac{57}{3} = 19 \text{ divisions of the plate, to be subdivided into } x+1, \text{ or } 20.$$

CHAP. II.

THE THEODOLITE.

Its Application..

HAVING given an explanation of the nature, adjustment, and method of using this Instrument, we will now proceed to show its application to the purposes of surveying.

It is an instrument calculated for extreme accuracy. It is an instrument that should always be used, when, as in the Ordinance Survey, quality and not quantity is the desideratum; when the correctness of the result, and not the rapidity of execution, is the object.

I would here beg to caution my young readers against falling in this profession, as is often the case in many others, into a mere system of exclusively advocating this or that particular system of surveying, whether of the Chain, Theodolite, Sextant, or Circumferentor; they are all useful in their way, and each, under certain circumstances, has the advantage; and there is scarcely any survey, of any extent, but what all of the three may advantageously be brought into use.

In broad and extensive flats, though the triangulation were better carried on by the chain, as the chain is indispensable for determining the cross hedges, yet the long lines of the triangulation must be run in by the theodolite; the common method of ranging by the poles would not be sufficiently accurate.

In broken and hilly countries, where the chaining could only be obtained by an application of the angles, taken by the theodolite, to the determining of the comparative lengths of the hypothenusal to the horizontal lines, this instrument is indispensable.

The correct length of one side of a triangle, together with the measures to minutes, half-minutes, obtained with the accuracy of which a good theodolite is susceptible, of its two adjacent angles, will always more certainly determine the position of a third point, when hills intervene, than the incorrectly measured distances of the two other lines. And, in both the above cases, the circumferentor,* or the sextant, under certain limitations, can be advantageously introduced for the filling in.

There are two methods of using this instrument, generally adopted; the first by the *needle*: the second by the *back angle*.

The first, (*by the needle*,) I will briefly describe—The broad arrow of the vernier, and the zero point of the horizontal limb, are, by means of the adjusting-screw, made carefully to coincide; *always with the magnifying glass*; the needle is then released, and allowed freely to play upon its agate; and the whole instrument, with the two circles, firmly clasped together, turned round until the north end of the needle coincides, *as nearly as the eye can tell*, to the north point or zero *in the graduated circle in the compass box*. The whole is then clamped, and if in clamping, any error has arisen, it is carefully corrected by the large adjusting-screw (Σ). Now, if the two plates be detached, and the vernier plate turned round to the object, the angle read by the vernier, will be the angle made at the station, between the first object and the north end of the needle.

If the vernier plate be again unclamped, and turned round to the second object, the vernier will, in this case, also denote the angle made between this second station and the same north point.

* The third part will be devoted to the uses and application of this instrument.

The difference between the first and second reading will be the measure of the angle required.

This is the method of taking an angle *by the needle*.

It is, however, a method that I consider *very objectionable*. It is after all only an imperfect circumferentor. The graduation of the inner plate of the compass is too contracted, the diameter too small, and the needle too clumsy, to admit of any accuracy in the setting of it in the first instance.

I should certainly advise its being used but sparingly. The only case, when it may be used with advantages, is, in the surveying of roads, as the angle made at the commencement of a line, between the other end and the north point, can always be checked by the angle at the end of the line, made between the commencement and the south point.

The second method, *by the back angle*.—In this case, the instrument is placed at the second station, the bearing between the first and second being assumed as a base line, determined in position, and the angles are all based upon that line; thus, supposing the starting point to be A, the first station B, the base line AB, and the several stations C, D, E, F, &c., then the angle at B is that between A and C; the angle at C, that between B and D, and so on.

The disadvantage of this method is, that, as in the case of the division of a many-sided field into triangles, the error of each angle is not confined to itself, but is increased by the sum of the errors of all the preceding.

While, in the first method, by the *needle*, each angle is confined to itself, being the angle made between each line severally, and certain meridian lines, that are theoretically considered parallel. These meridian or North and South lines, however, depending upon so small a needle, and so confined a graduation as that of the theodolite, can scarcely be made practically parallel. For myself, I should consider, that there would be a greater risk of error in two consecutive readings of the needle, than would ever occur in the reading of the vernier, by the back angles.

And, in the first method, there are double the errors of plotting, to what there would be in the second. In the FIRST, there are the errors of the reading of the angle, which is common to the second method, and then there is the error of the non-parallelism of the needle, which is peculiar to the first.

CHAP. III.

HEIGHTS AND DISTANCES.

IN measuring a base line across a river, the following problem will be found useful.

PROBLEM I.

Let DA be the direction of the line which has been measured up to the river. It is required to ascertain the distance BA at once upon the ground; so as to continue the measurement of the line.

On the line DA, take any point, B, whence a perpendicular, BC, can be taken, which will be free from obstruction, so that BC can be accurately measured; carry the range on across the river, and at A set up a flag; on BC, take any point, C, whence A can be seen; and at C erect a perpendicular to AC, intersecting the line AD in D.

Measure BD; then, because ACD is a right angle

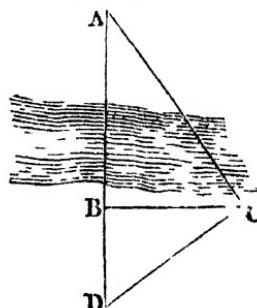
$$\frac{BC^2}{BD} = AB, \text{ (Theorem 9, page 13.)}$$

These angles can either be taken by a cross-staff or by the chain, with the distances of 30 links, 40 links, and 50 links; 50 being the hypotenuse of a right angled triangle, when the base and perpendicular are respectively 30 and 40.

EXAMPLE.—Was engaged in the measurement of a base line, that unfortunately crossed a river, too wide for the chain—measured up to the river 261 chains 45 links. Sent a man across in a boat, with a flag to carry on the range, and to plant the flag in the line on the other side. At 261 chains, at right angle with the base line on the right, measured 4 chains 50 links to the water's edge, whence only, on account of the trees near the river's side, the flag was visible.

At this point, at right angles with an imaginary line to the flag, measured to a point on the base line, which on trial I found to be 259 chains 25 links: required, the width of the river?

Width of river, 11 chains 57 links.



PROBLEM II.

To determine the area of a quadrilateral figure, whose diagonals are known, and the angle between them.

Let ABCD be the quadrilateral, having the diagonals AC and BD, and the angle AEB given. Let AEB = θ and diagonals = D and d, viz., AC = D, and BD = d.

$$\text{Now, the area of } ACB = \frac{D}{2} (EB \sin. \theta)$$

$$\text{and of } ACD = \frac{D}{2} (ED \sin. \theta)$$

$$\text{The area of the two therefore} = \frac{D \cdot \sin. \theta}{2} (EB + ED) = \frac{D \cdot d \cdot \sin. \theta}{2}$$

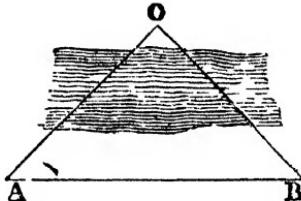
EXAMPLE.—Placed, the theodolite in the middle of a field, and taking the angle between its diagonals, 56 deg. 28 min., measured therefrom to the 4 corners, 6.80; 8.27; 5.49; 7.34 chains, what is the area of the field?

$$\begin{array}{ccc} \text{A.} & \text{R.} & \text{P.} \\ \text{Ans.} & 7. & 3. & 39. \end{array}$$

PROBLEM III.

To measure the width of a river by a base line alongside of it.

Take at either end of the base line, with a theodolite, the angle made between the base line and a flag placed at the edge on the other side of the river. Compute the length of the sides by the first of the three cases of trigonometry.



Then AO, nat. sine $\angle A = x$, the perpendicular width of the river.

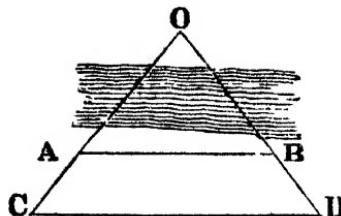
EXAMPLE.—Took a base line by the side of a river, 12 chains, and observed the angles at its ends to the flag on the other side, found them 25° and 55° : what is the perpendicular width?

$$\text{Ans. } 4.22. \text{ chs.}$$

PROBLEM IV.

To find the distance of one object from another, where a river divides them, without using the theodolite.

Let O and A be the given objects, and AO the distance required. From A draw AB, at any angle to AO, and produce OA to C, measuring AC about $\frac{1}{3}$ the length of AB; from C measure CD, parallel to AB,* and such that OBD may be in one straight line.



Then, because AB and CD are parallel, the triangles are similar, and therefore $CD : AB :: CO : AO$; and $CD - AB : AB :: CO - AO$; or $:: CA : AO \quad \therefore AO = \frac{AB \cdot CA}{CD - AB}$, the distance required.

EXAMPLE.—Took a line, 6 chains, alongside a river, and having had a flag placed on the other side, in the direction I was desirous of going, measured in a range with it from one end of the line, 4'50 chains, then took a second line parallel to the former, 8 chains, to such a point that I covered the flag and the second station of the first line.

What is the width of the river in the required direction?

Ans. 13 chains 50 links.

PROBLEM V.

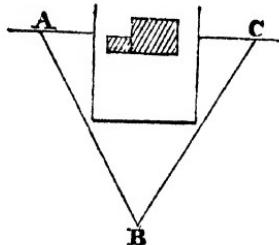
To find the distance between two places, not visible from each other, but visible and accessible from a third point.

Chain the length of the sides; take the included angle by the theodolite. The solution comes under the second of the three cases.

PROBLEM VI.

To continue the measurement and direction of a given line, when any obstacle stands in the way, which cannot be crossed, but can be avoided by going to the right or the left.

At any point (A), on the given line AC, take an angle with the given line of 120° , if you would turn to the left, or 240° , if to the right, as in this case, and proceed measuring to B, till an angle of 60° made with this line, towards the first line AC, will carry you clear of the obstacle.



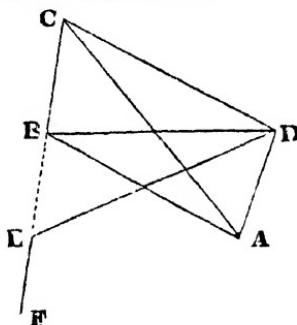
* To make CD parallel to AB: at A and B erect equal perpendiculars, which can be done by means of the unit proportion of the sides of a right angled triangle, viz., 3, 4, and 5, or any multiples of them whatever.

Take this angle, ABC, 60 degrees, and measure BC the same distance as AB; the point C shall be in the given line, and AC shall be equal to AB or BC. By taking an angle of 240 degrees with the line BC, the range of the line can be continued.

PROBLEM VII.

It is often desirable to be able to produce a given line BC, which is inaccessible and invisible at the required point of production B.

Take any stations, A and D, whence B and C are visible, and at A and D, take several angles required (see Chap. VI.), *on the determining of the length of a new base line, by angles taken from an old line,*) and determine BC, CD, and the angle BCD : through D, draw DE, at any angle with DC.



Now, because the line DC and the angle ECD have been computed, and CDE has been assumed, the triangle, DEC, comes under the first case, and the angle CED becomes determinable, as well as the required length of DE; measure this computed length to E; and at E lay off EF, making the angle DEF equal to the supplement of the computed angle CED.

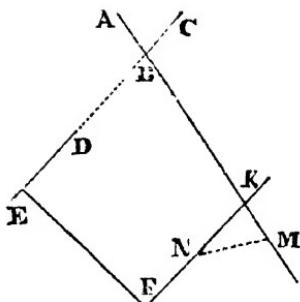
EXAMPLE.—Being desirous of continuing an inaccessible line on the opposite side of a river, I measured a base AD, of 12 chains 50 links, and took the several angles made at each end of it, between this new base and two points that were visible on the given line, viz., at A, $50^\circ, 37^\circ$; and at B, 85° , and 126° . I also measured, from the end of the new base line, nearest the river, another line, making an angle of 52° , with the new base. What must be the length of this line, and what the angle between this line and the production of the given line?

PROBLEM VIII.

To measure the angle made between two inaccessible lines, as the angle of a fort, or the salient angle of a bastion.

Let B be the salient angle of the bastion ABC, whose faces are AB and BC. It is required to measure the angle ABC.

Take any point D, out of musket shot, and produce DE in a line with the face CB. At E, erect EF perpendicular to EC, about equal to ED, and also the perpendicular FK, such that AB, the other face, and K, shall be in the same straight line; produce the direction BK, to any point M. Now, because DEF and KFE are two right angles, EB and FK are parallel, and, therefore, the outward angle FKM is equal to the inward opposite angle EBM, which is equal to the required angle ABC. Measure, therefore, the angle FKM, by the theodolite or the chain, and you obtain the measurement of the required angle.



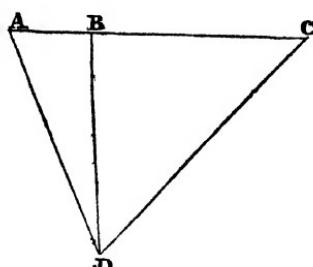
PROBLEM IX.

It sometimes happens in determining the position of two objects, (visible, but inaccessible from each other,) that both together are also inaccessible from any new station.

The only plan that can, in such cases, be adopted, is to select that station, at which the angle between them can be taken by the theodolite, and whence the most favourable lines, to clear the obstructions, can be drawn on either side, as new bases, to determine the lengths of the unmeasurable sides, that include the angle taken.

*First.—*Let B and C be the given objects, whose distance BC is required. Let D be the station, whence B and C can be seen, and the angle BDC measured; but BD and DC cannot be measured.

Now, if the stations B and C are of such a kind, and so situated, that, though AB cannot be measured, the line of direction of CB can be produced to any point A, whence D can be seen; by taking the angles at A and D, and measuring AD, you obtain the triangle ABD, and, therefore, BD and the angle ABD, which being the outward angle to the triangle BCD,



is equal to the angles BDC and BCD . BDC being known, BCD , therefore, becomes known, and BD is known; hence, the distances required become determinable, under the first of the three cases of trigonometry.

EXAMPLE 1.—Wanting to know the distance between two forts, on each side of the entrance to a harbour, I measured a base line, 35 chs. 20 links, along the beach, from a point on the beach in the produced range of the flags of the fort, 8 chains 30 links from the nearest flag, to another point, which was nearly opposite to the centre of the entrance, and found, at this second point, the angles made between this line and the two flags of the fort, to be $8^{\circ} 29' 40''$, and $45^{\circ} 11' 20''$.

Secondly.—When CB cannot be produced, it becomes necessary to consider DB and BC , as two new unknown distances required.

Select any points A and E , visible from D , and such, that B is visible from A , and C visible from E .

At D , take the angles ADB , BDC , and CDE ; measure DA and DE ; and, at A , take the angle BAD , and at E , the angle CED ,

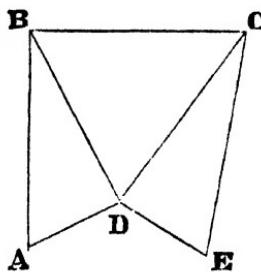
The triangles ADB and CDE , have one side and two angles given, and come under the first of the three cases.

Hence, BD and DC are determinable, and the triangle BDC , having now the two sides BD and DC , and the included angle known, comes under the second case.

EXAMPLE 2.—On another occasion, wishing to obtain the distance between two forts, similarly situated, at the mouth of a river, I found that, in consequence of the high ground in the rear, the line of direction could not be produced.

I was, therefore, under the necessity of adopting another plan. Placed my theodolite at the head of the harbour, and took the several angles, made between the flags of the fort and two new stations, whence these flags were also visible, viz., the angle ADB , 58° ; BDC , 42° ; and CDE , 72° ; measured to these stations, DA , 32 chains; DE , 35 chains; and at A , and E , found the angles to be 83° and 86° .

Ans. 1433 yds.



PROBLEM X.

Sometimes it happens, that no point can be found, whence B and C are both visible.

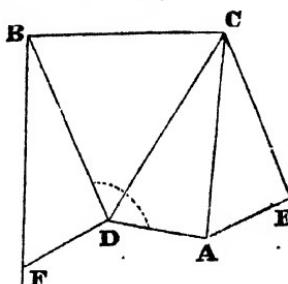
Some other arrangement, therefore, becomes necessary.

Let B and C be the unknown stations, as before, so placed that only one of them, B, is visible at D. Select any station A, whence D and C are visible ; take the angle BDA ; measure DA ; and take the angle DAC. These data, if AC were known, would give us the length of DC, and the included angle BDC, as BDC would be the difference between the measured angle BDA, and the computed angle CDA ; then, by obtaining the length of BD, the triangle BDC would have two sides, and the included angle known as before.

Find, therefore, the lengths of BD and AC, in the manner explained in the last example, considering them as two new unknown lines, by measuring DF and AE, and taking the angles BDF, BFD, and the angles CEA and CAE.

These data give you DB and CA ; in the triangle CAD, you have also DA, and the included angle DAC, and DC is determinable, and the angle CDA ; but the whole angle BDA was taken, therefore the angle BDC, the included angle of the first triangle, becomes known by computation, which, by the position of the objects, could not be taken by the theodolite.

EXAMPLE.—On a third occasion, with a similar object, could not, from a number of buildings, find any point at the head of the bay, whence both flags were visible. Took a station D, at the head of the bay, and found the angles, between the flag B and two new stations F and A (whence the flags were visible), FDB, 90° ; BDA, $125^\circ 40'$; found FD to be 10 chains; DA, 12 chains; at F found the angle DFB to be 54° ; and at A, the angles DAC, to the other flag, 79° ; and the angle CAE, to another station E, 50° ; found AE 8 chains 20 links; and then found the angle AEC to be $93^\circ 30'$. What is the distance BC ?



HEIGHTS.

PROBLEM I.

To ascertain the height of an inaccessible object.

Measure any distance from the base of the object, as nearly equal as possible to the height, and take the angle of elevation by the theodolite.

Let BA be the object; measure BC, and take the angle BCA, then BA is determinable by the case of right-angled triangles.

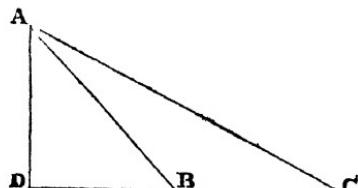
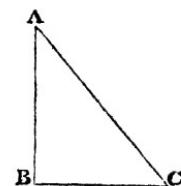
EXAMPLE 1.—What is the height of a tower, whose top, at the distance of 5 chains 75 links, subtends an angle of $33^{\circ} 17'$?

If the object be inaccessible at the base, determine the distance of any favourable station from it, by one of the methods, described in the CHAPTER ON DISTANCES, and take the angle of elevation, as in the preceding example.

Thus, measure a base line, at a convenient distance from the inaccessible object; at each end of this line, take the horizontal angle between the other end of it and the given object, and at some point within the given line, measured from either end, take the angle of elevation.

EXAMPLE 2.—*Another method may, however, be more conveniently adopted, where practicable, viz :—*

From D, take any point C, in a line with DB; measure BC; and at B and C, take the angles of elevation. The angle DBA being the outward angle, is equal to the angle at C, + the vertical angle BAC, and BC is measured; therefore the triangle ABC, in the vertical plane, comes under the first case; and AB, being thus determined, and the angle ABD, at the base, being known, the triangle ABD is



These are merely the same figures in a vertical plane, instead of a horizontal one.

determinable by the case of right-angled triangles, and AD becomes known.

EXAMPLE.—Wanted the height of a tower, and the width of a moat around it, when the angle subtended by the top of it at the edge of the moat was $64^{\circ} 20'$; and, at 4 chains 50 links off, was $25^{\circ} 54'$.

PROBLEM II.

To ascertain the height of a hill above the level of the Country, where the ground is so broken that a horizontal line cannot be measured, and the only angles, that can be taken, are from a point in the rising ground of another hill, which does not admit of measuring the horizontal distance from the object, and from the bottom of the first hill.

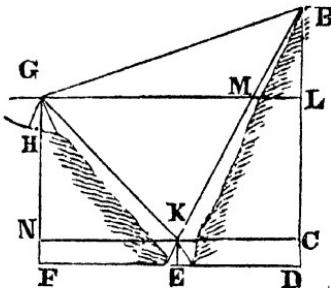
Let EBD be the hill, whose vertical height is DB. Let E and H be the two stations, for taking the vertical angles, in the same vertical plane with the object BD.

At E, take the vertical angle BKC, and from E, measure the distance HE along the slope of the hill: at H, take the angle of elevation BGL, and the angle of depression LGK, setting a flag KE, at the point E, equal to the height GH. The heights required are DB, and FG.

Now, in the triangle BKC, because the angle BKC is known, if KC or KB were known also, the triangle were determinable.

But in the triangle BGK, because $\angle BGK$ is equal to the sum of the angles of elevation and depression, and because GN is parallel to LC, the angle MKC is equal to the angle GMK, which, being the outward angle, is equal to the angles BGM and GBM; that is, the vertical angle GBM is equal to the angle of elevation at K, minus the angle of elevation at G. The angle GBM thus being known, the triangle GBK has the side GK, and two angles, known, and is therefore determinable, and KB becomes known; and, therefore BC and KC and GN, is determinable, because GK is known, and the angle GKN equals the angle of depression, MGK. Add CD, the height of the theodolite, to obtain the height of BD.

The difference between the heights of the two stations, B and H,



is found by subtracting NH from CB, their respective heights above the common *datum* line NC.

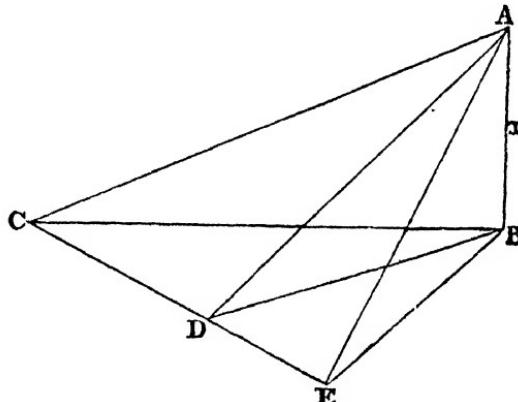
EXAMPLE.—Given at G, the angle of elevation BGL, $12^\circ 15'$, and the angle of depression LGK, $42^\circ 29'$; the hypothenusal distance GK, 8.25; and, at K, the angles of elevation NKG, $38^\circ 20'$; CKB, $64^\circ 15'$. Required the relative heights of the stations G and B.

PROBLEM III.

To determine the height of an inaccessible object by a sextant.

Select three points in a straight line, whence a distinct view of the object can be obtained, measure their distance from each other, and take the angles of elevation of the object at these several points.

Let AB = the height = x ; and the angles ACB, ADB, and AEB, or the angles of elevation = β , γ , δ , respectively, and $cd = a$, and $DE = b$; also let the angle CDB = ϕ .



$$\text{Now the cos. or angle } \text{CDB.} = \frac{a^2 + DB^2 - CB^2}{2 a \cdot DB}$$

$$\text{and} -\cos. \phi \quad \left. \right\} = \frac{b^2 + DB^2 - BE^2}{2 b \cdot DB}$$

$$\therefore b(a^2 + DB^2 - CB^2) = -a(b^2 + DB^2 - BE^2)$$

$$\text{but } DB^2 = \cot^2 \gamma \cdot x^2$$

$$CB^2 = \cot^2 \beta \cdot x^2$$

$$BE^2 = \cot^2 \delta \cdot x^2$$

$$\therefore b(a^2 + x^2(\cot^2 \gamma - \cot^2 \beta)) = -a(b^2 + x^2(\cot^2 \gamma - \cot^2 \delta)) \\ x^2(b \cot^2 \beta + a \cot^2 \delta - (a+b) \cot^2 \gamma) = -(a^2 + b^2)x^2$$

$$x^2(b \cot^2 \beta + a \cot^2 \delta - (a+b) \cot^2 \gamma) = (a+b)ab$$

$$x = \sqrt{\frac{(a+b)ab}{b \cot^2 \beta + a \cot^2 \delta - (a+b) \cot^2 \gamma}}$$

Where the height equals the root of the product into the sum, divided by the sum of each distance into cot.² of its remote angle minus the sum of the distances into the cot² of the middle angle.

When $a = b$

$$x = a \sqrt{\frac{2}{\cot^2 \beta + \cot^2 \delta - 2 \cot^2 \gamma}}.$$

EXAMPLE.—Took three stations in the same straight line, at some distance from an object, whose height was required, and, by means of a pocket sextant, took the angles of elevation $12^\circ 45'$, $14^\circ 15'$, and $18^\circ 7'$. The distances between the stations were 12 chains, and 15 chains. What is the height of the object?

CHAP. IV.

SURVEYING BY THE THEODOLITE.

HAVING given the theories of solution of most of the practical cases, that can occur in the measurement of inaccessible heights and distances, by means of the theodolite, we will now proceed to explain the measurement of a field, by this instrument, and the method of keeping the field notes.

In the survey of a field, by the theodolite, as well as in the survey of roads, as explained in Chap. II., there are likewise two methods adopted: first, by selecting one of the sides as the base, and using the back angle; and secondly, by two stations, by making the largest diagonal line the base of a number of triangles, and computing the position of the several corners, considered as vertices of triangles, whose angles at the base are determined by the theodolite, at the two stations.

The annexed field notes refer to both methods.

FIELD NOTES No. 1, (*By the back angle.*)

| | | |
|------------------------------|---------|--------------------------|
| from Δ 25·13 on 25·13 | 60·47 | to 0 on base line 2200 |
| from 13·76 on 25·13 | 9·83 | to 487 on 487 |
| from 487 | 571 | to 1848 on 26·13 |
| | | |
| from 25·13 | 487 | Δ to fence corner |
| | | R |
| 25+0 Δ | 25·13 | 45+12 |
| R | | R |
| 27+9 | 22·73 | 43+12 |
| | 18·48 | Δ |
| | 13·76 | Δ |
| R | 7 00 | 3+16 |
| 25+53+16 | 1·00 | |
| between 765 on 793 | 171°49' | and 25·13 |
| at 1200 | | |
| | 1200 | Δ |
| 17+38+16 | 11·90 | |
| R | | |
| 18+48+6 | 10·60 | |
| 10+10 | 260 | |
| | 1·50 | 20+10 |
| between 709 on 790 | 199°39' | and 1200 |
| at 705 on 793 | | |
| | 793 | 18+11 |
| | 7·65 | Δ |
| to paling 25+20 | 6·90 | |
| | 6·50 | 45 to house |
| | 610 | 38 to house |
| | 300 | 10+45 |
| | 1·00 | 33 |
| 20 | * | and 765 on 793 |
| between 697 on 707 | | |
| at 709 on 790 | | |
| | 7·90 | R |
| R | 7·09 | 6 R |
| 12+32 Δ | 5·00 | 10+35 |
| 2 | 1·50 | |
| 14+20 | | R |
| R | 1·00 | 11 |
| 40+30 | 161°11' | and 709 on 790 |
| between 11·11 on 11·11 | | |
| at 697 on 7·07 | | |
| hedge— | 7·07 | —X |
| Δ | 6·97 | 12 to end of fence |
| | 6·65 | 6+post+6 |
| 12+5 | 509 | 30 |
| 12+13 | 1·00 | 20 |
| between 844 on 8·80 | 158·28 | and 697 on 707 |
| at Δ 11·11 | | |

| | | | |
|-----------------------|---------------------|-----------|----------------|
| | 12+32 | 11·11 | 8 |
| | 12+8 | 500 | 33 |
| | R | | R |
| | 12+14 | 1·00 | 22 |
| | between 9·79 | 157·50·30 | and 11·11 |
| at 844 on 880 | | | |
| | p R | | |
| | 12+37 | 8·80 | |
| | | 8·44 | △ 8+D |
| | | 5·80 | 22+7 to G. P. |
| | | 4·80 | 20+7 to G. P. |
| | 10+27 | 2·00 | 6 |
| | p R | | R |
| | to fence 10+32 | 1·00 | 0 |
| | between 447 on 447 | 104·6 | and 844 on 880 |
| at Δ 979 | | | |
| | hedge— | 9·79 | —X |
| | road— | 9·30 | —X |
| | 21 | 9·10 | |
| | | 9·05 | 34 to wall — |
| | 11 | 8·84 | |
| (12 wide) to style 17 | | 6·93 | |
| | gate— | 6·90 | —X 20 to fence |
| | to parish stone 15 | 6·76 | |
| | | 6·40 | 27 to corner |
| | between 151 on 151 | 180·22 | and 979 |
| at Δ 447 | | | |
| | | 4·47 | |
| | other side— | 2·05 | —X |
| | fence of enclosure— | 1·30 | —X |
| between 2188 on 2200 | | 201·3 | and 447 on 447 |
| between 8·72 on 2200 | | 1°58'30" | and 2188 |
| at Δ 151 on 151 | | | |
| | D— | 1·51 | △ |
| | other side— | 1·45 | —X |
| | road and hedge— | 1·30 | —of hedge |
| | paling and road— | 1·05 | —X |
| | | 62 | —X |
| | | | |
| from 21·88 on 2200 | | | |
| | gate | 22·00 | post |
| | 9 | 21·88 | △ |
| | 49 | 18·50 | |
| | 120 | 1700 | |
| | 23 | 10·00 | |
| | to fence 4 | 8·94 | |

BASE LINE

No. 2, (By two Stations.)

| | | | | | |
|------------------|----------|------------------------|--------------------|----------|-------------------------|
| | 14.42 | to ΔG . | | 34.00 | |
| road— | 14.00 | —X | d— | 27.80 | —X |
| H— | 13.73 | —X | | 27.66 | Δ |
| to tree blazed 1 | 13.72 | | path— | 17.90 | —X |
| D— | 286 | —X | d— | 16.63 | —X Δ |
| | 264 | Δ | d— | 10.36 | —X |
| | 1.04 | to stile | — | 8 | 768 |
| from 16.63 | 23.41 | to ΔG | 10 | 600 | 0 to gate post +gate |
| sign post + | | | | 30 | 400 |
| d+16 | | | | 30 | 200 |
| R | | | | | 000 |
| 8+30+10 | 21.63 | 12 X d | to hedge 100 | | |
| | 21.06 | 21 to G.P. | from 473 on | | |
| | 2090 | 20 to G.P. | 473 | | |
| R | 1900 | 6 | producing FL | | |
| d+28x30+20 | 1800 | 0 | d+5 p+25 | 4.73 | to a point in |
| | 1700 | 6+d | d+10 | 200 | FL. |
| road+ | 1600 | | path— | 1.00 | |
| road+d | 15.40 | —X | bridge+1 | 54 | —+ |
| hedge— | 15.35 | —X | bridge | .53 | |
| hedge— | 15.10 | —X | 11+5+5 | .31 | |
| 94 | 1000 | | bank of stream | .30 | |
| 38 | 800 | | from ΔB | | |
| 28 | 700 | | producing d B | | |
| d— | 5.02 | —X | | | |
| to G.P. and 25 | 4.33 | | between ΔB | 36° 43' | and ΔL |
| d+22 | 1.00 | | at ΔF | | |
| d+21 path— | 0.83 | —X | between ΔB | 230° 10' | and ΔL |
| | 324° 00' | and ΔG | at ΔG | | |
| between 16.63 | 304° 38' | and top of St. | d+28 | 1.86 | to ΔG |
| at ΔD | | Paul's | H— | 0.20 | —X |
| | 308° 23' | and ΔC | 270° 45' | | and Δ ranging |
| | 129° 10' | and ΔF | | | with d produce |
| | 127° 55' | and $\Delta (G)$ in | | | |
| | | Maiden Lane. | d+63 | 2.69 | to ΔF |
| between 2766 | 36° 17' | and ΔE | d+63 | 1.00 | |
| at 16.63 on | 21° 10' | and ΔD | between ΔD | 275° 14' | |
| 3400 | | | at ΔC | | |
| | 350° 48' | and ΔF | | | |
| | 324° 32' | Ball of St. | | | |
| | | Paul's | | | |
| | 284° 51' | and ΔE | | | |
| | 242° 22' | and ΔD | | | |
| between 1663 | 42° 40' | and ΔC , being | | | |
| at 27.66 | | 11.97 or 1197 | | | |

N.B.—The plan of the first portion of these notes is purposely omitted.

The notes, in the first examples, are those of a survey of a road *by the back angle*. In the second example, they are those of a field surveyed *by two stations*.

The student will observe carried out, in these notes, the principle before recommended, of taking the angles always from left to right.

For instance, at station C, between station D, and station F, the angle is 275 degrees 14 minutes; that is, at the station C, the direction of F (CF), is, measuring from left to right, not only 180 degrees, but more than 270 ($275^{\circ}, 14'$), extending into the fourth quadrant of the circle. The angle, reading the other way, from right to left, might have been taken, or the instrument might have been set to F, making CF the base line. In the former case, the work would only have to be done in the field, which is now left to be done at home: in the latter, every station, making from left to right a greater angle than 180 degrees, must be made a new station; the entering of the field notes would be more complicated, and errors without number would result.

It is oftentimes requisite to take several angles at the same station, by considering the base line as fixed and constant, and determining the relative directions of the other points to this; the arrangement of the note becomes simple and clear. As, for instance, at 16·63 (see page 156), on line 3400, denotes that all the following angles (*reading upwards*) are taken *at that station*, with the theodolite unmoved. And between 27·66, on 3400, shows that the station, whence the angles are taken, is on the line between which they are measured; and the following lines, in the margin to the left, being left vacant, proves

that all the angles following are taken at the same station, 1663, and measured from the same line.

I have preferred the phraseology of ("between—and") to that of ("from — to "): there is less ambiguity about it; an angle is made *between* two lines, *at a certain point*.

The angles taken at this point are all of them, with the exception of the first, in the third or fourth quadrant. There is no possibility of error in this arrangement, and there is certainly nothing unmathematical in admitting an angle greater than 180 degrees.

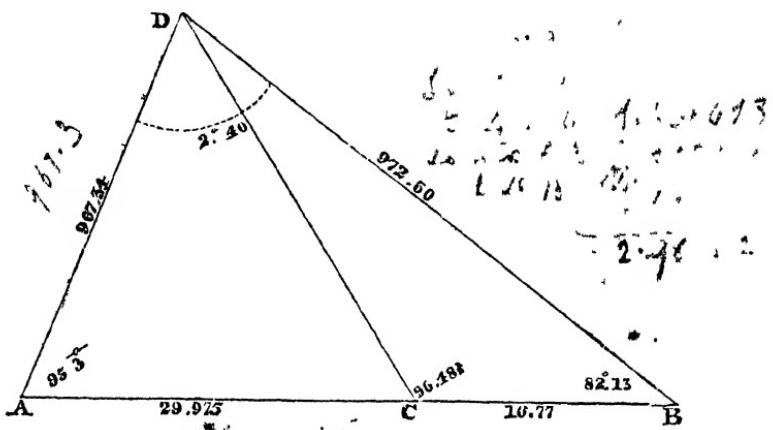
In taking several angles at a station, when it is only desired to measure along the thus ascertained direction of a new line, this angle should be taken last. and placed immediately before the chain distances. Thus, at ΔF , the angle between ΔC , and ΔG , although nearer to the base line FC ($215^{\circ} 46'$), is placed after that of ΔF ($270^{\circ} 45'$), because the following distances, 20 links, 1.86 links, are distances on the line FG (to ΔG).

The field notes No. 2, (page 156,) are those of the survey by the theodolite of the same field, which was before surveyed by the chain, the plan of which is given at p. 89. The same points are determined in this case as then ; but the offsets have been omitted as unnecessary.

I hope the student will understand, that I have not introduced the example of surveying (by the theodolite) the same field which was before surveyed by the chain, as conveying an opinion, that it is indifferent, which method is adopted under similar circumstances: far from it. I have merely given this, as an example of how it is to be done, should it be necessary. A theodolite would be quite out of place, in so small a survey as this.

FIELD NOTES, No. 8.

Being notes of a survey, actually made at Blackheath, explaining the method of keeping the field book, and the practical arrangement and computation of the angles.



Between A on base line | $83^{\circ} 10'$ and Knockholt Beeches.
at 16·77 on base line |

The base here was so disproportionate to the unknown sides, that it became necessary to take a check-angle to prevent inaccuracy.

| | | |
|-------------------|---------------------|---------------------------|
| vertical angle | $0^{\circ} 22' 0''$ | to top of Spire |
| $348^{\circ} 30'$ | $348^{\circ} 30'$ | and Charlton Church. |
| $337^{\circ} 0'$ | | |
| vertical angle | $0^{\circ} 33'$ | to Summit |
| $162^{\circ} 29'$ | $162^{\circ} 29'$ | small bush right of large |
| $324^{\circ} 58'$ | | Tree on Forest Hill. |
| vertical angle | $0^{\circ} 33'$ | to top of Spire |
| $108^{\circ} 20'$ | | |
| $216^{\circ} 38'$ | $108^{\circ} 19'$ | and Lee Church. |
| $324^{\circ} 57'$ | | |
| vertical angle | $0^{\circ} 30' 0''$ | to Summit |
| $82^{\circ} 12'$ | | |
| $164^{\circ} 26'$ | $82^{\circ} 13'$ | and Knockholt Beeches. |
| $246^{\circ} 39'$ | | |

9. 445980
1. 668915

| | | |
|--|-----------------------|---|
| vertical angle 53° 2' } 106° 5' } | 1° 30' 53° 2' 20" | to top of Spire and Blackheath New Church. |
| vertical angle 21° 29' } 42° 58' } | 1° 15' 21° 29' 20" | to top of Tower and pole at top of Seven- droog Castle. |
| between base line at Δ_B | | |
| 345° 33' 123° 37' | 345° 33' | and bush on right of large tree on Forest Hill. |
| vertical angle 325° 41' | 0° 36' | top of Spire of Church. |
| 83° 54' 202° 6' | 325° 41' 20" | and Lee Church. |
| vertical angle 284° 46' | 2° 8' 0" | to top of Spire |
| 209° 31' | 284° 45' 40" | and Blackheath New Church. |
| vertical angle 264° 57' | 0° 34' | to Summit |
| 169° 54' | 264° 57' | and Knockholt Beeches. |
| vertical angle 207° 29' | 1° 42' | to top of Turrets |
| 54° 54' | 207° 29' | and Sevendroog Castle. |
| between base line ΔA at Δ_A | | |
| | 46° 56' | to Δ_A |
| | 49° 71' | to 14° 43' on 14° 43' |
| from Δ_B the distances were | 10° 77' | Δ |

Correcting for the 2½ inches.

This chain had been previously carefully measured at the standard chain length at Somerset House, and was 2½ inches too long.

| | |
|---------------|-----------------|
| 46° 70½ | to Δ_A |
| 40° 83 | to 1443 on 1443 |
| 16° 82 | Δ |
| from Δ | |

The chain, which was used the first day, having been mislaid, another chain was used for the second measurement.

End of first day's work.

| | | |
|-------------------|---------|--------------------|
| from 1400 on 3952 | 30° 46' | to 5° 82 on 47° 81 |
| from 585 | 7° 40' | to 600 on 3952 |
| | 47° 51' | to 585 on 585 |
| | 46° 51' | Δ * |

| | | |
|---|---------------|---|
| | 38.98 5.82 | △ by side of road to △ 14.43 on 1443 |
| from △ A, by side of Shooter's Hill Road. | | |
| | 14.43 1.35 | △ to 39.52 |
| from S. E. corner of park, ranging with easterly wall. | | |
| | 38.52 | △ |
| side of — | 38.57 | —road |
| to wall 37 | 30.00 | |
| 3 | 20.00 | |
| | 1400 | △ |
| to park gate 0 | 13.98 | |
| | 600 | △ |
| | 0 | |
| | 1.96 | |
| to wall 4 | 1.27 | |
| | | along wall of park |
| | △ | |
| in a line with | 5.85 | |
| to wall 0 | 0.65 | park wall on left |
| 0.62 | | |

from S. W. corner of Greenwich Park, at Blackheath, near the Princess Augusta's.

To find the value of this check-angle by computation.

The base AB being given, 46.56, and the angles at the base $95^{\circ} 3'$, and $82^{\circ} 13'$, the other two sides were found to be 967 chains 34 links, and 972 chains 60 links.

Now, in the triangle ADC, we have the sides AD, AC, and the included angle at A; hence, by the second case of trigonometry, the other angles were computed, and that, at the base, ACD, found to be $83^{\circ} 11' 12''$, the supplement of which is $96^{\circ} 48' 48''$; the measure of the angle DCB being only 12 seconds less, than the observed angle $96^{\circ} 49'$.

The length DB, being the distance from the Knockholt Beeches to ΔB , was found to be 972 chains 60 links. To this was added the distance of B, from the S W. corner of the park, viz., $\sqrt{5.85^2 + 1.00}$, or 5 chains 93 links, and the total distance, obtained thus, was 978 chains 53 links, or 12 miles, 1 furlong, and 8 chains 53 links. The distance on the ordnance map is somewhat less than 12 miles 2 furlongs.

CHAP. V.

TRIGONOMETRICAL PROBLEMS.

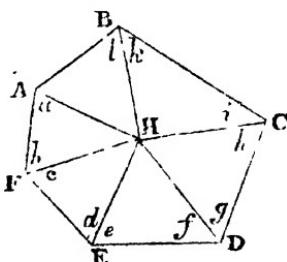
PROBLEM I.

Given the position of two known points, to determine the length of a new base line, by means of angles taken from its extremities with the given base.

(Resolved analytically.)

Before proceeding to the solution of this Problem, it must first be demonstrated, that if any polygon be taken, and lines be drawn from a point, either within or without it, to the several angles of this polygon, the continued product of the sines of one set of alternate angles, made by these lines, and the sides of the polygon, will be equal to the product of the sines of the other set.

Let H be the point when the lines HA, HB, HC, &c., are drawn to the angles of the Polygon.



Now, in the triangle AHF,

$$\frac{FH}{AH} = \frac{a}{b}; \quad \frac{EH}{FH} = \frac{c}{d}; \quad \frac{DH}{EH} = \frac{e}{f}, \text{ &c.}$$

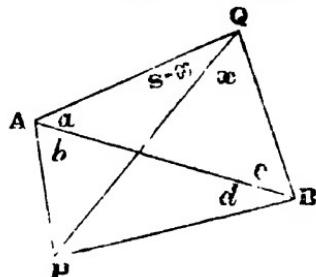
$$\frac{FH}{AH} \times \frac{EH}{FH} \times \frac{DH}{EH} \text{ &c.} \times \frac{AH}{BH} = \frac{\sin. a}{\sin. b} \times \frac{\sin. c}{\sin. d} \times \frac{\sin. e}{\sin. f} \times \frac{\sin. g}{\sin. h} \text{ &c.}$$

and $\therefore \frac{\sin.a. \sin.c. \sin.e. \&c.}{\sin.b. \sin.d. \sin.f.} = 1$, or

$$\sin.a. \sin.c. \sin.e. \sin.g. = \sin.b. \sin.d. \sin.f. \&c.,$$

The same result would obtain, where H is *without* the Polygon.

Let QP be a line determined in position; having selected two stations A and B, it is required to find their distance from each other, this distance cannot be measured, but the angles a, b, c, d , can be taken by a theodolite.



Let $s = AQB = (AQP + PQB)$ or, $180 - (a+c)$ [the other.
 $(\sin.-b. \sin.s - x. \sin.(c+d) = \text{one set}; \sin.(\overline{a+b}) \sin.x. \sin.d =$

$$\sin.b. \sin.c+d. \sin.s-x = \sin.d. \sin.a+b. \sin.x.$$

$$\sin.b. \sin.(c+d) (\sin.s. \cos.x - \sin.x. \cos.s) =$$

$$\sin.d. \sin.\overline{a+b} \sin.x;$$

$$\text{dividing by } \sin.x.] \quad \sin.b \sin.(c+d) (\sin.s \cot.x - \cos.s) = \\ \sin.d \sin.(a+b);$$

$$\sin.b \sin.(c+d). \sin.s \cot.x - \sin.b \sin.(c+d). \cos.s = \\ \sin.d \sin.\overline{a+b};$$

$$\therefore \sin.b \sin.(c+d) \sin.s \cot.x =$$

$$\sin.b \sin.(c+d) \cos.s + \sin.d \sin.\overline{a+b}.$$

getting rid of the coefficient of the $\cot.x$; we have

$$\cot.x = \cot.s + \sin.d. \sin.\overline{a+b}. \operatorname{cosec}.b. \operatorname{cosec}.c+d. \operatorname{cosec}.s.$$

This will give the value of x .

$$\text{Now } \sin.s : AB :: \sin.a : QB;$$

$$\therefore AB = \frac{\sin.s}{\sin.a} QB = \operatorname{cosec}.a. \sin.s. QB;$$

$$\text{and } \sin.(x+c+d) : QB :: \sin.(c+d) : PQ;$$

$$\therefore \sin.\frac{(x+c+d)}{\sin.(c+d)}. PQ = BQ.$$

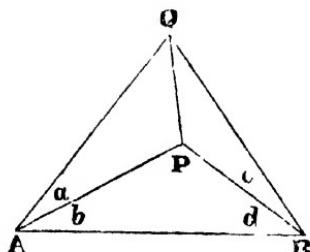
and, therefore, by substituting this value of QB , we have

$$AB = \operatorname{cosec}.a. \sin.s. \operatorname{cosec}.(c+d). \sin.(x+c+d). PQ;$$

and $\cot.x = \cot.s + \sin.d. \sin.(a+b). \operatorname{cosec}.b. \operatorname{cosec}.(c+d). \operatorname{cosec}.s$,

Now $(a+b)$ and $(c+d)$ are the angles opposite PQ , therefore,

PROBLEM. II.



If the point, P, should fall within the triangle, and it is required to find AB as before.

We shall have

$$\cot. x = \cot. s + \sin. d. \sin. a. \operatorname{cosec}. b. \operatorname{cosec}. c. \operatorname{cosec}. s.$$

when $s = 180 - (\text{the angles } QAB + QBA)$

$$\text{otherwise} \quad = \overline{180 - (a+b+c+d)}$$

$$\sin. b. \sin. s - x \sin. c = \sin. d. \sin. x. \sin. a.$$

$$\sin. b. \sin. c. (\sin. s. \cos. x - \sin. x. \cos. s.) = \sin. d. \sin. x. \sin. a.$$

$$\sin. b. \sin. c. (\sin. s. \cot. x - \cos. s) = \sin. d. \sin. a.$$

$$\sin. b. \sin. c. \sin. s. \cot. x = \sin. d. \sin. a + \sin. b. \sin. c. \cos. s.$$

$$\therefore \cot. x = \frac{\sin. b. \sin. a.}{\sin. b. \sin. c. \sin. s.} + \frac{\cos. s.}{\sin. s.} (\cot. s.)$$

$$\text{and } \therefore \cot. x = \cot. s + \sin. d. \sin. a. \operatorname{cosec}. b. \operatorname{cosec}. c. \operatorname{cosec}. s.$$

And the required $AB = \operatorname{cosec}. (a+b) \sin. s. \operatorname{cosec}. c. \sin. (x+c) = \operatorname{cosec}. (\text{opp. } \angle \text{ to } BQ), \sin. s. \operatorname{cosec}. (\angle \text{ opp. to } PQ) \sin. (x+QBP) PQ$

$$\text{For } \sin. s : AB :: \sin. (a+b) : BQ$$

$$\therefore AB = \sin. s. \operatorname{cosec}. (a+b). BQ$$

$$\text{and } \sin. c : PQ :: \sin. (x+c) : BQ$$

$$\therefore BQ = \sin. (x+c) \operatorname{cosec}. c. PQ$$

and therefore, by substituting the value of BQ, we have

$$AB = \operatorname{cosec}. (a+b) \sin. s. \operatorname{cosec}. c. \sin. (x+c) PQ$$

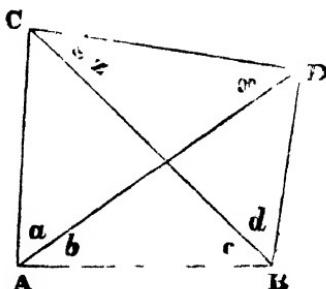
And, whether P fall within or without the triangle ABQ—

the required distance AB will always be

$$AB = PQ (\operatorname{cosec}. BAQ. \sin. AQB. \operatorname{cosec}. QBP. \sin. QPB.)$$

PROBLEM III.

Let CD be given in position, it is required to obtain a *correct base* AB, the angles a, b, c, d , being taken by the theodolite.



Now let $b+c=s=\text{CEA}=\text{EDC}+\text{ECD}$ and let $\text{EDC}=x$,
therefore $\text{DCB}=s-x$

Now

$$\begin{aligned}\sin. x. \sin. e. \sin. \text{ADB}. \sin. (s-x) &= \\ \sin. b. \sin. d. \sin. x. \sin. \text{ACB}. &\end{aligned}$$

but $\sin. \text{ADB}=\sin. \text{of supplemental } \angle (b+c+d)$.

and $\sin. \text{ACB}=\sin. \text{supplemental } \angle \sin. (a+b+c)$. (see p. 162)
 $\therefore \sin. a. \sin. c. \sin. (b+c+d). \sin. (s-x) =$

$$\sin. b. \sin. d. \sin. x. \sin. (a+b+d).$$

or, $\sin. b. \sin. c. \sin. (b+c+d). (\sin. s. \cos. x - \sin. x. \cos. s) =$
 $\sin. b. \sin. d. \sin. x. \sin. (a+b+c)$.

dividing by $\sin. x$) $\sin. a. \sin. c. \sin. (b+c+d). (\sin. s. \cot. x - \cos. s) =$
 $= \sin. b. \sin. d. \sin. (a+b+c)$.

$$\text{or } \sin. a. \sin. c. \sin. (b+c+d). \sin. s. \cot. x =$$

$\sin. b. \sin. d. \sin. (a+b+d) + \sin. a. \sin. c. \sin. (b+c+d). \cos. s$
hence, by transposition and getting rid of, as before, the coefficient of
 $\cot. x$, we have,

$$\cot. x = \cot. s + \sin. b. \sin. d. \sin. (a+b+c). \text{cosec. } a.$$

$$\times \text{cosec. } c. \text{cosec. } (b+c+d). \text{cosec. } s.$$

whence x . and $s-x$ are known.

Then as $\sin. (a+b+c) : AB : \sin. (a+b) : BC$

$$\therefore AB = \sin. (a+b+c). \text{cosec. } (a+b). BC.$$

again as $\sin. (d+s-x) : BC :: \sin. d : DC$

$$\therefore BC = \sin. (d+s-x). \text{cosec. } DC.$$

$AB = \sin. (a+b+c). \text{cosec. } (a+b). \sin. (d+s-x). \text{cosec. } (d). DC$.

$$= \text{cosec. } (a+b). \sin. (d+s-x). \text{cosec. } d. \sin. (a+b+c). DC.$$

$$\therefore AB = \text{cosec. } CAB. \sin. CDB. \text{cosec. } CBD. \sin. ACB. DC.$$

or, by using the former letters of P for D, and A for C,

$$AB = \text{cosec. } QAB. \sin. QPB. \text{cosec. } QBP. \sin. AQB. QP$$

Which is the same result as the two preceding cases, where the relative positions of the given and required bases materially differ from the present.

Hence, if the distance between two objects be correctly known, by previous surveys, and a **BASE LINE** be required, the actual measurement of which cannot be accurately depended upon, at the same time that perfect accuracy is indispensable in determining it, as it is to become the base of other triangulations, by selecting two stations, from each of which the two given objects and the other station can be seen, whatever may be the relative position of the given and required bases, the length of the *new base* can be always determined.

C H A P. V I.

PROBLEM I.

Given the position of three known points, to ascertain the position of a new station, whence those points can be seen.

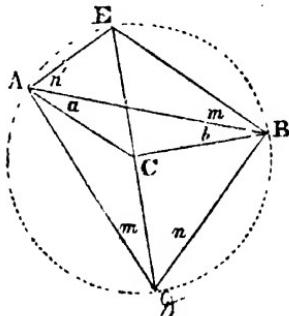
(Resolved geometrically.)

Having previously determined the position of three different places, A, B, and C, not in the same straight line, and being desirous of connecting with them a new survey, which is at some distance, but from which the three objects can be seen, it is required to determine the position of any point D, within the new survey, by means of angles taken at that point between the three given points.

Let A and B be the extreme stations; join AB, AD, and BD. Now the other given point, C, can either fall on the line AB, or within or without the triangle ABD.

1st. Let it fall within; about the triangle ABC; describe a circle, join DC, and produce to E; join AE and EB.

Let the $\angle ADE = m$, and the angle $BDE = n$, (in all the three cases,) then the angles ABE, BAE, will be also respectively equal to m and n , being upon the same segments of a circle, AE and BE.



Now, therefore, there are in the triangle AEB, the base AB, and the angles BAB and ABE, known.

AE and EB are thus determined.

Again, the sides AC, and AE, being known, and the included angle, the angle AEC can be obtained $= d$, which is also an angle of the triangle DAE, whose other angle (m) at D has been taken, and therefore the angle DAE; or, because the angle CAE, ($a+n$) is

known, their supplement becomes known, and there are, in the triangle DAC, the angles ADC, and DAC, and the side AC, given to determine the sides AD and DC; and in the triangle DCB, you have now DC and CB, and the angle CBD, (n), known.

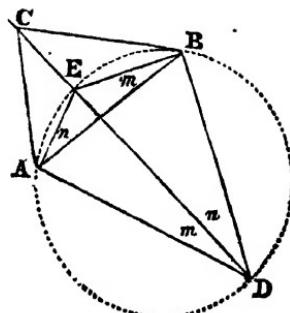
PROBLEM II.

Now let C fall without the triangle ABD.

Describe a circle as before; join AE, and BE; then the angle BAE = n , and ABE = m ; and the angle CAE = $a - n$; and the angle EBC = $b - m$.

Determine as before the sides AE, and EB, and, having the sides AB and BC known, you have two sides AC and AE, and their included angle, whence to obtain the angle ACD.

The after process is the same.

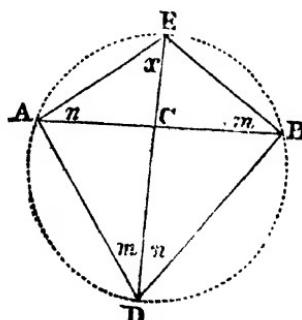


PROBLEM III.

Lastly, let A, B, and C, be in the same straight line.

Let $\angle ADC = m$, and $\angle CDB = n$, as before, and describe round ABD a circle; produce DC to E, and join AE and EB.

AE and EB are determined, and the angle E; AC also is given; therefore the two sides CA, AE, and the included angle, DAE, are known, and, therefore, the angle AEC; and in the triangle AED, the angle DAE, the supplement of the other known angles is known also; whence, in the triangle ADC, there are two angles and a side known, and also in DBC. The required distances AD, CD, BD, are thence found.



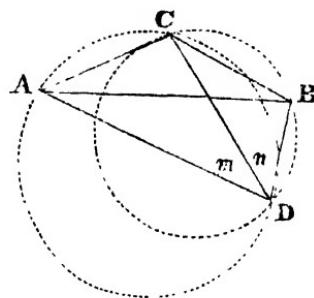
The above are examples of solution, when the student is not acquainted with analytical trigonometry.

There is an easy method of constructing the figure, and determining upon paper the position of the required station.

Let ABC be the given triangle, and D the given station, whence the angles ADC, and CDB are taken.

To find the point D.

Upon AC describe a segment of a circle, having an angle (m) equal to ADC, and upon CB the segment of a circle, having an angle (m) equal to CDB, the point of intersection of the circles shall be the station D required.



These cases may be proved analytically, if the student should prefer it, in the same way as the Problems in Chapter V.

In the first case, where AB is given, and two angles a and b .

Make $s =$ the difference between the known angles and 180, and $s - x$; and $s - x$, are the unknown angles required.

Equate the product of the sines of the alternate angles, as in Problem--, resolve the sin. ($s - x$) into its equivalent values of the simple sines.

Arrange the known and unknown values on their proper sides of the equation.

Divide by the coefficient of cot. x , and you obtain the value of one of the unknown angles. $S - x =$ the value of the other. The sides are obtained by the formulæ, for the relative values of the sides, and the sines of their opposite angles.

In the second case, where AC and CB are given, and the included angle, ACB.

Let $s = 360$ degrees—($ABC + m + n$); let the angle CBD = x , then the angle CAD will be equal to $S - x$; ED is the common base of the two triangles.

Equate the value of ED in the two triangles, thus—

$$\text{As } \sin. m : AC : \sin. s-x : DC$$

$$\text{as } \sin. n : BC : \sin. x : DC$$

$$\therefore \frac{AC, \sin. S-x}{\sin. m} = \frac{BC, \sin. x}{\sin. n}$$

Or, AC. $\sin. n. \sin. (s-x) = BC. \sin. x. \sin. m.$

AC. $\sin. n. (\sin. s. \cos. x - \cos. s. \sin. x) = BC. \sin. x. \sin. m.$

AC. $\sin. n. (\sin. s. \cot. x - \cos. s) = BC. \sin. m.$

AC. $\sin. n. \sin. s. \cot. x = BC. \sin. m. + AC. \sin. n. \cos. s.$

and $\therefore \cot. x = \frac{BC}{AC} \sin. m. \operatorname{cosec}. n. \operatorname{cosec}. s + \cot. s.$

CHAP. VII.

TRIGONOMETRICAL SURVEY.

THE following field notes are given for examples to the Student. A base line was taken on Hampstead Heath, and angles were taken between it and the Churches of Highgate, Hampstead, and St. Paul's. There was also an angle taken to a Stone Monument, at some distance on the Heath, on the ascent of a hill, whence a second angle could be taken to St. Paul's, which was not visible from both stations on the *base line*.

The distance between St. Paul's and Highgate was subsequently calculated, and this distance being assumed as an *old line*, a new line was taken at Streatham, whence angles, at each end, were taken to this old line, for the purpose of ascertaining the relative position of the new line, preparatory to determining the relative position of the two surveys at Streatham and Hampstead.

FIELD NOTES.

| | | |
|---|--|---------------------------------------|
| Between ΔP at ΔQ | $86^\circ 50' 0''$ $69^\circ 8' 27''$ | and St. Paul's and Highgate Church |
| betwn. Highgate Church ΔP | $108^\circ 27' 0''$ $19^\circ 48' 40''$ | and ΔQ and St. Paul's |

| | | |
|---------------------|--------------|---|
| fence— | 32·10 | △ e |
| side of Reservoir 5 | 4·81 | —x |
| side of Reservoir 5 | 4·42 | |
| | 0 61 | △ p near new Church, opposite Crown and Sceptre |

AT STREATHAM.

| | | |
|---|--|--|
| ∠ elevation between Δ A at Δ c | 0° 19' 96° 2' 11° 36' 35" | ball of St. Paul's and St. Paul's and Highgate Church |
| angles of { elevation elevation depression | 0° 18' 0" 3° 15' 5" 3° 15' 0" 306° 3' 40" 293° 49' 181° 51' 40" | of St. Paul's of Highgate Church of bottom of hill on b. l. and Stone and Hampstead Church and St. Paul's |
| between Δ A at Δ B | 74° 47' | and Highgate |
| between Δ B at Δ A | 278° 11' 94° 29' 81° 0' 8" | and Highgate Church and Stone and Hampstead Church |
| from Δ A | 24·74 chs. | to Δ B twice measured |

BASE LINE

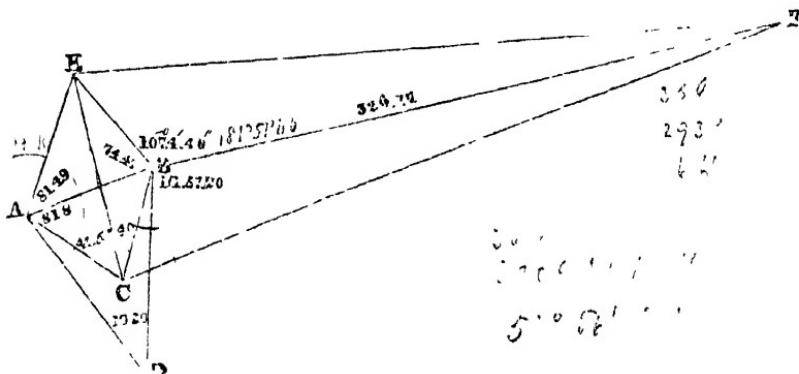
CALCULATIONS OF ANGLES TAKEN AT HAMPSTEAD.

Let AB be the base line, 24 chains 24 links long; E, Highgate Church; D, Hampstead; F, St. Paul's; and C, the Stone Monument by the road side.

At station A the following angles were taken—to the Stone, 81° 8'; to Hampstead Church, 94° 29'; computing, always, from the base line towards the right. The first angle is the angle BAC; the second is the angle BAD; the angle CAD is equal to the difference of these two, or 13° 21'; the other angle is 278° 11', or an angle in the fourth quadrant; the angle BAE is the difference between this and 360° or 81° 49'.

At station B, the first angle taken, is to Highgate Church, ABE, 74° 47'; the second angle, to St. Paul's, 181° 51' 40", being the angle ABE, 74° 47'+the angle EBF, which is therefore 107° 4' 40". The next, to Hampstead Church, 293° 49', making the angle ABD

the difference between this and 360° , or $66^\circ 11'$. The next angle is that to the Stone, which is $306^\circ 3' 40''$, making the angle ABC, $53^\circ 56' 20''$.



The angles of elevation were taken, in case of their being wanted. And, lastly, at the Stone C, the angles were taken to Highgate Church and St. Paul's, $11^\circ 36' 35''$, and $96^\circ 2'$.

The distances required, were EF, BF, and CF.

Calculations are annexed, for the sake of example, of a more accurate method of calculation, than is required in this case.

CALCULATIONS OF ANGLES TAKEN AT HAMPSTEAD.

Arrange, systematically, the work to be done, by dividing it into triangles, and apportioning the proper lines to be determined in each.

In the triangle AEB, find AE and BE.

In the triangle ACB, find AC and BC.

In the triangle CAE, find CE.

In the triangle CBE, find CE.

Their equality proves the work.

In the triangle CBF, find CF and BF.

In the triangle EBF, find EF¹.

In the triangle ECF, find EF².

EF¹ and EF² must agree.

To find BE in the triangle AEB.

$$\text{As sine } \angle E = 23^\circ 24' \quad = 9.5989523$$

$$\text{is to AB} = 24.74 \text{ chs.} \quad = 1.3933997$$

$$\text{so is sine } \angle A = 81^\circ 49' \quad = 9.9955552$$

$$11.3889549$$

$$9.9589523$$

$$\text{to BE} = 61.66 \text{ chs.} \quad = 1.7900026$$

To find AC, in the triangle ACB.

| | | |
|--------------------------------------|---|------------|
| As sine $23^\circ 24'$ | = | 9.5989523 |
| is to AB = 24.74 chs. | = | 1.3933997 |
| so is sine $\angle A = 74^\circ 47'$ | = | 9.9845004 |
| | | 11.3779001 |
| | | 9.5989523 |
| to AC = 60.1113 chs. | = | 1.7789478 |

To find BC, in the triangle ACB.

| | | |
|-------------------------------------|---|------------|
| As $\angle C = 44^\circ 55' 40''$ | = | 9.8489368 |
| is to AB = 24.74 chs. | = | 1.3933997 |
| so is sine $\angle A = 81^\circ 8'$ | = | 9.9947788 |
| | | 11.3881785 |
| | | 9.8489368 |
| BC = 34.61 chs. | = | 1.5392417 |

Again, to find AC in the triangle ACB.

| | | |
|--|---|------------|
| As sine $\angle C = 44^\circ 55' 40''$ | = | 9.8489368 |
| is to AB = 24.74 | = | 1.3933997 |
| so is $\angle B = 53^\circ 56' 20''$ | = | 9.9076207 |
| | | 11.3010204 |
| | | 9.8489358 |
| to AC = 28.3195 chs. | = | 1.4520836 |

In triangle CAE, to find the unknown angles.

| | | |
|--|---|------------|
| As sum of sides (AC + AE), 88.43 | = | 1.9465996 |
| is to their diff. = AC - AE, 31.79 | = | 1.5202905 |
| so is tan. of $\frac{1}{2}$ sum unknown \angle s E and C | | |
| or $8^\circ 31' 30''$ | = | 9.1757930 |
| | | 10.6780135 |
| | | 1.9465996 |
| to tan. of $\frac{1}{2}$ difference, $3^\circ 5' 4''$ | = | 8.7314839 |

The angles being known, to find EC.

| | | |
|---|---|------------|
| As cos. of $\frac{1}{2}$ diff. of unknown \angle s E and C or, $3^\circ 5' 4''$ | = | 9.9993704 |
| is to cos. of $\frac{1}{2}$ sum, $8^\circ 31' 30''$ | = | 9.9951749 |
| so is sum of sides (AC + AB), 88.4 chs. | = | 1.9465996 |
| | | 11.9417745 |
| | | 9.9993704 |
| to EC = 87.58 chs. | = | 1.9424041 |

In the triangle ECB, to find the unknown angles, and thence the side EC.

As sum of sides (CB+BE) 96·27 chs. = 1·9834910

is to difference (CB-BE)=27·05 chs. = 1·4321673

so is tan. of $\frac{1}{2}$ sum of unknown

$$\begin{array}{rcl} \angle s E+C = 25^\circ 38' 20'' & = & 9\cdot6812000 \\ & & 11\cdot1133673 \\ & & \underline{1\cdot9834910} \end{array}$$

to tan. $\frac{1}{2}$ diff. = $7^\circ 40' 49''$ = 9·1298763

And as cos. $\frac{1}{2}$ diff. of angles = $7^\circ 40' 49''$ = 9·9960862

is to cos. $\frac{1}{2}$ sum, $25^\circ 38' 20''$ = 9·9549845

so is sum of sides 96·27 chs. = 1·9834910

$$\begin{array}{r} 11\cdot9384755 \\ 9\cdot9960862 \end{array}$$

to EC = $87^\circ 58'$ as before = 1·9423893

In triangle CBF, to find CF and BF.

As sine $4^\circ 41' 40''$ = 8·9129747

is to 34·61 chs. = 1·5392417

so is $51^\circ 6' 20''$ = 9·8911489

$$\begin{array}{r} 11\cdot4303906 \\ 8\cdot9129749 \end{array}$$

to BF, 329·17 chs. = 2·5174157

And, as sine $4^\circ 41' 40''$ = 8·9129747

is to 34·61 chs. = 1·5392417

so is sine $124^\circ 12'$ = 9·9175478

$$\begin{array}{r} 11\cdot4567895 \\ 8\cdot9129747 \end{array}$$

to 342·80 chs. CF = 2·5438146

In triangle EBF, to find angle s.

As sum or sides = 390·83 = 2 5919879

is to diff. = 267·51 = 2·4273400

so is tan. $\frac{1}{2}$ sum $\angle s$ = $36^\circ 27' 40''$ = 9·8685922

$$\begin{array}{r} 12\cdot2959322 \\ 2\cdot5919879 \end{array}$$

to $\frac{1}{2}$ diff. = $26^\circ 49' 42''$ = 9·7039443

$\angle BEF = 63^\circ 17' 22''$

$\angle BFG = 9^\circ 37' 58''$

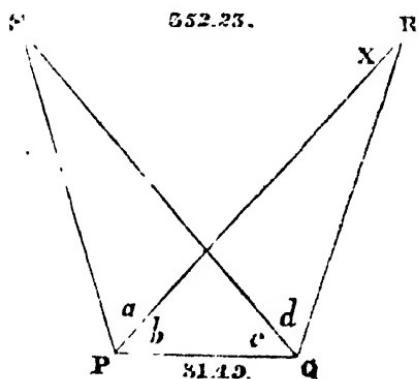
$$\begin{array}{lll}
 \text{As cos. of diff.} & 26^\circ 49' 42'' & = 9.9505413 \\
 \text{is to cos. } \frac{1}{2} \text{ sum,} & 36^\circ 27' 40'' & = 9.9053967 \\
 \text{so is sum of sides,} & 390.83 \text{ chs.} & = 2.5919879 \\
 & & \underline{12.4973846} \\
 & & 9.9505413 \\
 \text{to } 352.24 \text{ chs.} & = (\text{EF}) & = 2.5468433
 \end{array}$$

In triangle ECF, to find EF.

$$\begin{array}{lll}
 \text{As sine } 14^\circ 19' 38'' & = 9.3935204 \\
 \text{is to } 87.58 \text{ chs.} & = 1.9424041 \\
 \text{so is sine } 84^\circ 25' 29'' & = 9.9979409 \\
 & \underline{11.9403450} \\
 & 9.3935204 \\
 \text{to } 352.23 \text{ chs.} & = \text{EF} & = 2.5468236
 \end{array}$$

Calculations to determine the length and the relative position with the Hampstead Survey of the base line at Streatham.

As it was doubtful, whether, in the observation taken at station Q, the angle PQS was taken to the proper object, from the extreme fogginess of the morning, which prevented the spire of the Church (Hampstead) from being distinguished from many others in the neighbourhood, though that observation, if to the right object, was, in itself, correctly and carefully taken, it was checked by assuming the distance PQ (taking its measured distance), as known, and calculating what the angle should be, between the proper object and the given base line.



To find the angle PQS, being the angle at the point Q, made between the base line and Highgate Church.

First find PR, in the triangle PQR.

$$\begin{array}{lcl}
 \text{As sin. } 4^\circ 31' 40'' & = & 8.8973097 \\
 \text{is to } 31.49 \text{ chs.} & = & 1.4981727 \\
 \text{so is sin. } 86^\circ 50' & = & 9.9993364 \\
 & & \underline{11.4975091} \\
 & & 8.8973097 \\
 \text{to PR. } 398.29 \text{ chs.} & = & 2.6001994
 \end{array}$$

To find the angle PSR, in the triangle PRS.

$$\begin{array}{lcl}
 \text{As } 352.23 \text{ chs.} & = & 2.5468246 \\
 \text{is to sin. } 17^\circ 48' 40'' & = & 9.5300976 \\
 \text{so is PR } 398.29 \text{ chs.} & = & 2.6001994 \\
 & & \underline{12.1302970} \\
 & & 2.5468246 \\
 \text{to sin. PSR, } 22^\circ 32' 4'' & = & 9.5834724
 \end{array}$$

To find PS in the same triangle.

$$\begin{array}{lcl}
 \text{As sin. } 19^\circ 48' 40 & = & 9.5300976 \\
 \text{is to } 352.23 \text{ chs.} & = & 2.8283272 \\
 \text{so is sin. SRP, } 137^\circ 32' 16'' (42^\circ 20'.44'') & = & 9.8283972 \\
 & & \underline{12.3752218} \\
 & & 9.5300976 \\
 \text{to PS, } 700.045 \text{ chs.} & = & 2.8451242
 \end{array}$$

$$\begin{array}{lcl}
 \text{As PS+PQ, } 731.53 \text{ chs.} & = & 2.8642311 \\
 \text{is to PS-PQ, } 668.55 \text{ chs.} & = & 2.8251339 \\
 \text{so is tan. } \frac{1}{4}s = 35^\circ 46' 30'' & = & 9.8576700 \\
 & & \underline{12.6828039} \\
 & & 2.8642311 \\
 \text{to tan. } \frac{1}{4}d = 33^\circ 21' 27'' & = & 9.8185728
 \end{array}$$

$$\begin{array}{r}
 \text{the required angle } 69^\circ 8' 27'' \\
 \quad 2^\circ 34' 33'' \\
 \quad \underline{108^\circ 27' 00''} \\
 \quad 180^\circ 0' 0''
 \end{array}
 \qquad \text{which was the same as that observed by the theodolite.}$$

The following calculations were made for the purpose of verifying the correctness of the several angles taken at the two stations P and Q, by comparing the computed with the measured length of PQ.

Let $s = b + c$, where b is the angle RPQ, $88^\circ 38' 20''$, and c is the angle PQS, $69^\circ 8' 27''$; $b + c$ being, together, equal to the unknown angles QSR and SRP.

Let $\text{SRP} = x$, then QSR will be equal to $S - x$.

Now, by Problem III., Chap. V., let $s = b + c$
 $\cot. x = \cot. s + \sin. b. \sin. d. \sin. (a+b+c). \text{cosec. } a \text{ cosec. } c. \text{ cosec. } (b+c+d). \text{ cosec. } s$

$$\begin{array}{rcl} \sin. b & = & 88^\circ 38' 20'' = 9.9998774 \\ \sin. d & = & 17^\circ 41' 33'' = 9.4827426 \\ \sin (a+b+c) & = & 177^\circ 35' 27'' = 8.6236120 \\ \text{cosec. } a & = & 19^\circ 48' 40'' = 10.4699023 \\ \text{cosec. } c & = & 69^\circ 8' 27'' = 10.0294406 \\ \text{cosec. } (b+c+d) & = & 175^\circ 28' 20'' = 11.1026903 \\ \text{cosec. } = & 157^\circ 46' 47'' & = 10.4223198 \\ \text{to rad. (10 Index)} & = & \frac{70.1305849}{-70} \\ & & \end{array}$$

$$(\text{to rad. 1,}) 1.3507831 = \frac{0.1305849}{}$$

again $\cot. S = \cot. 157^\circ 46' 47''$ and being in the second quadrant
 $= -\cot. 22^\circ 13' 13''$.

$$-(\log. \cot. 22^\circ 13' 13'') = -(10.388892) \text{ to rad. 10.}$$

subtract 10

$$\begin{array}{r} (\text{to rad. 1}) - (2.4483819) = -(\overline{0.388892}) \\ \phantom{(\text{to rad. 1})} - 1.3507838 \text{ (see above)} \\ \phantom{(\text{to rad. 1})} - \overline{2.4483819} \\ \hline - (42^\circ 30') = \text{natural cot. } x = -1.0975971 \end{array}$$

$$\begin{array}{rcl} \text{rad. cot. } 42^\circ 31' & = & 1.0977020 & 5971 = \text{given angle} \\ \text{rad. cot. } 42^\circ 30' & = & \overline{1.0970609} & 0609 = 42^\circ 30' \\ & & 6411 & 5362 \\ & & & 60 \\ & & \hline 6411 & | & 311720 & | 48 \end{array}$$

$= \cot. - (42^\circ 30' 48'') = \text{its supplement } 137^\circ 39' 12'' = x$
 but $PQ = \text{cosec. SPR. sin. PSQ. cosec. PQS. sin. SRP (x)}$

viz. log. SR. 352.23 chs. = 2.5468246

log. cosec. $19^\circ 78' 40'' = 10.4699023$

log. sin. $2^\circ 21' 33'' = 8.6336120$

log. cosec. $69^\circ 82' 7'' = 10.0294405$

log. sin. $137^\circ 39' 12'' = 9.8284115$

the length of $PQ = \log. 31.4912 \text{ chs.} = 1.4981909$

CHAP. VIII.

TO REDUCE ANGLES TO THE CENTRES OF STATIONS.

IN large trigonometrical surveys, as those objects, which, from their position, are more commanding, and are furthest visible, are generally so situated, as to prevent the theodolite being placed immediately in the centre, it has been found necessary to calculate expeditiously the angle of reduction to the centre, or the difference between the angle, as taken from a station (S) near the centre of this object, and the angle from the centre.

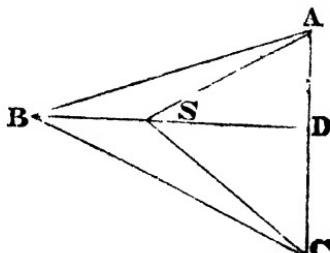
You have, for instance, determined the position of a church and tower, and taken angles to the vane of the church, or the pole of the tower, but on wishing to base upon the line connecting them a further triangulation, you find you cannot place your instrument at the centre of either, you are therefore compelled to take your angles as near to it as possible. This angle, thus taken, will fall either within this new triangle, or without, or upon one of its sides.

If it falls within, it will be greater than the true angle; if without the triangle, but within the circumscribing circle, it is greater; if upon the circumference, equal; if without it, less.

The amount of this excess or deficiency we are now to ascertain.

1st. Let it (S) fall within the triangle ABC; the angle taken will be ASC.

To find its excess, or, to reduce it to ABC, produce BS to D.



$\angle ASD = \angle ABS + \angle SAB$, $\angle CSD = \angle CBS + \angle SCB$, and
 $\angle ASC = \angle ABC + \angle BAS + \angle BCS$;
but BC, and AB, and BS are known distances, and the angle BSC is known also.

Now, in the triangle BCS,

$$\text{as } BC : \sin. BSC :: BS : \sin. \text{angle BCS},$$

therefore $\angle BCS = \frac{BS}{BC} \sin. BSC$, and for the same reason,

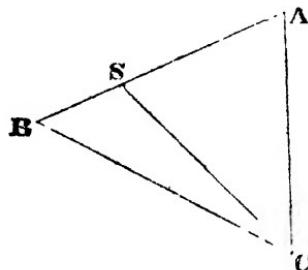
$$\angle SAB = \frac{BS}{BA} \sin. BSA, \text{ and, therefore,}$$

$$\angle ABC = \angle ASC - BS \left(\frac{\sin. BSC}{BC} + \frac{\sin. BSA}{BA} \right)$$

2nd. Let s fall upon the side AB,
then $\angle ABC = \angle ASC - BS \left(\frac{\sin. BSC}{BA} \right)$

$$+ \left(\frac{\sin. 180^\circ}{BA} = 0 \right)$$

$$\therefore \angle ABC = \angle ASC - BS \left(\frac{\sin. BSC}{BC} \right)$$

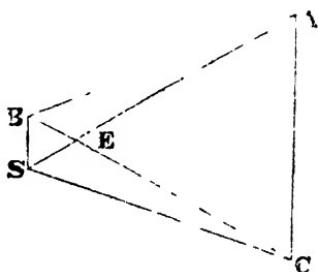


3rd. Let s fall *without* the triangle ABC, and let E be the point of intersection of AS and BC. Now the $\angle AEC = ABC + BAS = ASC + BCS$, therefore $\angle ABC = \angle ASC + \angle BCS - \angle BAS$; but

$$\angle BCS = \frac{BS}{BC} \sin. (BSA + ASC)$$

$$\text{and } \angle BAS = \frac{BS}{AB} \sin. (BSA)$$

$$\therefore \angle ABC = \angle ASC + BS \left(\frac{\sin. (BSA + ASC)}{BC} - \frac{\sin. BSA}{AB} \right)$$



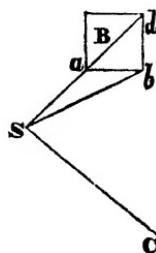
In the case where BC and AB are infinite, each of the expressions, where they occur, will vanish; and $\angle ABC$ becomes equal to $\angle ASC$, which is the case when the objects are heavenly bodies.

This obtains also, when the station falls upon the circumference of the circumscribing circle.

It is not always possible, however, to measure BC, or take the angle CSB.

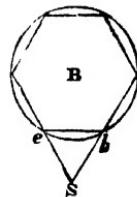
As it is desirable that the instrument should be placed close to the centre of the station, the vane of the steeple, or the flag pole of the tower, may be invisible from it.

When this is the case, if the centre be the vane of a church, as in the diagram, when the tower is squared, select S in a line with the diagonal. Measure $Sa + \frac{a d^*}{2} = SB$; and take the angle $CSa = CSB$.

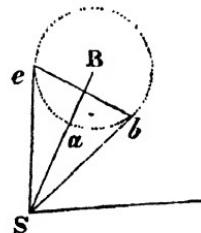


Again, let the tower be a hexagon.

Produce outward two of the sides to S, where place the instruments; (ebS) is an equilateral triangle, and $SB = eb\sqrt{3}$ * and $\angle CSB = \angle(CSa + \frac{bSe}{2})$



If the tower be circular, the $\triangle CSB = \angle CS + \frac{bSe}{2}$, but $\angle SB = bS \cdot \sec \sqrt{\frac{bSe}{2}}$. or if the circumference of the tower can be taken $= Sa + \text{radius}$.



EXAMPLES OF CALCULATIONS OF THE REDUCTION OF ANGLES TO THE CENTRE.

EXAMPLE 1.—Determining the angle made at the centre of the spire of Highgate Church, between the spire of Hampstead Church and St. Paul's, from angles taken in the Church-yard, between St. Paul's and Hampstead, and St. Paul's and Highgate.

* The interior angle of a hexagon is 120° , the angle at e 120° , and Be bisects it, and the $\angle B eb$, is 60° , but the angle $S eb$ is also 60° , and therefore BS is bisected by eb, but each of its bisectors $= \tan. 60^\circ \times \frac{eb}{2}$.

∴ The whole $BS = \tan. 60^\circ \times eb$, and, as $\tan. 60^\circ = \sqrt{3} = eb \cdot \frac{\sqrt{3}}{2}$

| | | |
|--------------------|----------------|--------------------------|
| from Δ s | $2 \cdot 56$ | to Highgate Church Spire |
| between St. Paul's | $156 \cdot 16$ | and Highgate Church |
| at Δ s | $85 \cdot 25$ | and Hampstead Church |

Let $x =$ the unknown angle DEF.

Now, by the preceding formula,

$$\angle DEF + \angle EDS = \angle EFS + \angle DSF.$$

$$\therefore \angle DEF \text{ or } x = \angle DSF + \angle EFS - \angle EDS$$

but the sine of the angle EFS = ES ($\frac{\sin. \angle EFS}{EF}$)

where EF is the distance from St. Paul's to Highgate Church;

and the sin. $\angle EDS = ES (\frac{\sin. \angle ESD}{DE})$, where DE

is the distance from Hampstead to Highgate Church.

Hence the values of the angles EFS, and EDS can be obtained; and, by adding their difference to the observed angle DSF, you obtain the value of the corrected angle DEF, from the centre of the previous point of observation.

To find the distances EF and DE, which are at present unknown, having the data given in the preceding chapter.

In the triangle ADB (see Diagram p. 171), to find the distance DB,

$$\text{As sine } \angle ADB^* (19^\circ 20') = 9 \cdot 5199112$$

$$\text{is to AB, } 24 \cdot 74 \text{ chains} = 1 \cdot 3933997$$

$$\text{so is sine } \angle DAB, (94^\circ 29') = 9 \cdot 9986691$$

$$11 \cdot 3920688$$

$$9 \cdot 5199112$$

$$= 1 \cdot 8721576$$

$$\text{to DB} = 74 \cdot 50 \text{ chains,}$$

In the triangle EDB, to find ED.

Having calculated the length of DB, we have two sides and the included angle, therefore—

$$\text{As sum of sides (DB+BE)} 74 \cdot 50 + 61 \cdot 66 = 2 \cdot 1340495$$

$$\text{is to their difference, } 74 \cdot 50 - 61 \cdot 66 \text{ chs.} = 1 \cdot 1085650$$

$$\text{so is tan. } \frac{(x+y)}{2} 19^\circ 41' = 9 \cdot 5535477$$

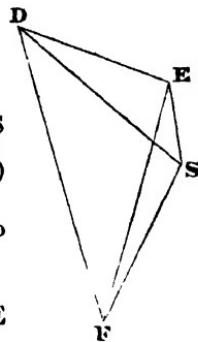
$$10 \cdot 6621127$$

$$2 \cdot 1340495$$

$$= 8 \cdot 4194982$$

$$\tan. \frac{x-y}{2}, 1^\circ 30' 18''$$

* The angle ADB is the supplement of the observed angles DAB, and DBA.



$$\begin{array}{r}
 19^\circ 41' \\
 1^\circ 30' 18'' \\
 \hline
 21^\circ 11' 18' = \text{greater angle DEB} \\
 18^\circ 10' 42'' = \text{smaller angle EDB}
 \end{array}$$

Again, in the same triangle,

$$\begin{array}{lcl}
 \text{As } \sin. \angle EDB, 18^\circ 10' 42'' & = & 9.4941207 \\
 \text{is to EB, 61.66 chains,} & = & 1.7900026 \\
 \text{so is } \sin. \angle EBD, 39^\circ 22' & = & 9.8022816 \\
 & & \hline
 & & 11.5922842 \\
 & & 9.4941207 \\
 \text{to ED, 125.36 chains,} & = & 2.0981635
 \end{array}$$

Now substitute their proper values in the two equations, viz.,

$$\sin. \angle EFS = ES \frac{(\sin. \angle ESF)}{EF}$$

and $\sin. \angle EDS = ES \frac{(\sin. \angle ESD)}{DE}$, thus :—

$$\begin{array}{lcl}
 \log. \sin. \angle EFS = \log. ES + \log. \sin. \angle ESF - \log. EF, \\
 \text{and } \log. \sin. \angle EDS = \log. ES + \log. \sin. \angle ESD - \log. DE, \\
 \log. ES, 2.56 \text{ chs.} & = & 0.4082400
 \end{array}$$

$$+ \log. \sin. \angle ESF, (156^\circ 16') = 9.6030166$$

$$-\log. EF, = 10.0112566$$

$$\log. \sin. EFS (0^\circ 10' 1'') = 2.5468246$$

$$\text{and } \log. ES 2.56 = 7.4644320$$

$$+ \log. \sin. \angle ESD (70^\circ 51') = 9.9755394$$

$$-\log. DE, 125.36 \text{ chains} = 10.3837794$$

$$\log. \sin. EDS (1^\circ 6' 22'') = 2.0981630$$

$$\text{but } \angle DEF \text{ or } x = \angle DSF + (\angle EFS - \angle EDS)$$

$$\angle EFS = 0^\circ 10' 1''$$

$$-\angle EDS = 1^\circ 6' 22''$$

$$\text{their difference} = 56' 21''$$

therefore $85^\circ 25' (\angle DEF) + (-56' 21'') = (84^\circ 28' 39') = \angle DEF = x$
 which is the angle at the spire of Highgate Church, between Hampstead Church and St. Paul's.

EXAMPLE 2.—Having measured the distances between two objects AB 2 miles, 6 furlongs, and taken the angles at the base line, to a third object, viz., $59^\circ 25' 40''$ and $78^\circ 24' 20''$. It was required to observe the remaining angle at the third station, as a check upon the other two angles; planted the theodolite at a distance of 3 chains from it, and found one of the angles 80° . What must the other be, so that the previous observations should be correct?

CHAP. IX.

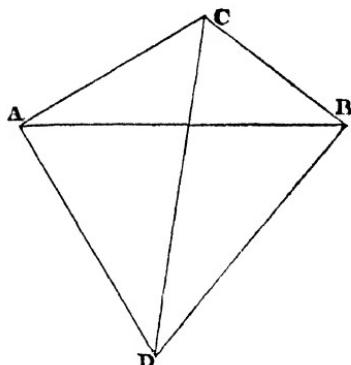
ON TRIANGULATION.

In extensive surveys, carried on by a continued system of triangulation, the most important part is the proper selection of a base line, proportionate to the intended extent of the survey. This base line should be measured on a nearly level surface of country. In hilly countries the plane of a valley is generally selected for that purpose.

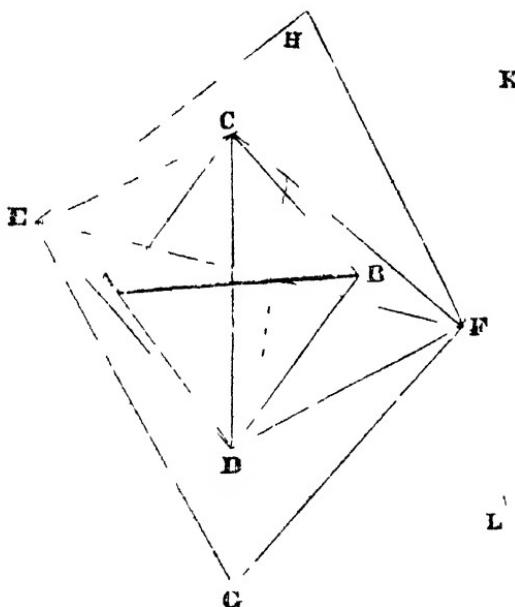
Let AB be the base, measured in a valley, and CD, two prominent objects, on neighbouring hills, which are visible from A and B.

Take at C, the angles ACD, BCD; and at D, the angles ADC, BDC; and the stations, C and D, are determined, by the formulæ given in Chap. V., Prob. I. DC thus becomes a new base line, of longer extent and equal accuracy with AB; or, by taking at A and B, when B is visible from A, the angles between the base, and C and D respectively; C and D can be more directly and more accurately determined, as in either triangle its length will be the same. The three angles should, in all cases, be taken, if practicable, as from their sum, which should be equal to 180 degrees, the accuracy of the work can be tested.

The sides of the triangle should be, as nearly equilateral as possible, or the angle at the new station should not differ, materially, from 90 degrees. All the sides should be calculated and plotted from their determined lengths, and not protracted from their angles, as the smallest error of an angle would be of injurious effect, in determining the position of the new station.



The accompanying diagram will exhibit the method of extending a system of triangulation, and of obtaining, between inaccessible stations, a base line, commensurate with the extent of the survey.



Let GH be the base line required, and inaccessible ; AB, the only favourable base line that can be measured.

By determining from AB the position of C and D, CD becomes a new station ; from CD determine E and F, EF again becomes a new base for the stations G and H, which, by observing the angles EHF and EGF, and comparing their calculated with their observed values, becomes a base, as much to be depended upon as the first AB.

It is not, of course, necessary that the triangulation should be carried on in the regular manner, exhibited in the figure, as it might be branched off in any direction that might be required ; for after having, from AB, determined C and D, CB might be taken as the new base, as correctly as DC, and the triangulation extended towards K ; or, after having determined E and F, it might be extended towards L ; and by a similar process, in any other direction whatever.

Having carried the triangulation in the direction, and to the extent required, it becomes desirable, for the sake of testing the accuracy of the work, to make one of the lines, of the last triangles, a base of verification, by selecting for it a level position, along the slope of a

hill, or in the bottom of a valley. This base, as it can be computed trigonometrically, being compared with its actual length by measurement, is a test, not only of its own accuracy, but of all the various triangles that subserve to its determination.

As the triangulation goes on, the sides increase in length, and the angles taken are between objects of some miles distant. It becomes then imperative to call in the aid of science to make the objects distinct. For this purpose, various contrivances have been, at different times, adopted, such as plane mirrors, disks of tin, plane convex lens, parabolic reflectors, to receive the rays of artificial light, that were thrown upon them, by means of Argand lights, balls of burning lime, &c. These latter were introduced by Captain Drummond, who managed to send through a flame of alcohol, a powerful stream of oxygen gas, upon the lime, which was placed in the focus of a parabolic reflector.

By these means, angles have been taken to objects from 40 to 60 miles off.

REDUCTION OF A BASE LINE, TO THE LEVEL OF THE SEA.

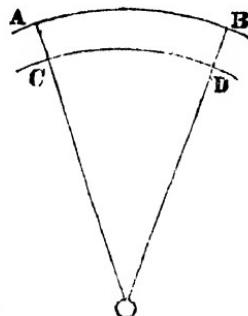
Let CD be the length of the line at the level of the sea $= x$, AB , the measured line above it. Let AC be the height of the measured line upon the level of the sea $= h$, and CO be the radius of the earth $= R$. It is required to find the length of CD .

As the arcs of different circles, subtending the same angle, are proportional to their radii, we have,

$$AO : CO :: AB : CD, \text{ or}$$

$$R+h : R : L : x$$

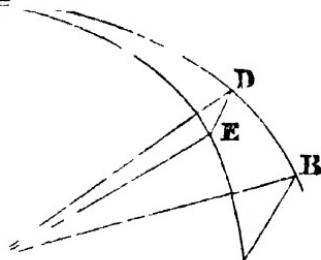
$$\therefore x = \frac{R}{R+h} \cdot L; \text{ and } \therefore L-x = L - \frac{R}{R+h} \cdot L = \frac{h}{R+h} \cdot L$$



Or the *excess* of any line AB , measured above the level of the sea, is equal to the length of the line, into its height above that level, divided by its distance from the centre of the earth. Now the radius of the earth is 21,008,000 feet, therefore $\log. L$ in feet + $\log. h$ in feet - $\log. (21,008,000+h)$ in feet = $\log.$ excess of measured base above the true base at the level of the sea. This base is the arc CD .

To reduce angles, taken in a plane inclined to the horizon, to angles in the horizontal plane.

Let DAE, the angle taken at the Z point D, between the objects D and E, be in a plane, inclined to the horizon. Let BAC be the horizontal plane, and Z the zenith of the observer at A, then AZEC, AZDB will be portions of planes, of large circles, passing through the radius AZ, and the horizontal angle will be BAC.



Let CAE be the angle of elevation of the station E, and DAB that of D, of which EC and EB are the measures ; ZE and ZD are the zenith distances, or measures of the complements of these angles of elevation ; and DE is the measure of the angle DAE.

In the triangle ZDE, there are ZE, ZD, and DE given, hence the sine $\frac{1}{2}$ BAC or $\frac{1}{2}$ DZE = $\sqrt{\frac{\sin \frac{1}{2}(S-ZE) \sin \frac{1}{2}(S-ZD)}{\sin ZE \cdot \sin ZD}}$

S, being the sum of the sides of the triangle ZED ; or $\sin \frac{1}{2} Z$ $= \sqrt{\frac{\sin \frac{1}{2}(\angle DAE + \angle BAD + \angle CAE) \cdot \sin \frac{1}{2}(\angle DAE + \angle CAE - \angle BAD)}{\cosine \angle BAD \cdot \cosine \angle CAE}}$

If the angle BAD = angle CAE, or the objects be of the same altitude, then,

$$\sin \frac{1}{2} Z = \sqrt{\frac{\sin^2 \frac{1}{2} DAE}{\cos^2 CAE}} = \frac{\sin \frac{1}{2} DAE}{\cos CAE}$$

In the first case, when the angles of elevation differ slightly, and when each of them is but small, not exceeding 2° or 3° , as the cosines of angles vary slowly, the following formulæ may be safely adopted, viz., $\sin \frac{1}{2} Z = \frac{\sin \frac{1}{2} DAE}{\cos(H+h)}$. when H and h are the respective angles of elevation, which becomes a convenient logarithmic formulæ, viz., $\log. \sin \frac{1}{2} Z = 10 + \log. \sin \frac{1}{2} DAE - \log. \cos \frac{1}{2}(H+h)$; but the angle BAC is the measure of the angle Z, or the plane angle made between the two planes.

SPHERICAL EXCESS.

The angles taken between any three points on the surface of the earth, by a theodolite, are, strictly speaking, spherical angles, and their sum must exceed 180° ; and the lines bounding them, are not the chords, as they should be, but the tangents to the earth.

This excess is inappreciable in common cases, but in the larger triangles it becomes necessary to allow for it, and to *diminish* each of the angles of the observed triangle, by one third of the spherical excess.

To calculate this excess.

Divide the area of the triangle in feet, by the radius of the earth in seconds, and the quotient is the excess, viz.,

$$\text{excess} = \frac{\text{area}''}{R^2}. \text{ or,}$$

because the radius = as above, 21,008,000 feet, and one second of space = 101.43 feet, then 101.43² feet = the area in a square second, and radius = 206.264 seconds of space ∴ excess in seconds = $\frac{\text{area in square feet}}{(101.43)^2 \times 206264}$ or, log. excess = log. area - 9.3267737.

When two sides and the included angle are given.

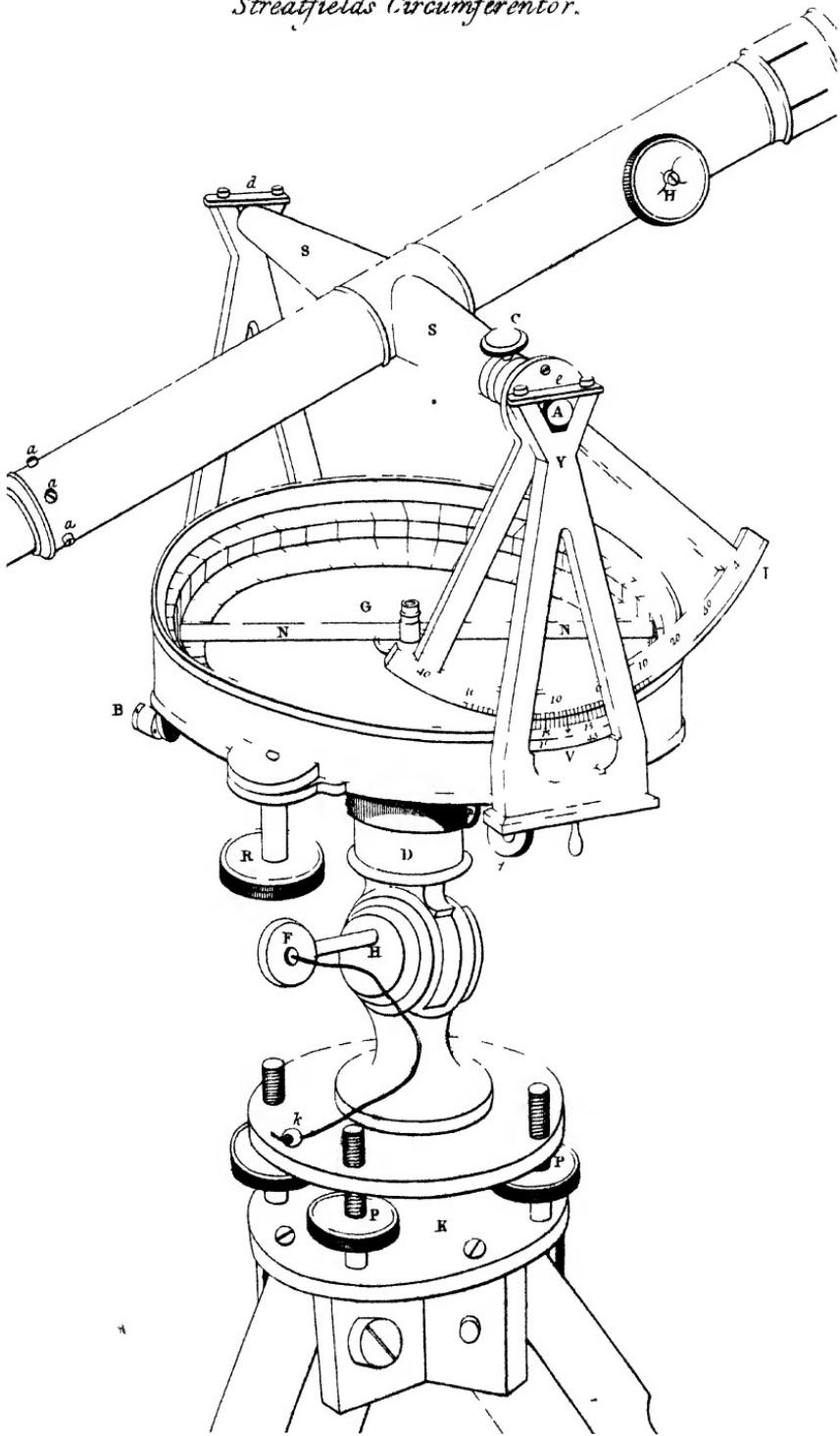
$$\cot. \frac{1}{4} \text{ excess} = \frac{\cot. \frac{1}{2} a - \cot. \frac{1}{2} b + \cos. C.}{\cos. C.}$$

When the three sides are given.

$$\begin{aligned} \tan. \frac{1}{4} \text{ excess} &= \left(\frac{\tan. a+b+c}{4} \right) \cdot \left(\frac{\tan. (a+b-c)}{4} \right) \\ &\quad \left(\frac{\tan. a-b+c}{4} \right) \quad \left(\frac{\tan. b+c-a}{4} \right) \end{aligned}$$

To allow for this excess.

In any triangle take the three angles, find their sum. Calculate from these angles the spherical excess, by the above rule; the sum of the angles taken should amount to $180^\circ +$ this spherical excess; if not, the difference must be divided among the three observed angles, so as to make them, when thus corrected, equal to $180^\circ +$ the excess; then subtract one third this excess from each of the angles, and their sum will be reduced to 180° , the correct measure of a plane triangle.

Sreatfield's Circumferentor.

LAND SURVEYING.

Part the Third.

THE CIRCUMFERENTOR.

THE Circumferentor is the mariner's compas differently divided, and furnished with sights, standing upon one or three legs, and capable of a horizontal motion, by means of the usual parallel plates, or a ball and socket. In new countries, where expedition is required, the circumferentor is generally used with a ball and socket, and with one leg or staff, strongly shoed with iron. After taking the angle at any station, the head part is taken off, and is carried by the Surveyor under his arm, the staff is seized in his hand, and the next station is proceeded to. The mould of the woods is soft, and easily penetrated; and as for the purpose of blazing* the trees the general direction alone is wanted, which may be blazed within 10 yards on either side of the line, there is no delay arising from the placing of flags and driving of stakes, the axemen, being

* Blazing, in American phraseology, is taking a slice off the bark of a tree into the wood.

generally in advance or close upon the heels of the Surveyor, and the chain men overtaking him, before he has taken his next sight.

All the LINE trees, as trees are termed, which stand directly in the line, in addition to being *blazed*, are *witnessed* with three notches, and being marked where the line strikes them, become permanent boundary marks—of so permanent and distinctive a character, that the very year almost of their being blazed is distinguishable by an experienced hand. The scar seldom grows over entirely: never without a seam, which, if carefully cut, will, by the number of rings, that every year has added to it, show the exact year of the incision.

The three legged staff, however, is very useful; especially in old countries, or in the surveying of roads in frosty weather, and under various other circumstances, when the single staff cannot penetrate, or when walls intervene, upon which the short legs of the staff can be advantageously placed.

STREATFIELD'S CIRCUMFERENTOR.

This instrument consists of a compass box (G); divided into degrees, and, by means of a vernier, subdivided into three minutes.

It stands upon three legs, and, by means of a pair of parallel plates (Kk,) is capable of a truly horizontal position, which is determined by a level, placed, so as not to interfere with the reading, under the compass box (the end of it alone can be seen in the accompanying plate at B).

This compass box, (G,) has an absolute horizontal

motion round its centering at D, and is fastened by the clamp screw (a side of which is visible) at p.

When this is clamped, by detaching the pin f, which passes through the two plates of the compass box, the brass one, which, with the vernier attached, works round the inner one, on which are divided the degrees, is capable of a relative motion, and thus partakes of the character of the theodolite; this motion is communicated to it by a rack and pinion at R.

There is, however, one thing wanting in this instrument, which is indispensable in a new country. There is no means of clamping the vernier, except when it is at zero of the dividing-plate. In running lines, of a given direction, through the woods of a new country, where the bearings are often odd minutes, it would be impossible, at every fresh station, to be altering the vernier.

In the circumferentors that are used in America, they have placed the vernier outside, and by that means have contrived to clamp it to any position; there is also a tangent-screw for fine adjustment, so that being properly adjusted for the odd minutes, or the variation of any kind, and clamped, the bearing of the full number of degrees need alone be referred to throughout the whole line. None but a practical man can be aware of the immense advantages resulting from this arrangement.

YY are two frames, or supports, capable of being taken on and off, on the Y's of which rests the arm (Ss) of a telescope, so contrived, as to move in a truly vertical position, when the instrument is horizontal. The telescope has its usual adjustments for the object and eye-glasses, and for the line of collimation.

To one end of the cross piece of the telescope is attached a graduated arc of a circle, with a vernier fastened to the supports of the Y, for taking vertical angles. This instrument has also, in addition to the telescope (not shown in the plate), the usual pair of sights, whose position would be, in the meridian line of the instrument, in the same plane as the telescope moves in.

The introduction of the telescope was forced upon me, from the constant inconvenience I experienced in the back woods of America, (where I was engaged in government surveys,) from the almost uselessness of the common sights, in surveying up and down hills, across steep but narrow vallies; all these difficulties were remedied by this contrivance, the advantages of which I have tested practically in the woods. The telescope is capable of a vertical motion either way of 45 degrees, and moving, truly vertically, enables you to carry a line of a given direction either down or up a hill, and from its magnifying power to secure, at the same time, a check position at the top of the ascent beyond; so that, after having descended and ascended, you may prove your correctness.

The instrument, also, has other contrivances; the pin F, when taken out, allows the whole instrument to be turned upon its side, and the spirit level at B, being then in a horizontal position, the instrument is made capable of a vertical motion, reading off to three minutes, by means of the vernier.

This may sometimes become a fair substitute for a sextant on a clear night; though I should myself, in all cases, prefer each instrument being kept to its

especial purposes, as, the more simple an instrument is, the more accurate.

DIVISION OF THE CIRCUMFERENTOR.

The line of sights is made the north and south end of the *instrument*, and from each of these the circular rim is graduated toward the east and west points, from 0° to 90° .

On the right of the north of the instrument, looking to the north point of it, should be lettered west, and on the left should be lettered east; and any point between the north and west point of the instrument, read by the north end of the needle, is read north, so many degrees west.

When the needle is released, and is allowed to play freely, it points toward the magnetic north. The north of the instrument points to the object whose bearing is required, the angle made between these two must necessarily give the relative position of the line of the object, and the magnetic north and south line; and the bearing of the object, by reading off the number of degrees to which the needle points on the graduated circular rim, is thus obtained.

As the needle points to the north, should the object bear to the west, the line of sights would be on the left of the needle, and the north end of the needle would be on the right of the line of sights; if, therefore, on the right of the instrumental north, looking northwards, were marked east, the needle, which should at once, with the number of degrees, read off the bearing, would give east, but the bearing is west. Having been, as was shown above, marked

west, the needle reads west, that is, north so many degrees west. By this arrangement the needle gives at once the degrees and bearing.

As the accuracy of the bearing depends, in a considerable degree, upon the goodness of the needle, great care should be observed in using it, and in marking whether it continues vibrating, or soon settles.

In the latter case there is some radical defect, arising either from the diminution of magnetic power in the needle, or from the wearing away of the centre on which it plays ; this must be corrected immediately.

A really good needle is actually wearisome in its tardiness to settle.

DEFINITIONS.

Meridian lines are due north or south lines, and strictly considered, are arcs of large circles of the earth meeting at the poles ; these arcs, however, are subtended by so small an angle, and are so infinitely small in comparison of the whole circle, as to admit of their being assumed as parallel.

The distance of a line is its horizontal measurement, as a tangent to the earth, not following the surface of the ground, and is usually calculated in chains and links.

The angle of bearing of any line, is the angle of bearing made between that line and a meridian line running through the point, where the instrument is placed ; and is measured always from north or south, eastward or westward.

Thus a line is said to bear *magnetically* north 16 degrees west, when the needle points to 16 degrees on the graduated circle, and when the direction of the line is to the west or north.

The *reverse bearing* of a line, is merely the bearing taken in a contrary direction.

The reverse bearing of a line, bearing north 38 degrees east, is south 38 degrees west ; that of south 75 degrees west, is north 75 degrees

east. The reverse bearing, therefore, is measured by the same angle,* as the direct bearing, only taken in opposite directions: from south bearing northward, from east westward.

Difference of latitude, or northing and southing, is the distance that the end of the line is further north or south than the beginning.

The *difference of longitude, or departure,* is the distance that the end of the line is east or west from the beginning.

In changing our position from one point on the earth's surface to another, in a direction making any angle with the meridian, we at once change our latitude and longitude—the one is the northing and southing, the other the easting and westing.

To obtain the latitude and departure geometrically, when the length and bearing of a line are given.

Draw a meridian line through either end, and let fall a perpendicular from the other. This perpendicular is the departure; and that portion of the meridian line, intercepted by it, is the difference of latitude.

The latitude,† departure, and distance, form the three sides of a right-angled triangle, whose angle at the vertex is the angle of bearing; whose hypotenuse, or radius, is the distance; the latitude, the cosine; and the departure, the sine of the angle of bearing.

The meridian distance of any station, is the distance of that station from the meridian line passing through the first or any other assumed point, and is equal to the difference between the sums of the eastings and westings from that point; and is east or west, as the eastings or westings predominate.

THE TRAVERSE TABLE

Is a table giving the latitudes and departures to any distance and bearing, to the extent of the number of minutes to which it may be calculated.

The tables, which are appended to the present work, are calculated to every three minutes of the quadrant, and to every number from 0

* This is not the case where the lines are long, say two or three miles, and where the observations are taken in very high latitudes; for most practical purposes, it is sufficiently correct to assume the direct and reverse bearings the same.

By the term latitude, here and elsewhere, is meant *difference of latitude.*

to 10, from 10 to 100, &c. They have been prepared upon the following principle :—

The whole northing or southing of a line, of any bearing, is equal to the sum of the northings and southings of any number of lines of the same bearing, when the sum of the several distances is equal to the one distance of the whole line.

Let AE be a line, having any bearing whatever ; through E draw the meridian line EL ; and, from A, draw AL perpendicular to EL. AL is the departure, and EL is the latitude of the line AE.

Now, divide AE into AB, BC, CD, DE, and through B, C, D, draw the several meridian lines, Bm, Ch, De, and Bh, Ce, Df, at right angles to them.

Bm, Ch, De, Ef, are the latitudes of the several parts of the line AE, and are together equal to EL, that is, the sum of the northings of the parts is equal to the one northing of the whole.

And, in the same way, Am, Bh, Ce, Df, the sum of the eastings are together equal to AL, the one easting of the whole line AE.

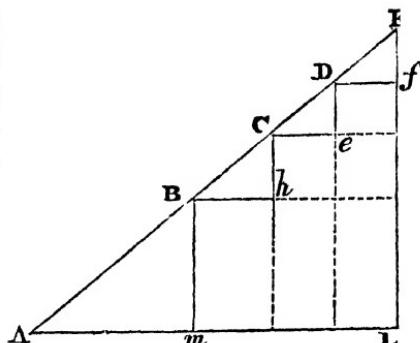
Again, if AE be a multiple of AB, (by similar triangles,) AL is the same multiple of Am, and EL is the same multiple of Bm, being but sines and cosines of similar arcs to different radii.

If Bm, therefore, be the northing of a line, whose distance is AB, or one chain, then the northing of a line AE, which is ten times AB, or 10 chains, is LE, or ten times Bm.

This being premised, the understanding of the tables of latitudes and departures, or traverse table, becomes very simple.

EXAMPLE.—Let it be required to find the latitude and departure of a line bearing north $4^{\circ} 3'$ east, and 15 chains 25 links long.

Look in the margin of the pages for the degrees, and down the column on the left hand for the minutes, then on the same line, collaterally with the three minutes, will be found the latitude and departures for any number of links or chains.



Take the latitude first.—For the 15 chains, look out in the tables for the latitude of 1'00 chains $\equiv .998$

multiply by $\equiv 10$

and you have $\equiv 9.98$

the latitude of 5'00 chains $\equiv 4.99$

For the 25 links, look out in the latter for the latitude of 2'00 chains $\equiv 1.99$

divide by $\equiv 10$

(because each 10 links are one tenth of a chain)

and you have $\equiv 0.19$

the latitude of 5'00 chains $\equiv 4.99$

divide by $\equiv 100$

(each link being one hundredth of the chain) $\equiv 0.05$

Add them together, and you obtain, as the one northing of the whole line, $\equiv 15.21$

Or, by supposing the number at the top of the page not to be confined to 1 chain, 2 chains, &c., for it is not necessary that they should be so limited, but to be 1 chain or 10 chains, 2 or 20, 3 or 30 chains, &c.; or 10 links or 1 link, 20 or 2 links, 30 or 3 links, &c., a more expeditious method is used, in obtaining the same result.

To find the latitude of 15.25 chains to the angle of bearing of 4 degrees 3 minutes.

Look in the tables as before, and for one chain you find 0.998, that is some decimal less than 1; instead of 1 chain, let this be 10 chains, then the northing becomes 9.98, or some decimal less than 10 chains; therefore the latitude

of 10 chains $\equiv 9.98$

of 5 chains $\equiv 4.98$

$\overline{14.96}$

Of 20 links, by looking in the tables under the head of 2, you find 1.99, that is, as before, some decimal less than 2. This may be 20 as well as 2, and 20 chains or 20 links indifferently. The latitude, therefore, of

$\overline{14.960}$

20 links $\equiv 0.199$

5 links $\equiv .050$

the total of which is as before $\equiv 15.21$

which is the latitude for 15 chains 25 links, at a bearing of 4 degrees and 3 minutes.

This is north latitude, because the given bearing is north.

To find the Eastings or Westings.

Look in the same place as before, under the head of degrees, and you will find that, at a bearing of 4 degrees 3 minutes, the departure of 1 chain = .071

| | |
|---|--------------|
| and (by the first method) by multiplying by | 10 |
| the departure of 10 chains | = 0.710 |
| of 5 chains | = 0.353 |
| of 2 chains = 0.141 dividing by 10 | = 0.014 |
| of 5 chains = 0.353 dividing by 100 | = 0.003 |
| total, east departure, | <u>1.080</u> |

which is the required departure of the whole line.

EXAMPLES.

Required the difference of latitude and departure of a line, which bears south 16 degrees 30 minutes east, 3 chains 47 links.

Ans. 3.33 south lat.; 0.98 east dep.

Given a line, bearing north 13 degrees 30 minutes west, and 6 chains 10 links long, to find the latitude and departure.

Ans. 5.93 north lat.; 1.42 west dep.

What are the latitude and the departure of a line bearing north 41 degrees 9 minutes east, 4 chains 47 links?

Ans. 3.36 north lat.; 2.95 east dep.

A line bears north 22 degrees 45 minutes west, 27 chains 62 links, required its latitude and departure.

Ans. 25.47 north lat.; 10.68 west dep.

These tables are found to be extremely useful; they enable you quickly to test the accuracy of the survey of a large extent of country; for, as the given distances and bearings are all capable of being resolved into their respective latitudes and departures, and as you cannot, from any point, go northwards without (to return to that point) coming back the same distance southward, nor any distance eastward, without remeasuring westward the same distance back, it follows, that, in going completely round a tract of country, the sum of the northings must be equal to the sum of the southings, and the sum of the eastings to that of the westings.

In adding up, however, the northings and southings, and eastings and westings, of bearings and distances, actually taken by the circumferentor, there will be generally found some small discrepancy, which the inaccuracy of the instrument may render inevitable. Within a certain limit this error is allowable. This error should not, however,

exceed one link to every five chains of the sum total of distances. Beyond this, a re-survey becomes necessary. This error must be apportioned among the whole, proportionally to the several distances of each bearing. The method will be explained hereafter.

The method of finding the bearing of a line by the Circumferentor.

Having placed the circumferentor over the point of the station, release the needle, and then, having unclamped the body of the instrument, by means of the parallel plates, as in the theodolite, make the whole instrument level. Now, turn the whole round until the *north* end of it lies towards the object, and looking through the sight, at the *south* end, fix the instrument, so that the fine web-line, in the north sight, exactly covers the object; then, when the needle has perfectly settled, (which should be immediately released on setting the instrument,) read off, by the north end of the needle, the number of degrees that it points to, from the north or south division line of the compass-box, according as the north end of the needle is in the north or south semicircle of the instrument; the angle measured by these degrees is called the angle of bearing.

Should the needle not point exactly to any degree, but lie between two of them, turn the instrument carefully, so as to make the needle point exactly to the next lower, and clamping the whole head of the instrument, detach, by withdrawing the connecting pin, the two horizontal plates. The instrument having been altered to suit the needle, the flag is no longer covered by the web. By using the rack and pinion, and carefully bringing the sights back, which are connected with the same plate as the vernier is, to cover the flag, the difference of the angle, in minutes, is denoted by the distance of the broad arrow in the vernier from the 360° , or the zero point of the other plate, which distance, as in the case of the vernier of the theodolite, is read, by observing which line of division in the vernier, reading to the left or the right (as the broad arrow is to the left or the right of the 360°), first coincides with some line in the graduated circle. In the circumferentor, the broad arrow of the vernier is placed in the centre, and if the distance from the 360° exceed half a degree, it is requisite to carry on the observation, as to which line first coincides, all round the plate, so as to end at the 360° . In some circumferentors the vernier is erroneously marked, as, in taking observations, the $15'$ may become $45'$, and the $45' 15'$, they should have been marked $\frac{1}{15}$ and $\frac{1}{45}$.

Having taken the angle, the two plates must again be brought to their proper position, by making the broad arrow of the vernier coincide with the 360° , where the connecting pin keeps them, till they are again required to be separated. The needle must be fixed, and the whole instrument clamped. It might not, perhaps, be out of place to observe here, that the kind of surveying required in a new country, as Australia or New Zealand, is directly the reverse of that which must be in use here. Here, a representation is, in most cases, wanted on paper, of actual boundaries of properties in the field: there, various lines, or certain positions, are to be marked down upon the ground. Here, the course of the hedge of a field is required: there, lines have to be run of given courses. The circumferentor, as constructed in this country, answers exceedingly well for the nature of the country it is generally used for here; but, when wanted to run a particular course, the objection, I have before mentioned, holds specially against it.

Having measured the distances to the next station, before taking the forward angle of the second line, take carefully the reverse bearing of the first, this will verify the last forward bearing, as the two should be the same; it also prevents, in a great measure, the probability of the needle being acted upon, without detection, by any magnetic substance in the neighbourhood; and I would especially recommend the young beginner invariably to adopt it. It is anything but a loss of time, and no survey can be depended upon without it.

VARIATION.

Before proceeding, it were better here to observe, that the magnetic meridian may not be the true meridian. The pole-star is not exactly at the pole; the needle seldom points either to the one or the other; two needles seldom point exactly the same way; and the same needle seldom points exactly the same way two years together. There are, therefore, two or three kinds of variation. The variation of the needle, strictly so termed, is the angle made between the magnetic north and the true north, and the variation is called east or west, according as the magnetic north is on the east or west side of the true north.

The measure of this angle, or the variation, is different in different countries, at the same time; and in the same country at different times. The various methods for determining this variation will be hereafter explained.

This general variation is not, however, that variation which principally affects the accuracy of a Surveyor's work; it is not the *general angle of variation*; but it is the *actual angle* made by his own needle and the true meridian line—that is the variation he has more particularly to guard against. This also shall be explained hereafter—observing only, now, that it is but necessary to determine once a true meridian by the method that will be then explained. By determining the true bearing of any fixed line—such as a long wall or fence—once, it becomes a standard of reference afterwards, as, at any time, the difference between its magnetic bearing, taken by an instrument and its true bearing, will determine the positive variation of the instrument required.

No Surveyor should assume that the variation of his instrument agrees exactly with the general *variation of the needle* given. Nor should he attempt to connect a new survey with an old one, after a year's interval, without determining, by the check or standard line, whether any, and what variation had taken place in his needle, and duly allowing for the difference; without this, no new line can be depended upon. In running divisions, or side lines, between lots in a new country, every instrument must be tested by the boundary line between the townships, as a standard line, and the division lines run parallel to it, whatever be the magnetic bearing, or the bearing of the individual instrument. This ever secures the parallelism of the lines.

CHAP. II.

To find the true bearing of a line, the magnetic bearing and the variation being given.

By the variation of the compass, is always meant the variation of the needle, which is the only variable east or west, from the true north, which is ever constant.

RULE.—*Mark upon paper*, the relative positions of the given line and the *magnetic north*, which represents the north end of the needle, then observe, whether the variation be easterly or westerly; if easterly, the *true north* will be to the left of the *magnetic*; and

if westerly, to the right; place this also on paper, in its proper relative position.

The angle made between this last line (of the true north) and the given line, is the angle of bearing; the expression of this angle depending, of course, whether, by the variation, it may or may not have been moved into a different quadrant, being often changed from the magnetic N. W. to the true S. W., or N. E.

EXAMPLES.

- Let the line AB bear N. 24° E., and the variation be $23\frac{1}{2}^{\circ}$ W.; required the true bearing of the line.

AB, bearing N. 24° E., the needle is on the left of the line, but the needle bears west, the true north is on the right of the needle, therefore the line and the true north, being both on the right of the needle, the line being at the greater angle, the difference of the D angles is the bearing of the line eastwards, N. $0^{\circ} 30'$ E.

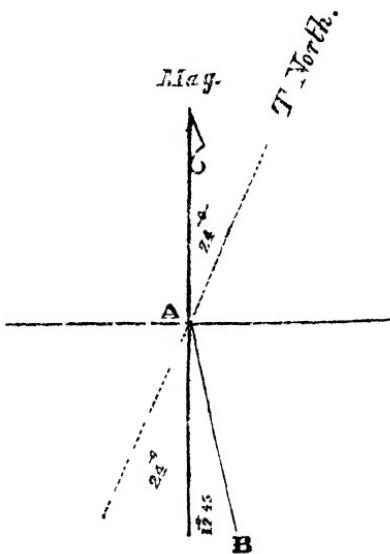
- Let the reverse bearing of AB be S. 24° W., with the same variation, what is the true bearing?

Ans. S. $0^{\circ} 30'$ W., which is the result of the former.

- Let AB bear S. $12^{\circ} 45'$ E., and the variation be 24° W.; required the true bearing.

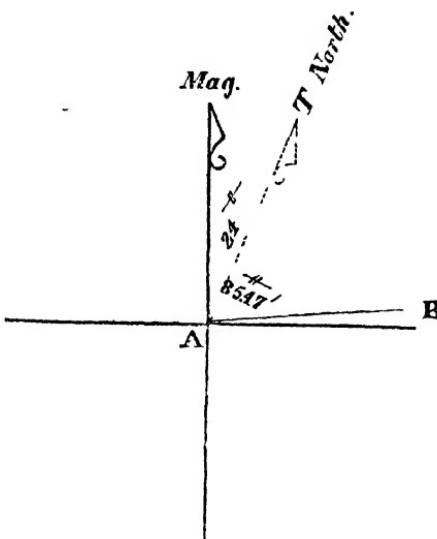
Now the north end of the needle is to the left of the true north, therefore,

$$\begin{array}{r} 24^{\circ} \\ + 12^{\circ} 45' \\ \hline = S. 36^{\circ} 45' E. \end{array}$$



4. Given AB, N. $85^{\circ} 47'$ E., with the same variation of 24° W.; required the true bearing.

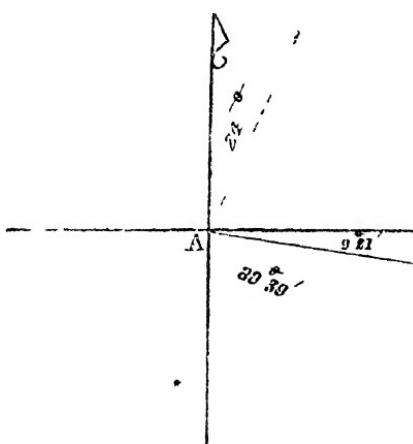
$$\begin{array}{r} \text{N. } 85^{\circ} 47' \\ - 24^{\circ} 00' \\ \hline \text{N. } 61^{\circ} 47' \text{ E.} \end{array}$$



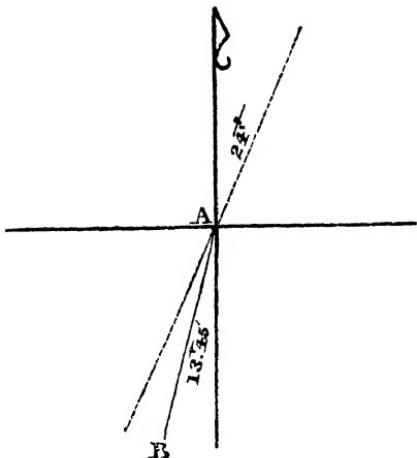
5. Given AB, S. $80^{\circ} 39'$ E., with same variation; required the true bearing.

$$\begin{array}{r} 90^{\circ} \\ - 24 \\ \hline 66^{\circ} \end{array}$$

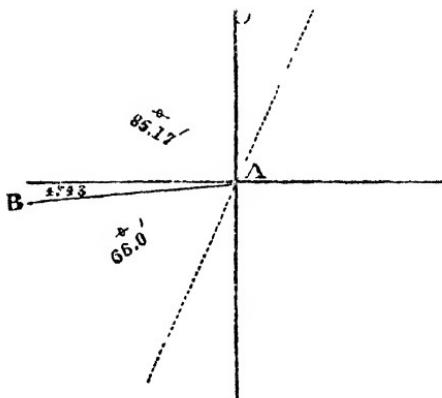
$$\begin{array}{r} 90' \\ - 80^{\circ} 39' \\ \hline 9^{\circ} 21' \\ 66^{\circ} \\ \hline \text{N. } 75^{\circ} 21' \text{ E.} \end{array}$$



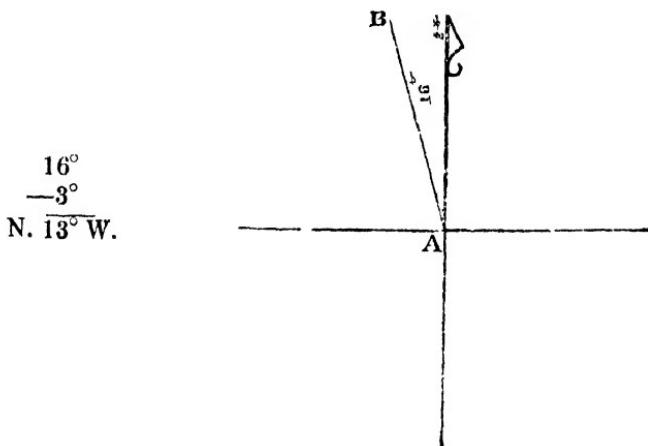
6. Given AB, S. $13^{\circ} 45'$ W., with the same variation; required the true bearing.

$$\begin{array}{r} 24^{\circ} 00' \\ - 13^{\circ} 45' \\ \hline \text{S. } 10^{\circ} 15' \text{ E.} \end{array}$$


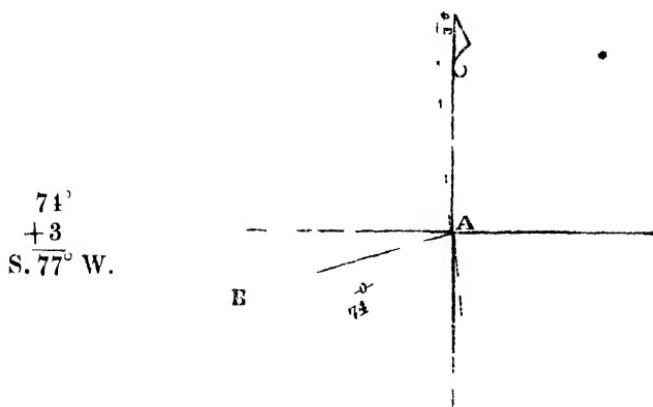
7. Given AB, N $85^{\circ} 17'$ W., variation as before; required the true bearing.

$$\begin{array}{r} 90^{\circ} & 99^{\circ} \\ - 24^{\circ} & - 85^{\circ} 17' \\ \hline 66^{\circ} & 14^{\circ} 43' \\ + 66^{\circ} & \\ \hline \text{S. } 70^{\circ} 43' \text{ W.} \end{array}$$


8. Given AB, N. 16° W., variation 3° east; required the true bearing.

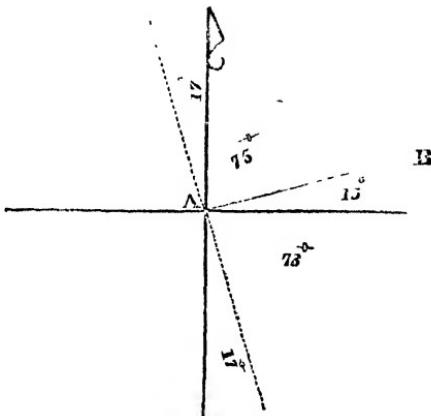


9. Given AB, S. 74° W., variation 3° east; required the true bearing.



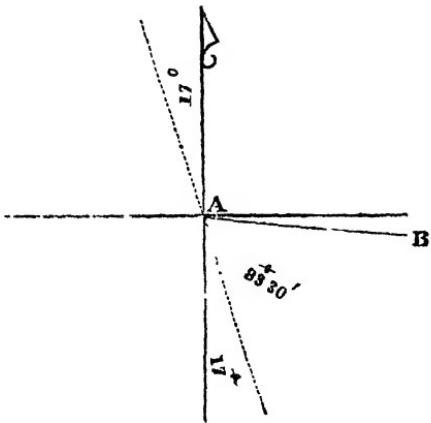
10. Given AB, N. 75° E., and the variation of needle 17° east; required the true bearing.

$$\begin{array}{r} 90^\circ \\ - 75^\circ \\ \hline 15^\circ \end{array} \quad \begin{array}{r} 90^\circ \\ - 17^\circ \\ \hline 73^\circ \\ 15^\circ \\ \hline \overline{S.88^\circ E.} \end{array}$$



11. Given AB, S. $83^\circ 30'$ E., variation 17° east; required the true bearing.

$$\begin{array}{r} 83^\circ 30' \\ 17^\circ \\ \hline \overline{S.66^\circ 30'E.} \end{array}$$



CHAP. III.

THE BEARINGS OF TWO LINES BEING GIVEN, TO DETERMINE THE ANGLE BETWEEN THEM.

RULE.—First let both these lines run northwards or southwards. If they run on the same side of north or south, whether eastward or southward, this angle will be the *difference* of their angles of bearing ; if on different sides, it will be their sum.

Next, let one run north and the other south.

If they both run on the same side of the meridian, this angle will be the supplement angle of the *sum* of the angles of bearing.

If one be on the east, and the other on the west of the meridian, this angle will be the supplemental angle of the *difference* of their angles of bearing.

In the interior angles of a polygon, as an angle may exceed 180° , the required angle might be the difference between the angle calculated as above, and 360° .

EXAMPLE 1.—Given AB, N. 16° W., and AC, N. $12'$ E., to find the angle between them.

$$\begin{array}{r} 12^\circ \\ 16^\circ \\ \hline 28^\circ = \text{angle between them.} \end{array}$$

EXAMPLE 2.—Given AB, N. 16° W., and AC, S. 12° W., to find the angle between them.

angle = supplement of sum.

$$\begin{array}{r} 12^{\circ} \\ 16^{\circ} \\ \hline 28^{\circ} \end{array} \qquad \begin{array}{r} 180^{\circ} \\ 21^{\circ} \\ \hline 152^{\circ} \end{array}$$

152° = angle between them.

EXAMPLE 3.—Given AB, N. $84^{\circ} 20'$ W., and AC, S. $49^{\circ} 51'$ E., to find the angle between them.

here the angle = the supplement of their difference.

$$\begin{array}{r} 84^{\circ} 20' \\ 49^{\circ} 51' \\ \hline 34^{\circ} 29' \end{array} \qquad \begin{array}{r} 180^{\circ} \\ 34^{\circ} 29' \\ \hline 145^{\circ} 31' \end{array}$$

$145^{\circ} 31'$ = angle required.

EXAMPLE 4.—Given AB, S. 25° E., and AC, S. 16° E.; required the angle between them.

angle = their difference.

$$\begin{array}{r} 25^{\circ} \\ 16^{\circ} \\ \hline 9^{\circ} \end{array}$$

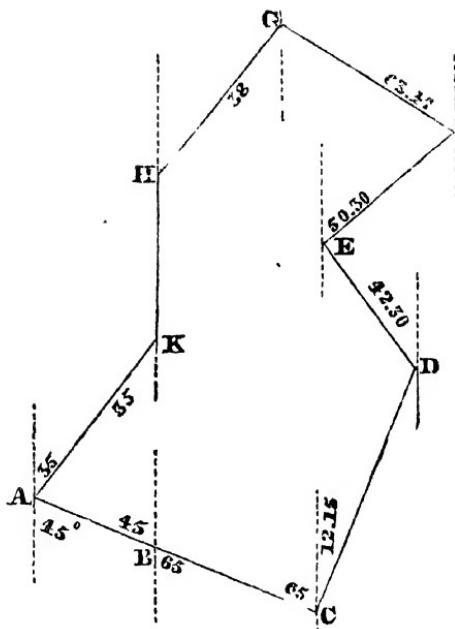
9° = angle required.

For further practice, I adjoin the following Example, where the bearings only are given, and the interior angles (being severally the angles between every two adjacent lines of the whole) are required. This Example will establish an important truth, that the sum of the interior angles of any figure, obtained from the *bearings* of its sides, must, *right* or *wrong*, always amount to four less than twice as many right-angles as the figure has sides, and that, therefore, the sum of these angles, can, in no case, be used as a check upon the correctness of the work.

EXAMPLE 1.—Given a tract of country, with the bearings of its several boundaries, to find the interior angles, and determine their correctness, viz., AB bears S. 45° E.—BC, S. 65° E.—CD, N. $12^{\circ} 15'$ E.—DE, N. $42^{\circ} 30'$ W.—EF, N. $45^{\circ} 30'$ E.—FG, N. $63^{\circ} 47'$ W.—GH, S. 38° W.—HK, due S., and KA (S. 35° W.) Required each of the angles.

Take first the angle ABC; this is equal to the first angle of bearing, 45° + the supplement of the second angle, 65° , that is, equal to $45^\circ + 115^\circ = 160^\circ$.

BCD, the second interior angle, is equal to the second angle of bearing, 65° + the third angle $12^\circ 15'$, or $77^\circ 15'$.



Thus, ABC being found to be $= 160^\circ$
and BCD $= 77^\circ 15'$

$$\text{CDE} = 125^\circ 15'$$

$$\text{DEF} = 273^\circ 00'$$

$$\text{FGH} = 65^\circ 43'$$

$$\text{FGH} = 101^\circ 47'$$

$$\text{GHK} = 142^\circ 00' \quad 915^\circ 0'$$

$$\text{HKA} = 215^\circ 00'$$

$$\text{KAB} = 100^\circ 00'$$

$$90 | \underline{1260^\circ 00'}$$

14 right angles

$$+ \frac{4}{18} \text{ equals twice the number of}$$

sides of the polygon.

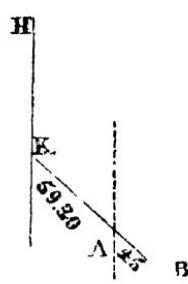
Now let KA, instead of S. 35° W., bear S. $59^\circ 30'$ E., and find the amount of the present interior angles.

All the former angles, but the last two, remain the same; the sum of the constant angles equals $945^\circ 00'$

The angle HKA now equals $120^\circ 30'$

And the angle KAB $194^\circ 30'$

resulting the same total as before, of $1260^\circ 00'$



Hence the sum of the interior angles of a polygon, when these angles are calculated from the bearings of the several lines, is no check upon the correctness of the work.

But the equality, or discrepancy of the northings and southings, of the eastings and westings, depending upon the lengths, as well as the bearings of the lines, is a certain test of its accuracy.

The method of applying this test, we will, therefore, now proceed to consider.

EXAMPLE.—The bearings and distances of the several sides of a polygon being given, it is required to determine their correctness, and to divide the allowable error, proportionally, among the whole.

Rule a schedule, as in the annexed example, dividing it into columns, headed stations, bearings, distances, northings and southings, eastings and westings.

Find, as above shown, the difference of latitude and departure, in the tables for the several bearings and distances, and place the numbers, thus found in the proper column of northings, southings, eastings, or westings. Add them together, separately, find the difference of the northings and southings, and of the eastings and westings; the former is called the error of latitude, and the latter the error of departure. This error, if it does not exceed one link in five chains in the sum total of the distances, must be apportioned among each of the distances by the following proportions, viz.—*As the sum of all the distances is to the whole error, so is each distance to its correction.*

This must be done for the latitudes and departures, and must be placed in a column appropriated to each, called the north and south correction, and the east and west correction; the correction, thus determined, must be placed, collaterally, with the distance to which it refers, without distinguishing as to north, or south, east, or west.

In making this proportion, the links of the distances need not be taken into account, and frequently, as an approximation is sufficient, the apportionment may be made without reference to calculation.

Having found the several corrections for each of the latitudes and departures, add them together, severally, and see whether their total agrees with the whole error; then draw four other columns, heading them, corrected northings, southings, eastings, westings, and proceed to allot the corrections. If the error be an excess of northings, subtract each correction from its collateral northing, or add it to the collateral southing; if an excess of easting, add to the westing; and

subtract from the easting; the respective sums of their corrected latitudes and departures will now be found exactly to agree.

EXAMPLE 1.—Given the bearings and distances as follows, viz.,—
 S. $16^{\circ} 30'$ E., 3 chains 47 links; and S., 17° E., 3 chains 2 links; S. 26° E., 5 chains 51 links; S. $31^{\circ} 30'$ E., 7 chains 34 links; S. $5^{\circ} 20'$ E., 10 chains 55 links; S. 15° E., 5 chains 9 links; S. 8° W., 4 chains 3 links; S., $3^{\circ} 30'$ E., 4 chains 70 links; S., $45^{\circ} 30'$ W., 6 chains 50 links; S. $64^{\circ} 45'$ W., 7 chains 34 links; and N., $1^{\circ} 21'$ E., 49 chains 18 links, to the place of beginning, to find their northings and southings, eastings and westings, and hence determine their correctness.

| Stations | Bearings | Distances ch. lks. | North | South | East | West |
|----------|------------------------|-----------------------|-------|-------|-------|-------|
| | | | Lat. | Lat. | Long. | Long. |
| 1 | S. $16^{\circ} 30'$ E. | 3.47 | | 3.33 | 0.98 | |
| 2 | S. 17° 0' E. | 3.02 | | 2.89 | 0.89 | |
| 3 | S. 26° 0' E. | 5.51 | | 4.94 | 2.41 | |
| 4 | S. $31^{\circ} 30'$ E. | 7.34 | | 6.25 | 3.84 | |
| 5 | S. $5^{\circ} 20'$ E. | 10.55 | | 10.49 | 0.97 | |
| 6 | S. 15° 0' E. | 5.09 | | 4.91 | 1.31 | |
| 7 | S. 8° 0' W. | 4.03 | | 3.99 | | 0.56 |
| 8 | S. $3^{\circ} 30'$ E. | 4.70 | | 4.69 | 0.28 | |
| 9 | S. $45^{\circ} 30'$ W. | 6.50 | | 4.56 | | 4.64 |
| 10 | S. $64^{\circ} 45'$ W. | 7.34 | | 3.13 | | 6.64 |
| 11 | N. $1^{\circ} 21'$ E. | 49.18 | 49.17 | | 1.16 | |
| | | 106.73 | 49.17 | 49.17 | 11.84 | 11.84 |

The northings being the same as the southings; the eastings the same as the westings.

It is seldom, however, that they agree so exactly, as has been given in this example. As the sum total of the distances amount to 106 chains 73 links, there would have been an allowable difference of 21 links (*i.e.*, of one link in every five chains) between the northings and southings, and the eastings and westings.

As errors will sometimes, notwithstanding the greatest care, occur in a survey of extent, we subjoin an example of two blocks of land, adjoining each other, having a common line, in which a considerable error was suspected, with the method of determining the locality of the error, and the means of correcting it..

EXAMPLE.—Given the bearings of the one block, S. $49^{\circ} 30'$ E., 7 chains 25 links; S. 73° E., 8 chains 82 links; N. 64° E., 15 chains 41 links; S. 68° W., 17 chains 84 links, to the place of beginning;

and the bearings of the second block, reversing the bearing of the last line, which is common to the two, N. 68° E., 17 chains 84 links; N. $67\frac{1}{4}$ ° E., 6 chains 37 links; N. $47\frac{1}{4}$ ° E., 3 chains 86 links; N. 76° W., 14 chains 63 links; S. 32° W., 19 chains 73 links, also returning to the place of beginning. It is required to calculate the northings and southings, eastings and westings, of each of them separately, as it is imagined an error has been somewhere committed, which it is required to discover.

1st. SURVEY.

(Having a common line with the second.)

| Stations | Courses | Distance in chains | North Lat. | South Lat. | East Dep | West Dep |
|----------|------------------------|-----------------------|---------------|---------------|--------------|-------------|
| 1 | S. $49^{\circ}30'$ E. | 7.55 | | 4.91 | 5.74 | |
| 2 | S. 73° E. | 8.82 | | 2.58 | 8.43 | |
| 3 | N. $6\frac{1}{4}$ ° E. | 15.41 | 15.32 | | 1.68 | |
| 4 | N. 68° W. | 17.84 | | 6.69 | | 16.54 |
| | | | 15.32 | 14.18 | 15.85 | 16.54 |
| | | | | 14.18 | | 15.85 |
| | | | | Excess of N. | Excess of W. | .69 |
| | | | | 2.14 | | |

There is some considerable error here.

2nd. SURVEY.

(Having a common line with the first.)

| Stations | Courses | Distances in chains | North Lat. | South Lat. | East Dep. | West Dep. |
|----------|-------------------------|------------------------|---------------|---------------|--------------|---------------------------|
| 1 | N. 68° E. | 17.84 | 6.58 | | 6.54 | |
| 2 | N. $67\frac{1}{4}$ ° E. | 6.37 | 2.61 | | 5.68 | |
| 3 | N. $47\frac{1}{4}$ ° E. | 3.86 | 2.61 | | 2.84 | |
| 4 | N. 76° W. | 14.63 | 3.54 | | | 14.19 |
| 5 | S. 32° W. | 19.73 | | 16.73 | 25.06 | 10.46 |
| | | | 15.44 | 16.73 | | 24.65 |
| | | | | 15.44 | 24.65 | |
| | | | | 1.29 | .41 | Excess of eastings. |
| | | | | | | |

There is a considerable error here also.

This error is southerly, the former was northerly; but the bearing to this line, in the present example, is the reverse of the former; as this line forms, in itself, in the first survey, the whole of the northings, and, in the second survey, the whole of the southings, and as it has, in each case, an excess of more than a chain, the

error will most probably be found in this line. To ascertain whether this is the case, make one survey of the two, by throwing out of it this common doubtful line, and proceed to test the correctness of the several other lines and bearings by the usual way.

3rd. SURVEY,

(*Being the union of the two, omitting the common line.*)

| Stations | Courses | Distances | North Lat. | South Lat. | East Dep. | West Dep. |
|----------|-------------------------|-----------|--------------|------------|--------------|-----------|
| 1 | S. $49\frac{1}{4}$ ° E. | 7·55 | | 4·91 | 5·74 | |
| 2 | S. 73° E. | 8·82 | | 2·58 | 8·43 | |
| 3 | N. $6\frac{1}{4}$ ° E. | 15·41 | 15·82 | | 1·68 | |
| 4 | N. $67\frac{1}{4}$ ° E. | 6·37 | 2·61 | | 5·68 | |
| 5 | N. $47\frac{1}{3}$ ° E. | 3·86 | 2·61 | | 2·84 | |
| 6 | N. 76° W. | 14·63 | 3·54 | | | 14·19 |
| 7 | S. 32° W. | 19·73 | | 16·73 | | 10·46 |
| | | 76·37 | 24·08 | 24·22 | 24·37 | 24·65 |
| | 5 76 | | | 24·08 | | 24·37 |
| | 15 | | Excess of S. | ·14 | Excess of W. | ·28 |

As 76·37 is the sum of the distances, the allowable error in this case is 15 links, that being one link in every chain of the sum of the distances.

The excess of southing is less than this, being ·14; that of the westings is more (·28), being, however, sufficiently near for our object in proving that there is no material error in any one of the lines that form the present block; and it may, therefore, be presumed that the bearing and distance, common to the two polygons, must be incorrect.

Having found the place of error, we will now consider the best method of correcting it.

CHAP. IV.

ERRORS OR OMISSIONS OF SURVEY.

As it is necessary, for the correction of this common line, when any two of the following data, viz.,—bearing, distance, difference of latitude, and of departure are given, to understand the method of determining the other two; we will first briefly say a few words on the subject.

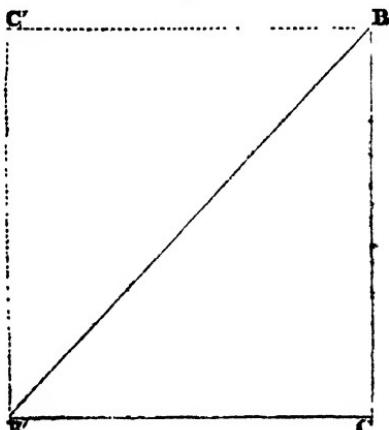
Any two of the above data being given, to determine the other two.

Let $B'B$, or BB' , be the distance of the given line; $BB'C'$, or $B'BC$, is the angle of bearing; BC , or $B'C'$, is the difference of latitude, and $B'C$, or BC' , is the departure; but BC , or $B'C'$, is the cosine of the angle $B'BC$, or $BB'C'$, which is the angle of bearing; and $B'C$, or BC' , is the sine, also, of the same angle of bearing; therefore, the latitude, departure, and distance, are respectively the cosine, sine, and radius of a circle, whose arc is measured by the angle of bearing. By the rules for the cases of right-angled triangles, given in the Chapter on Trigonometre, in the Introduction, having two terms given, the rest can be found.

EXAMPLE 1.—A line bears N. 45° E., 31 chajns, 40 links; required its difference of latitude and departure.

$$\begin{array}{rcl}
 \text{As sine } 90^\circ & = & 10\cdot00000 \\
 \text{to } 31\cdot41 \text{ chs.} & = & 1\cdot49693 \\
 \text{so is sin. } 45 & = & 9\cdot84949 \\
 & & \underline{11\cdot34642} \\
 & & \underline{10\cdot00000}
 \end{array}$$

$$\begin{array}{rcl}
 22 \text{ chs. } 20 \text{ lks. N. lat.} & = & \underline{1\cdot34642} \\
 \text{sine } 45^\circ = \cos. 45^\circ \therefore \text{dep.} & = & \text{lat.}
 \end{array}$$



EXAMPLE 2.—The difference of latitude of a line was 4 chs. 40 lks. N. ; the departure was 25° W. ; required the distance of the line.

Ans. $25\cdot57$ chs.

EXAMPLE 3.—A line bears N. 72° W. ; its departure is 24 chs. 58 lks. : what is its length ?

Ans. 25.84 chs.

EXAMPLE 4.—A line is 38 chs. 45 lks. in length, and its departure is 14 chs. W. ; what is its angle of bearing, the same being between the north and west ?

$$\begin{array}{rcl}
 \text{As } 38\cdot45 \text{ chs.} & = & 1\cdot58490 \\
 \text{is to rad.} & = & 10\cdot00000 \\
 \text{so is } 14 \text{ chs.} & = & \underline{1\cdot14613} \\
 & & 11\cdot14613 \\
 \text{sine angle of bearing} & = & \underline{1\cdot58490} \\
 21^{\circ} 21 & = & 9\cdot56123
 \end{array}$$

To correct the fourth station, which is common to the two polygons.

RULE.—Add up the northings and southings, and subtract them for the northing or southing of the unknown line ; do the same with the eastings or westings to obtain its departure ; then, by the preceding note having the departure and latitude of a line given, the distance and bearing can be determined.

| Stations | Courses | Distances | North Lat. | South Lat. | East Dep. | West Dep. |
|----------|-----------------------|---------------|---------------|------------|-----------|-------------------------|
| 1 | S. $49^{\circ}30'$ E. | c. l. 7 55 | | 4.91 | 5.74 | |
| 2 | S. 73° E. | 8 82 | | 2.58 | 8.43 | |
| 3 | N. 64° E. | 15 41 | 15.32 | | 1.68 | |
| 4 | S. W. | | | | | |
| | Deficiency of S. | | 15.32 7.49 | 7.49 | 15.85 | Deficiency of westings. |
| | | | 7.83 | | 15.85 | |

Now AC, the westing or base of the right-angled triangle, being given 15 chs. 85 lks.; and BC, the southing or perpendicular, being 7 chs. 83 lks.; the angle BAC, or the bearing of the line AB, and its distance, which are required, can be determined by the rules of the bases of right-angled triangles.

Thus—take AC as radius, and describe the circle CD; then CB becomes the tangent to angle BAC, and AB is the secant of the same angle; then say—

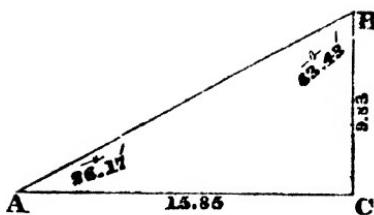
$$\begin{array}{ll}
 \text{As } 15\cdot85 & = 1\cdot20003 \\
 \text{is to radius} & = 10\cdot00000 \\
 \text{so is } 7\cdot83 & = 0\cdot89376 \\
 & = 10\cdot89376 \\
 & = 1\cdot20003 \\
 \text{to tan. angle BAC, } 26^\circ 17' & = 9\cdot69373 \\
 \\
 \text{Also, as radius} & = 10\cdot00000 \\
 \text{is to } 15\cdot85 & = 1\cdot20003 \\
 \text{so is secant } \angle A, 26^\circ 17' & = 10\cdot04739 \\
 \text{to hypotenuse, } 17\cdot68 & = 1\cdot24742 \\
 \text{and the bearing in S. } 63^\circ 45' \text{ W.} &
 \end{array}$$

CORRECTED SURVEY.

| Stations | Courses | Distances | N. | S. | E. | W. |
|----------|-------------------------|--------------|-------|-------|-------|---------------------|
| 1 | N. $63\frac{3}{4}$ ° E. | 17·68 | 7·83 | | 15·85 | |
| 2 | N. $67\frac{3}{4}$ ° E. | 6·37 | 2·61 | | 5·68 | |
| 3 | N. $47\frac{1}{2}$ ° E. | 3·86 | 2·61 | | 2·84 | |
| 4 | N. 76 ° W. | 14·63 | 3·54 | | | 14·19 |
| 5 | S. 32 ° W. | 19·73 | 16·73 | | | 10·46 |
| | | 62·27 | 16·59 | 16·73 | 24·37 | 24·65 |
| | | | | 14·59 | | 24·37 |
| | | Excess of S. | ·14 | | ·28 | Excess of westings, |

which are the same errors as before.

Let us now divide these errors proportionately among the several distances, by the method previously given, viz.,—by saying, as the whole distance is to the whole error, so is each distance to its particular correction.



CORRECTED SURVEY.

| Stations | Bearings | Distances | N. | S. | E. | W. | Corrected Land for Gees. Corrections for Gees. | Corrected N. | Corrected S. | Corrected E. | Corrected W. |
|----------|----------|-----------|------|-------|-------|-------|--|-----------------|-----------------|-----------------|-----------------|
| 1 | N.63° E. | 17.68 | 7.83 | | 15.85 | | 1 | 5 | 17.84 | | 15.90 |
| 2 | N.67° E. | 6.37 | 2.61 | | 5.68 | | 0 | 2 | 2.61 | | 5.70 |
| 3 | N.47° E. | 3.86 | 2.61 | | 2.84 | | 0 | 1 | 2.61 | | 2.85 |
| 4 | N.76° W. | 14.63 | 3.54 | | | | 14.19 | 0 | 4 | 3.54 | 14.15 |
| 5 | S.32° W. | 19.55 | | 16.57 | | | 10.36 | 1 | 6 | | 10.30 |
| | | 62.09 | | 16.59 | 16.57 | 24.37 | 24.55 | 2 | 18 | 16.58 | |
| | | | | 16.57 | | | 24.37 | | | 16.58 | 24.45 |
| | | | | | | | dif. E. • 18 | | | | 24.45 |
| | | | | | | | dif. S. • 02 | | | | |

As the whole distance 62 : 18 : 1

$$\frac{1}{62} \mid \frac{18.000}{18.000} \mid 290 = 3 \text{ links nearly}$$

$$\frac{1}{124} \mid \underline{\underline{124}} \mid \underline{\underline{560}}$$

$$\frac{5}{5} \mid \underline{\underline{5}} \mid \underline{\underline{558}}$$

18 deficiency.

EXAMPLES.

1.—Given the following bearings and distances of an old waggon road, through a new settlement, running from one concession road to another, viz.,—1st. N. 78° E., 2·20 chains; 2nd. N. $45^{\circ} 30'$ E., 14·80 chains; 3rd. N. 16° W., 21·75 chains; 4th. N. $68^{\circ} 20'$ E., 13·90 chains; 5th. N. 10° W., 15·60 chains; 6th. N. 70° E., 8·96 chains. It is required to lay out a new straight road, connecting the two ends of the present road. What will be its bearing and distance?

2.—Given the boundaries of a tract of land, as follows, viz.:—1st. S. $16^{\circ} 30'$ E., 3·47 chains; 2d. S. 17° E., 3·02 chains; 3d. S. 26° E., 5·51 chains; 4th. S. $31^{\circ} 30'$ E., 7·34 chains; 5th. S. $5^{\circ} 20'$ E., 10·55 chains; 6th. S. 15° E., 5·09 chains; 7th. S. 8° W., 4·03 chains; 8th. S. $3^{\circ} 30'$ E., 4·70 chains; 9th. S. $45^{\circ} 30'$ W., 6·50 chains; 10th. S. $64^{\circ} 45'$ W., 7·34 chains; 11th. to the place of beginning. Required the bearing and distance of this eleventh line.

Ans. N. $1^{\circ} 21'$ E., 49 chs. 18 lks.

When the distance of two sides cannot be obtained.

RULE 1.—Find, by the preceding chapter, the length and bearing of the closing line, connecting the known sides of the survey. This line and the two unknown sides will form a triangle, having a known base, and the bearings known also of all its sides; with these data compute the several angles of the triangle, by the previous rule (page 205), and you have the three angles and one side given; whence the two required sides are determinable by the first case.

EXAMPLE 1.—The boundaries of a tract of land were taken as follows:—1st. S. $16^{\circ} 30'$ E.; 2nd. S. 17° E.; 3d. S. 26° E., 5·51 chains; 4th. S. $31^{\circ} 30'$ E., 7·34 chains; 5th. S. $5^{\circ} 20'$ E., 10·55 chains; 6th. S. 15° E., 5·09 chains; 7th. S. 8° W., 4·03 chains; 8th. S. $3^{\circ} 03'$ E., 4·70 chains; 9th. S. $45^{\circ} 30'$ W., 6·50 chains; 10th. S. $64^{\circ} 45'$ W., 7·34 chains; 11th. N. $1^{\circ} 21'$ E., 49·18 chains, to the place of beginning. Required the distances of the two first stations, which, from local obstructions, could not be measured.

| Stations | Bearings | Distances | N. | S. | E. | W. |
|----------|-----------------------|-----------------------|-------|-------|------|-----------------------|
| 1 | S. $16^{\circ}30' E.$ | | | | | |
| 2 | S. 17° E. | | | | | |
| 3 | S. 26° E. | 5.51 | | 494 | 241 | |
| 4 | S. $31^{\circ}30' E.$ | 7.34 | | 6.25 | 3.84 | |
| 5 | S. $5^{\circ}20' E.$ | 10.55 | | 10.49 | 0.97 | |
| 6 | S. 15° E. | 5.09 | | 4.91 | 131 | |
| 7 | S. 8° W. | 4.03 | | 3.99 | | 0.56 |
| 8 | S. $8^{\circ}30' E.$ | 4.70 | | 4.69 | 0.28 | |
| 9 | S. $45^{\circ}30' W.$ | 6.50 | | 4.56 | | 4.64 |
| 10 | S. $64^{\circ}45' W.$ | 7.34 | | 3.13 | | 6.64 |
| 11 | N. $1^{\circ}21' E.$ | 49.18 | 49.17 | | 1.16 | |
| | | | 49.17 | 42.95 | 9.97 | 11.84 |
| | | | 42.95 | | | 9.97 |
| | | deficiency of S. 6.22 | | | | 1.87 deficiency of E. |

Therefore,
 the southings $6.22 = \text{cosine of angle of bearing}$ } where radius = the
 the eastings $1.87 = \text{sine of angle of bearing}$ } distance of the
 closing line.

By the case of right-angled triangles.

| | |
|---|------------|
| As 6.22 chs. | = 2.79379 |
| is to rad. | = 10.00000 |
| so is 1.87 chs. | = 2.27184 |
| to tan. angle of bearing $16^{\circ} 44'$ | = 9.47805 |

| | |
|----------------------------------|------------|
| As cosine of angle of bearing | = 9.98121 |
| is to 6.22 chs. | = 2.79379 |
| so is radius | = 10.00000 |
| to distance required = 6.49 chs. | = 2.81258 |

The angle of bearing is 16 degrees 44 minutes, which, as the deficiencies are southings and eastings, makes the closing line bear south 16 degrees 44 minutes east, 6 chains 49 links.

This closing line and the two unmeasured lines form a triangle. Draw any line AB, marking upon it the bearing S. $16^{\circ} 44' E.$, and AC, BC, in their respective relative position to AB, viz.,—AC, bearing S. $16^{\circ} 30' E.$, and CB, S. $17^{\circ} E.$.

Because AB and AC bear both southwards and eastwards, the angle CAB, is their difference; for the same reason, the angle ABC

is the difference of the bearings of AB and CB. The sides AC and CB can now be calculated by the first case of trigonometry.

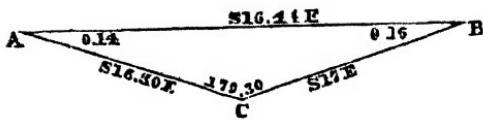
$$\begin{array}{ll}
 \text{As sine } 179^\circ 30' & = 7.94084 \\
 \text{is to } 6.49 \text{ chs.} & = 2.81224 \\
 \text{so is sine } 0^\circ 16' & = \underline{\underline{7.66784}} \\
 & 10.48008 \\
 \text{to } x 3.46 \text{ chs.} & = \underline{\underline{2.53924}} \\
 \text{and so is sine } 0^\circ 14' & = \underline{\underline{7.60985}} \\
 & 10.42209 \\
 \text{to } y 3^\circ 02' & = 2.48125
 \end{array}$$

being 3.46, instead of 3.47 (see page 221, example), a difference of one link only in the first distance; and being 3.02, the same as the second distance.

EXAMPLE 2.—The following bearings and distances were taken, viz.:—at station 1, the bearing of 2nd station was, S. $71^\circ 24'$ W.; at station 3, the same point bore S. $46^\circ 18'$ E., the distances could not be obtained; at station 3, N. $15^\circ 42'$ E., 6.20 chains; at station 4, N. $52^\circ 18'$ E., 6.75 chains; at station 5, S. $78^\circ 48'$ E., 5.96 chains; at station 6, $5^\circ 51'$ E., 4.84 chains; at station 7, S. $49^\circ 15'$ W., 4.75 chains; at station 8, S. $4^\circ 57'$ E., 3.98 chains, to the place of beginning.

What are the distances of the first and second lines?

RULE 2.—By means of a changed bearing.—Suppose the whole tract of land to revolve, until it becomes in such a position, as to have one of the unknown sides in a direct meridian line. This will not change the relative position of the sides, as, whatever angle of variation be taken for this line, the same will be taken for all; but, as in the first instance, there



were but two lines, whose departures were unknown, so now, in consequence of one, from having been made due north and south, having no departure, that, of the only one that now remains unknown, is the measure of the difference of the sums of the eastings or westings.

The changing of the bearing is performed in the same way as correcting for the variation, viz.,—by bringing all the given angles to a *new* meridian, which makes so many degrees east or west, with the first meridian.

Arrange, therefore, the bearings and distances, as in the following columns; change the bearings of each, so as to make the bearing of one of the unknown sides a meridian line. Place these changed bearings in their proper column; find the latitude and departure of these bearings, to their several distances. Find the sum of the eastings and the sum of the westings; their difference will be the departure of the second unknown side, in terms of the deficiency.

Calculate the latitude and distance of this second side, from its bearing and departure, which are known, and place them also in their proper column.

Then add the northings together, and the southings together, and their difference will be the distance of the side, which was made a meridian.

EXAMPLE.—Solve the preceding Problem, by using a changed bearing.

Make the first bearing S. $16^{\circ} 30'$ E., a meridian, turning all the southerly bearings $16^{\circ} 30'$ to the west, and the northerly bearings $16^{\circ} 30'$ to the east.

| Stations | Bearings | Changed | Dis. | N. | S. | E. | W. |
|----------|------------------------|------------------------|-------|-------|--------|--------|-------|
| 1 | S. $16^{\circ} 30'$ E. | South | | | | 0.00 | 0.00 |
| 2 | S. 17° E. | S. $0^{\circ} 30'$ E. | | | (1.26) | (0.11) | |
| 3 | S. 26° E. | S. $9^{\circ} 30'$ E. | 5.51 | | 5.43 | .91 | |
| 4 | S. $31^{\circ} 30'$ E. | S. $15^{\circ} 00'$ E. | 7.34 | | 7.09 | 1.90 | |
| 5 | S. $5^{\circ} 20'$ E. | S. $11^{\circ} 10'$ W. | 10.55 | | 10.35 | | 2.06 |
| 6 | S. 15° E. | S. $1^{\circ} 30'$ W. | 5.09 | | 5.09 | | 0.13 |
| 7 | S. 8° W. | S. $24^{\circ} 30'$ W. | 4.03 | | 3.67 | | 1.67 |
| 8 | S. $3^{\circ} 30'$ E. | S. 13° W. | 4.70 | | 4.58 | | 1.06 |
| 9 | S. $45^{\circ} 30'$ W. | S. 62° W. | 6.50 | | 3.06 | | 5.74 |
| 10 | S. $64^{\circ} 45'$ W. | S. $81^{\circ} 15'$ W. | 7.34 | | 1.11 | | 7.26 |
| 11 | N. $1^{\circ} 21'$ E. | N. $17^{\circ} 51'$ E. | 49.18 | 46.84 | | 15.00 | |
| | | | | | | 17.81 | 17.92 |

$$\text{Eastings} = 17.81$$

$$\text{Westings} = 17.92$$

$$\text{Deficiency of eastings} = 0.11$$

Place this in its proper column, against the bearing, S. $0^{\circ} 30'$ E., and find the southing of the line.

Given the bearing of the line, S. $0^{\circ} 30'$ E., and the easting, 11 links, to calculate the distance and the southing.

Ans. The distance and the southing each 1.26 chs.

Now make the second bearing a meridian.

| Station | Bearings | Changed Bearing | Distance | N. | S. | E. | W. |
|---------|------------------------|------------------------|----------|-------|----------------------------|---------|-------|
| 1 | S. $16^{\circ} 30'$ E. | S. $0^{\circ} 30'$ W. | | | | (0.04) | |
| 2 | S. 17° E. | South. | | | | | |
| 3 | S. 26° E. | S. 9° E. | 5.51 | | 5.44 | 1.84 | |
| 4 | S. $31^{\circ} 30'$ E. | S. $14^{\circ} 30'$ E. | 7.34 | | 7.11 | | |
| 5 | S. $5^{\circ} 20'$ E. | S. $11^{\circ} 40'$ W. | 10.55 | | 10.55 | | 2.15 |
| 6 | S. 15° E. | S. 2° W. | 5.09 | | 5.09 | | 0.17 |
| 7 | S. 8° W. | S. 25° W. | 4.03 | | 3.66 | | 1.70 |
| 8 | S. $3^{\circ} 30'$ E. | S. $13^{\circ} 30'$ W. | 4.70 | | 4.57 | | 1.09 |
| 9 | S. $45^{\circ} 30'$ W. | S. $62^{\circ} 30'$ W. | 6.50 | | 3.00 | | 5.76 |
| 10 | S. $64^{\circ} 45'$ W. | S. $81^{\circ} 45'$ W. | 7.34 | | 1.05 | | 7.27 |
| 11 | N. $1^{\circ} 21'$ E. | N. $18^{\circ} 21'$ E. | 49.18 | 46.71 | | 15.40 | |
| | | | | | | 18.10 | 18.14 |
| | | | | | Excess of westings 4 links | | |

When the difference above was 11, which is the sine of the angle of bearing, 30 minutes, the distance, or the radius to that angle, was only 12·6, it cannot now, therefore, be more.

Hence, when the two unknown bearings differ so slightly, as to make the changed bearing of one but a few minutes, this method cannot be adopted; the first method, by the closing line, must, therefore, be used.

I subjoin a correct result by the changed bearings, obtained from the same example, in consequence of the angles of the two bearings, whose distances are unknown, differing considerably.

EXAMPLE.—Make the *seventh* bearing south, and change the other bearings to correspond. The changed bearings have been omitted for practice to the student, but their latitudes and departures are inserted.

| Stations | Bearings | Changed Bearing. | Distances | N, | S. | E. | W. |
|-------------------------|---------------|------------------|-----------|---------|------------------|-------|-------|
| 1 | S. 16° 30' E. | | 3·47 | | 3·15 | 1·43 | |
| 2 | S. 17° E. | | 3·03 | | 2·73 | 1·27 | |
| 3 | S 26° E. | | 5·51 | | 4·51 | 3·08 | |
| 4 | S. 31° 30' E. | | 7·34 | | 5·66 | 4·66 | |
| 5 | S. 5° 20' E. | | 10·55 | | 10·26 | 2·41 | |
| 6 | S. 15° E. | { 5·08 | | (4·67) | (1·98) | | |
| 7 | S. 8° W. | { 4·03 | | (4·04) | | | |
| 8 | S. 3° 30' E. | | 4·70 | | 4·60 | 0·96 | |
| 9 | S. 45° 30' W. | | 6·50 | | 5·15 | | 3·95 |
| 10 | S. 64° 45' W. | | 7·34 | | 4·02 | | 6·13 |
| 11 | N. 1° 21' E. | | 49·18 | 48·83 | 48·83 | 13·81 | 15·70 |
| | | | | 44·11 | 44·11 | | 13·81 |
| Deficiency of southings | | | | 4·02 | deficiency of E. | | 1·98 |

| | | |
|-----------------|---|----------|
| As sine 23° | = | 9·59188 |
| is to 1·98 chs. | = | 2·29667 |
| so is radius | = | 10·30000 |
| | | 12·29667 |
| | | 9 59188 |
| to 5·08 chs. | = | 2·70479 |
| As radius | = | 10·00000 |
| is to 5·06 chs. | = | 2·70479 |
| so is sin 67° | = | 9·97403 |
| | | 12·66882 |
| | | 10·00000 |
| to 4·67 chs. | = | 2·66882 |

When the bearings of any two sides of a tract of Survey have been incorrectly taken.

Arrange the given bearings and distances in their proper columns, and find, as before, the difference of the northings and southings, and of the eastings and westings, for the latitude and departure of a closing line. The line, and the distances of the two unknown sides, form a triangle, with sufficient data to determine its inward angles ; and thence, as the bearing of the closing line is known, the required bearings of the two other sides are obtained.

CHAP. V.

Given the bearings and distances of a tract of land, to determine its area.

Arrange the several bearings and distances as before, and find the respective latitudes and departures corresponding to each.

Determine, as in the preceding Problems, the several corrections to each, for latitude and departure, and allow for these corrections, placing the amended latitudes and departures in their proper columns.

Now make five more columns, and head them—East Double Departure ; West Double Departure ; Multipliers ; North Areas ; and South Areas.

Take the sum or difference of every two consecutive departures ; adding them, if of the same kind, and subtracting them, if of different ; and place this sum, or excess, in the column to which it belongs ; of west, in the column of W.D.D. ; of east, in that of E.D.D.

If the sum of the first and last departures, east or west, be found equal to the first corresponding double departure, east or west, the working has been correctly performed.

Commence from any angle whatever of the survey, and assume any line, either close to, or at any distance from it, as a multiplier, and place it in the column of multipliers collateral with that angle, terming it either east or west, according as the principal double departures are east or west. The sum of this assumed multiplier, and that of the double departure, corresponding to the next side, if they are of the same kind, or their difference, if of opposite kinds, in terms of the greater, will be the next multiplier ; proceed with the second multiplier, and the following double departure, until multipliers have been found for all the sides.

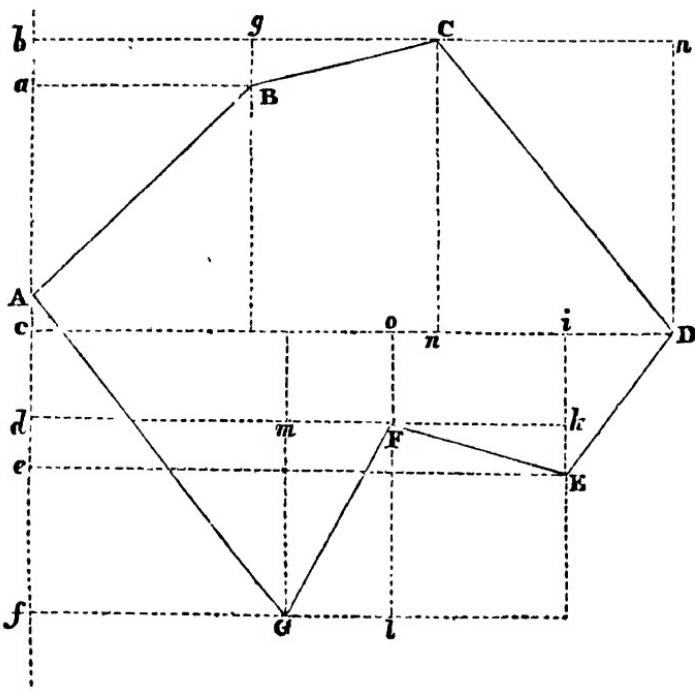
The difference, if of opposite kinds, or the sum, if of the same, of the last multiplier and the first double departure, will be equal to the assumed multiplier, if the work is right.

Multiply each of the corrected differences of latitude by its collateral multiplier, and place the product in the north or south area columns, observing, that whenever the multipliers are of the same name as the assumed, those products are to be placed in the column of areas, which is of the same name as the latitude ; if of different names, in the opposite column.

Half the difference, between the sum of the north and south areas will be the area of the survey.

DEMONSTRATION.

Let ABCDEFG be a tract of country whose area is required;



through A draw a meridian line fb , and through the several points, B, C, D, E, F, and G, draw Ba , Cb , Dc , Ee , Fd , and Gf , perpendicular to this meridian, and also gB , Cn , Dh , and C , parallel to it.

Let the assumed multiplier be AB .

Now the area of the figure, $bCDEFGf$, is equal to the figure $bCBAGf$ + the required area of the tract. And the difference of latitude and departure of the several sides will be found as in the following table. Thus Aa is the northing of AB , and aB is its easting; ab is the northing of BC , and gC its easting, and so on.

Also, according to the rule, $aB+gC$ or bC , is the east north departure of BC , as they are of the same kind, and $fG-aB$, is the W.D.D. of aB .

Having obtained the double departures for the several sides, assume the multiplier aB , and find, by the rule, the several other multipliers $ab+bc$, bl , observing carefully, whether these multipliers

are east or west. And place the areas of the product of the multipliers into their collateral northings or southings, in the columns of north or south areas, according to the proper relations between them ; multipliers and northings is given in the rule. These areas are double the required areas—being each of them double the areas of the several trapezoids into which the figure has been divided.

Half the difference, therefore, of the north and south areas, will be the required area.

| | N. | S. | E. | W. | E. D. D. | W. D. D. | Multiplier | N. Areas | S. Areas |
|----|----|----|----|----|----------|----------|--------------|----------|----------|
| AB | Aa | | aB | | | fg—aB | a B E | | |
| BC | ab | | gC | | bC | | B bc E | 2 abCB | |
| CD | | bc | Ch | | gh | | bC+cD E | | 2 bCDc |
| DE | | ce | | Di | hi | | cD+ci (cE) E | | 2 cDEe |
| EF | ed | | | kF | | Do | eE+dF E | dFEE | |
| FG | | df | | Fm | | km | dF+dm (fg) E | | 2 dFGf |
| GA | Af | | | Gf | | co | dm E | AGf | |

Now “ dm ” the multiplier last found—the first W. D. D., i. e. dm or $fg—(fg—aB) = aB$, the assumed multiplier.

EXAMPLE I.—Given the bearings and distances of a tract of country, to find the area, viz.:—1st, S. $16^{\circ} 30'$ E., 3·47 chains ; 2nd, S. 17° E., 3·02 chains ; 3rd, S. 26° E., 5·51 chains ; 4th, S. $31^{\circ} 30'$ E., 7·34 chains ; 5th, S. $5^{\circ} 20'$ E., 10·55 chains ; 6th, S. 15° E., 5·09 chains ; 7th, S. 8° W., 4·03 chains ; 8th, S. $3^{\circ} 30'$ E., 4·70 chains ; 9th, S. $45^{\circ} 30'$ W., 6·50 chains ; 10th, S. $64^{\circ} 45'$ W., 7·34 chains ; 11th, S. $60^{\circ} 15'$ W., 4·98 chains ; 12th, N. $22^{\circ} 45'$ W., 27·62 chains ; 13th, N. 67° E., 2·40 chains ; 14th, N. $13^{\circ} 30'$ W., 6·10 chains ; 15th, S. $62^{\circ} 30'$ W., 4·16 chains ; 16th, N. $27^{\circ} 30'$ W., 8·24 chains ; 17th, N. $41^{\circ} 10'$ E., 4·47 chains ; 18th, N. 4° W., 16·40 chains ; 19th, N. 84° E., 4·03 chains ; 20th, S. $69^{\circ} 30'$ E., 18·21 chains—to the place of beginning.

| Bearings | Dist. | N. Lat. | S. Lat. | E. Long. | W. Long. | Cotangent of Latitude | Cor. N. | Cor. E. | Cor. W. | Cor. R. | Multiple | N. Areas | S. Areas | |
|-----------------|--------|---------|---------|----------|----------|--------------------------|---------|---------|---------|---------|----------|----------|----------|------------|
| | | | | | | | | | | | | | | |
| 1 S. 16°30' E. | 3.47 | 3.33 | 0.98 | — | — | 3.33 | 0.98 | — | — | 18.04 | 00.00 | E. | 000 | 54043 |
| 2 S. 17° E. | 3.02 | 2.89 | 0.89 | — | — | 2.89 | 0.89 | — | — | 1.87 | — | E. | 255398 | 713750 |
| 3 S. 26° E. | 5.51 | 4.95 | 2.41 | — | — | 4.94 | 2.41 | — | — | 5.17 | — | E. | 1702537 | 908841 |
| 4 S. 31°30' E. | 7.34 | 6.26 | 3.84 | — | — | 6.25 | 3.84 | — | — | 6.25 | — | E. | 708474 | 890162 |
| 5 S. 5°20' E. | 10.55 | 10.51 | 0.97 | — | — | 10.49 | 0.97 | — | — | 4.87 | — | E. | 665210 | 104542 |
| 6 S. 15° E. | 5.09 | 4.92 | 1.31 | — | — | 4.91 | 1.31 | — | — | 2.28 | — | E. | 18.7698 | 18.7698 |
| 7 S. 8° W. | 4.03 | 3.99 | — | 0.56 | — | 3.99 | — | 0.56 | — | 0.75 | — | E. | 5777728 | 5777728 |
| 8 S. 3°30' E. | 4.70 | 4.67 | 0.28 | — | — | 4.69 | 0.28 | — | — | 0.28 | 18.98 | E. | 292434 | 1801008 |
| 9 S. 45°30' W. | 6.50 | 4.56 | — | — | — | 4.64 | — | — | — | 4.36 | 14.62 | E. | 680856 | 3141744 |
| 10 S. 64°45' W. | 7.34 | 3.14 | — | — | — | 6.64 | — | — | — | 6.64 | — | E. | 1470672 | 6878883 |
| 11 S. 60°15' W. | 4.98 | 2.47 | — | — | — | 4.33 | — | — | — | 4.33 | — | E. | 16.0351 | 16.0351 |
| 12 N. 22°45' W. | 27.62 | 25.47 | — | — | — | 25.52 | — | — | — | 10.68 | — | W. | 18.04 | 18.04 |
| 13 N. 67° E. | 2.40 | 0.94 | — | — | — | 0.94 | — | — | — | 2.21 | — | W. | 847 | 31.11 |
| 14 N. 13°30' W. | 6.10 | 5.93 | — | — | — | 1.42 | 1 | 5.94 | — | 1.42 | 0.79 | W. | 30.32 | 1801008 |
| 15 S. 62°30' W. | 4.16 | 1.92 | — | — | — | 3.69 | — | 1.92 | — | 3.69 | — | W. | 5.11 | 35.43 |
| 16 N. 27°30' W. | 8.24 | 7.31 | — | — | — | 3.80 | 1 | 7.32 | — | 3.80 | — | W. | 7.4 | 42.92 |
| 17 N. 41°10' E. | 4.47 | 3.86 | — | — | — | 2.95 | — | 3.56 | — | 2.95 | — | W. | 0.85 | 43.77 |
| 18 N. 4° W. | 16.40 | 16.36 | — | — | — | 1.15 | 3 | 16.39 | — | 1.15 | 1.80 | W. | 41.97 | 41.97 |
| 19 N. 84° E. | 4.03 | 0.41 | — | — | — | 0.41 | — | — | — | 4.01 | — | W. | 2.86 | 39.11 |
| 20 S. 69°30' E. | 18.21 | — | — | — | — | 4 | — | — | — | 6.33 | 17.06 | W. | 21.07 | 1141932 |
| | 154.16 | 59.78 | 60.00 | 36.91 | 36.91 | 59.88 | 59.88 | 36.91 | 36.91 | 36.91 | 36.91 | W. | 2009880 | 25585761 |
| | 59.78 | — | — | — | — | — | — | — | — | — | — | W. | 2009880 | 2923575881 |

22 deficiency of northings.

31 links of allowable error.

515416

Ans. = 117 3 20

Required the area of a tract of land, whose bearings and distances are as follows, viz.:—1st, N. $15^{\circ} 42'$ E., 6·20 chains; 2d, N. $52^{\circ} 18'$ E., 6·75 chains; 3d, S. $78^{\circ} 48'$ E., 5·96 chains; 4th, S. $5^{\circ} 51'$ E., 4·84 chains; 5th, S. $49^{\circ} 15'$ W., 4·75 chains; 6th, S. $4^{\circ} 57'$ E., 3·98 chains; 7th, S. $71^{\circ} 24'$ W., chains; and 8th, N. $46^{\circ} 18'$ W., to the place of beginning.

Given the following bearings and distances to find the area, viz.:—
 1st, N. $10^{\circ} 21'$ W., 4·50 chains; 2d, N. $9^{\circ} 48'$ E., 5·20 chains;
 3d, N. 75° W., 3·00 chains; 4th, N. $20^{\circ} 3'$ E., 4·86 chains; 5th,
 S. 45° E., 5·20 chains; 6th, N. $3^{\circ} 18'$ W., 2·50 chains; 7th, E.
 700 chains; 8th, S. 12° W., 3·94 chains; 9th, S. 43° E., 4·15 chs.;
 10th, S. $46^{\circ} 57'$ W., 8·20 chains; 11th, S. 29° E., 9·15 chains;
 12th, S. 48° W., 4·56 chains; 13th, N. 19° W., 3·42 chains; 14th,
 S. 28° W., 8·54 chains; 15th, N. 53° W., 4·60 chains; and 16th,
 —,—, to the place of beginning.

CHAP. VI.

DIVISION OF LAND.

PROBLEM I.

To lay out a given quantity of land in a square form.

Bring the given quantity into square chains, and take the root which will be in chains.

EXAMPLE 1.—It is required to lay out 400 acres of land, in a square form.

$$400 \text{ acres} = 4000 \text{ square chains.}$$

$$\sqrt{4000} = 63\cdot24 \text{ chs. the length of the side required.}$$

EXAMPLE 2.—What will be the side of a square, which contains 496 acres, 2 roods, and 32 perches?

PROBLEM II.

To lay out a given area in a rectangular form, having the length to the breadth in a given ratio.

Let A = the area in square chains, and x = the common unit measure of the sides, and m and n the ratio ; then, mx = one side, and nx = the other, and $mnx^2 = A$, and $x = \sqrt{\frac{A}{mn}}$ or the unit of measure of the sides = the root of the given area divided by the product of the ratios.

EXAMPLE 1.—There are 2000 acres of land to be laid out in a rectangular form, whose sides are to each other, as 4 is to 5 ; what will their lengths be ?

$$m = 4, n = 5, \text{ and } mn = 20. \quad A = 20,000 \text{ sq. chains}$$

$$\therefore x = \sqrt{\frac{20000}{20}} = \sqrt{1000} = 31\cdot62 \text{ chs.}$$

$$4x = 31\cdot62 \times 4 = 126\cdot48 \text{ chs.} = \text{one side}$$

$$5x = 31\cdot62 \times 5 = 158\cdot10 \text{ chs.} = \text{other side}$$

EXAMPLE 2.—One side of a rectangular field is double the other, what are the sides, when the area is 20·0·18 perches.

$$Ans. 10\cdot03 \text{ chs. and } 20\cdot06 \text{ chs.}$$

EXAMPLE 3.—A man has a farm of 150 acres, of a rectangular form, the depth of the farm is 50 chains. He is desirous of adding 50 acres to it, having the same depth, to make up 200. What will be the length of frontage of his farm, after the addition ?

$$Ans. \text{ Half a mile.}$$

PROBLEM III.

When one of the sides is a certain length longer than the other.

Let x be the unit of measure as before, and m the excess of the one side, then x and $x+m$ will be the two sides, and their product, or x^2+mx , will be the given area (A) in square chains : completing the equation we have $x = \sqrt{A + \frac{m^2}{2}} - \frac{m}{2}$; or the following rule,

To obtain the smaller side (x), square half the difference of the two sides, add it to the given area, and taking the root of their sum, from this root subtract half the difference.

EXAMPLE.—Given 464 acres, it is required to lay it out in a rectangular form, the one side being 6 chains longer than the other.

Let x = the one side, $x+6$ = the other

$$\begin{array}{rcl} x^2 + 6x & = & 4640 \text{ square chains} \\ x^2 + 6x + 9 & = & 4649 \\ x + 3 & = & 68.18 \\ x & = & 65.18 \\ x + 6 & = & 71.18 \end{array}$$

Ans. One side = 65.18 chs.; the other = 71.18 chs.

PROBLEM IV.

From a given block of land, with parallel sides, the angle of inclination of the front and sides being given, to cut off a given area, by a line parallel to the sides.

RULE.—Find first the perpendicular depth of the given block, (which is the nat. sin. of the angle of inclination \times the length of the side), then, as all parallelograms upon equal bases, and between the same parallels, are equal, divide the given area by this depth for the frontage required.

EXAMPLE 1.—The concession road, of a certain township, bears N. 74° E., while the side lines bear N. 9° W. The length of the side lines is 66.66 chains, and the fronts of the lots are 30 chains. Required the frontage that must be taken, to cut off 100 acres, by a line parallel to the side lines.

The side lines, bearing N. 9° W., and the fronts, N. 74° E., the included angle, or angle of inclination, of the front to the sides, is $9^\circ + 74^\circ$ or 83° .

\therefore the depth of the lots equals nat. sin. 83° into 66.66 chs.

$$\begin{array}{rcl} \log. \sin. 83^\circ & = & 9.99675 \\ + \log. 66.66 & = & \underline{\underline{1.82387}} \\ & & 11.82062 \\ - \log. \text{radius} & = & \underline{-10} \\ \log. \text{perpend. depth } 66.66 & = & 1.82062 \end{array}$$

$$100 \text{ acres} = 1000 \text{ chains}$$

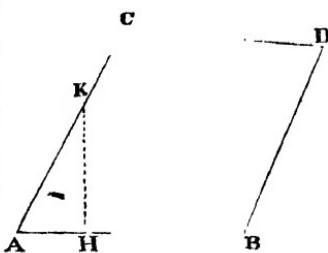
$$\frac{1000}{66.16} = 15.12 = \text{the frontage required.}$$

EXAMPLE 2.—When the fronts of the lots bear S. 80° W., and the side lines N. 15° E., the side lines being 101·50 chains long, and the concession road frontage 20·00 chains, what will be the frontage required, to divide the whole lot between A and B, giving A 50 acres more than B, and what quantity will each have?

PROBLEM V.

With the same data as the preceding, except that the dividing line is not to be parallel to the sides, but at right angles to the concession roads.

Let ACDB be the given lot; from C draw CE, at right angles to its front AB; CE is the perpendicular depth of the lot; find this, as in the preceding Problem, and determine AE, which is the cosine of the given angle of inclination, to which CE is the sine; find the area of the triangle AEC, which is equal to $\frac{AC}{2} \sin. \theta \cdot \cos. \theta$.



If this be greater than the required area, then the dividing line will fall within the triangle AEC as HK, and the triangle AHK will be similar to AEC; but similar triangles are as the squares of their similar sides, (see Theor. 19, p. 15,) and, therefore, making $Ak = x$, we have the following proportion to determine it, viz.:—

As the whole area AEC : the given area :: $AE^2 : x^2$

EXAMPLE 1.—The data of the preceding example being assumed, it is required to cut off six acres, by a line at right angles to the front of the lot.

We found in the last example $CE = 66\cdot16$

$$\log. \cos. CAE = 9\cdot08589$$

$$+\log. AC = 1\cdot82387$$

$$\log. 8\cdot12 = 0\cdot90976$$

$$\text{area of AEC} = \frac{66\cdot16 \times 8\cdot12}{2} = 268\cdot61 \text{ sq. chs.}$$

By proportion,—As $\log. 268\cdot61 \text{ chs.} = 2\cdot42911$

is to $\log. 60 \text{ chs.} = 1\cdot77815$

so is $2 (\log. 8\cdot12) = 1\cdot81912$

$\underline{3\cdot59727}$

$\underline{2\cdot42911}$

$2 | \underline{1\cdot16816}$

$\text{AK} = \log. 3\cdot83 \text{ chs. frontage} = 0\cdot58408$

EXAMPLE 2.—It is required, with the same data, to find the length of the side HK, and determine the actual area of AKH.

A. R. P.
Ans. HK = 31.28 chs.; and the area of AEC = 5 3 39

PROBLEM VI.

To lay out a given area in the form of a triangle, when the base, and the angle at the base, are given.

Let ABC = the given triangle, having the base AB, and the angle BAC, given; and let the area of the triangle = A; it is required to find, first, AC, and thence AD, and DC.

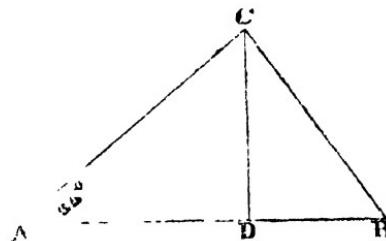
By the nature of triangles,

$$A = \frac{AB}{2} \cdot DC = \frac{AB}{2R} \cdot \sin. \angle A \cdot AC$$

$$\therefore \frac{AC}{2} = \frac{\text{Area} \times R}{AP \sin. \angle A}$$

$$\text{then } DC = AC \sin. \angle A$$

$$\text{and } AD = AC \cos. \angle A$$



EXAMPLE 1.—There are 46 acres of land to be laid out in the form of a triangle, whose base is 15 chains, and the angle 35 degrees; required the lengths of the other two sides.

Here AB = 15, and the angle CAD = 35°,

$$\text{now } \frac{AC}{2} = \frac{\text{Area}}{AB \sin. \angle A}$$

| | |
|-------------------------------|-----------|
| log. area (460 square chains) | = 2.66276 |
| + log. rad. | = 10. |
| <hr/> | |
| 12.66276 | |

$$\log. AB (15 \text{ chs.}) = 1.17609$$

$$+ \log. \sin. \angle A (35^\circ) = 9.75859$$

$$10.93468 = -10.93468$$

$$\frac{AC}{2} = \log. 53.47 = 1.72808$$

$$2$$

$$\text{chs. } 106.94 = AC$$

EXAMPLE 2.—Having the same data, to find the length of BC, in the previous example, and also DC; and thence to determine, whether, with these lengths, the triangle will contain the given area.

PROBLEM VII.

From a given triangle, to cut off any given area, by a line drawn from the vertex to the base.

Triangles, of equal altitudes, are proportional to their bases (see Theorem 17, page 15); therefore, making A, the given area of the triangle; a , the part to be cut off; and x , the required portion of the base, we have

$$x = \frac{a \cdot \text{base}}{A}$$

EXAMPLE 1.—There is a Gore of land between two townships, whose area is 425 acres, and base is 85 chains. It is required to cut off 400 acres by a line drawn from the vertex.

As 435 : 400 : 85 chains.

$$\begin{array}{r} 400 \\ 425)34000(80 \text{ chains} \\ \underline{34000} \\ \text{length of new base} = 80 \text{ chains} \end{array}$$

EXAMPLE 2.—From a Gore of land, having a base of 40 chs., containing 125 acres, to cut off 50, by a line from the vertex, required the base.

EXAMPLE 3.—From a triangle, with a base of 74·54 chs., containing 35 acres, to cut off 860 square yards, required the base.

I have annexed a few questions of a more practical nature, and worked them out fully, so that the reader may make himself acquainted with the means of ensuring that practical accuracy, which is indispensable for success, in a profession to which so much responsibility is attached, and where the consequences of inaccuracy are so serious and lasting as in the survey of a new country. In Canada, where I was engaged for some time, one third (I believe I am speaking within compass) of the common law cases were either disputed surveys, or originated in the bad feeling engendered from encroachments, or supposed encroachments upon each other's property.

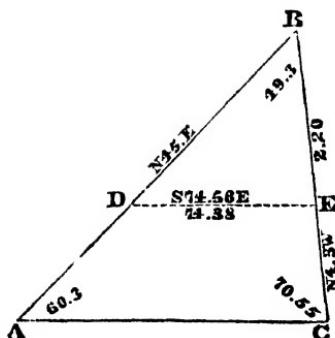
PRACTICAL EXAMPLES.

1st. There is a Gore of land between two townships, the boundary line of one being N. 45° E., and that of the other N. 4° 3' W.; the length of the first line, 240·00 chs., that of the second 220 chs.

It is required to cut off, at the vertex end, 300 acres, by a line parallel to the base of the Gore.

Because the one line bears N. 45° E., and the other N. 4° 3' W., the angle between them is 49° 3'.

$$\frac{240 + 220 + \sin 49^\circ 3'}{2 R} = \text{area.}$$



$$\begin{array}{rcl}
 \log. \frac{240}{2} = 120 \text{ chains} & = & 2.07918125 \\
 \log. 220 \text{ chains} & = & 2.34242268 \\
 \log. \sin. 49^\circ 3' & = & 9.87810900 \\
 \hline
 & & 14.29971293 \\
 - 10 & & \\
 \hline
 1994 \text{ acres} = 1994000 \text{ sq. chains} & = & 4.29971293
 \end{array}$$

Now, as similar figures are proportional to the squares of their homologous sides in the whole, the whole area is to the given area as the square of one of the given sides is to the square of that portion of the side, that is to be cut off: that is,

$$\begin{array}{l}
 \text{As } 19940 \text{ sq. ch.} : 3000 \text{ sq. ch.} :: \overline{240^2} : x^2 \\
 \text{when } x^2 = \text{length to be cut off.}
 \end{array}$$

$$\begin{array}{rcl}
 \text{As log. } 19940 & = & 4.29971293 \\
 \text{is to log. } 3000 & = & 3.47712130 \\
 \text{so is log. } 240^2 & = & 4.76042240 \\
 & = & 8.23754370 \\
 & = & 4.29971293 \\
 \text{to log. } 93 \text{ chs. } 09 \text{ lks.} & = & 2/3.93783077 \\
 & & 196891538
 \end{array}$$

The other side BE will by the same process be found to be 85.33 chs.

Having obtained the lengths of BD and BE, the two sides of the piece to be cut off, that will contain 300 acres, together with the included angle 49° 3', we can calculate the angles at the base by the second case of trigonometry.

These angles will be found to be as follows, viz.:—70° 55' = greater angle, and 60° 3' = the less.

To prove the calculation.

$$\text{Because the area} = \frac{\overline{DB \cdot BE \cdot \sin. 49^\circ 3'}}{2 R}$$

$$300 \text{ area} = \frac{85.33 \times 93.09 \times \sin. 49^\circ 3'}{2 R}$$

$$\begin{array}{rcl}
 \text{log. } 42^{\circ} 66 \text{ chs.} & = & 1.630029 \\
 + \text{ log. } 93^{\circ} 09 \text{ chs.} & = & 1.968915 \\
 + \text{ log. sin. } 49^{\circ} 3' & = & \underline{9.878109} \\
 & & 13.477053 \\
 & & 10 \\
 3000 \text{ square chains} & = & \underline{3.477053}
 \end{array}$$

Having verified the correctness of the calculated sides, we proceed to obtain the length and bearing of the base.

$$\begin{array}{rcl}
 \text{As the sin. } 60^{\circ} 03' & = & 9.9377492 \\
 \text{is to } 85^{\circ} 83 \text{ chs.} & = & 1.9311260 \\
 \text{so is sin. } 49^{\circ} 03' & = & \underline{9.8781090} \\
 & & 11.8092350 \\
 & & 9.9377492 \\
 \text{DE} = 74.38 \text{ ch.} & = & \underline{1.8714858}
 \end{array}$$

To obtain the bearing.

$$\begin{array}{l}
 \text{AB bears N. } 45^{\circ} 3' \\
 \text{add } 60^{\circ} 3'
 \end{array}$$

$105^{\circ} 6'$ is the angle made with the north, because it is greater than a right angle; subtract it from 180, and its supplement, $74^{\circ} 54'$, is the angle made with the south point of the needle, being, therefore, S. $74^{\circ} 54'$ E.

In order, therefore, to cut off 300 acres from the Gore ABC, measure from B to A, 93.08 chs., and at that point run a line, bearing S. $74^{\circ} 54'$ E, which will be parallel to the base of the Gore, and should be 74.38 chs. in length.

The same data being given, it is required to cut off 300 acres towards the township, whose boundary line bears N. 45° E, by a line drawn from the vertex to the base.

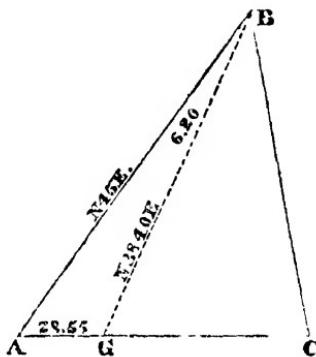
Using whatever calculations may have been made above, that may be useful to our purpose, first ascertain the base of the Gore.

$$\begin{array}{rcl}
 \text{As sin. } 60^{\circ} 3' & = & 9.9377492 \\
 \text{is to } 220 \text{ chs.} & = & 2.3424227 \\
 \text{so is sin. } 49^{\circ} 3' & = & \underline{9.8781090} \\
 & & 12.2205317 \\
 & & 9.9377492 \\
 \text{to } 191.17 \text{ chs.} & = & \text{AC length of base of Gore} = 2.2827825
 \end{array}$$

Now triangles of equal altitudes are to each other as their bases; therefore, the whole Gore is to the piece to be cut off, as the whole base is to the base of the piece cut off.

$$\begin{array}{rcl}
 \text{As } 19940 \text{ sq. chs.} & = & 4.27991293 \\
 \text{is to } 3000 \text{ sq. chs.} & = & 3.47712130 \\
 \text{so is } 191.77 \text{ chs.} & = & 2.28272250 \\
 & & \hline
 & & 5.75990380 \\
 & & 4.29971297 \\
 \text{to AG} = 28.85 \text{ ch.} & = & 1.46019087
 \end{array}$$

Next find the angles of the triangle ABG, by the rule for the second case of trigonometry, so as to determine the length and bearing of BG.



These angles will be found as follows, viz.:— $113^{\circ} 36'$ = larger angle AGB, and $6^{\circ} 20'$ = the smaller angle ABG.

$$\begin{array}{rcl}
 \text{BA is S. } 45^{\circ} \text{ W.} & & 45^{\circ} \quad 0' \\
 \text{subtract} & & 6^{\circ} \quad 20' \\
 & & \hline
 & & 38^{\circ} \quad 40'
 \end{array}$$

the bearing, therefore, is S. $38^{\circ} 40'$ W.

To find the length of the line BG.

$$\begin{array}{rcl}
 \text{As sine } 6^{\circ} 20' & = & 7.04262490 \\
 \text{is to } 2885 \text{ chs.} & = & 1.46019087 \\
 \text{so is sin. } 66^{\circ} 3' & = & 9.93774920 \\
 & & \hline
 & & 11.39794007 \\
 & & 9.04262490 \\
 \text{to } 226.65 \text{ chs.} = \text{BG} & = & 2.35531517
 \end{array}$$

To lay off the line BG, cutting off 300 acres on the side AB, proceed to the vertex B, and run BG south $38^{\circ} 40'$ west, till you come to the base, and measure the distance GA, which should be 28.85 chs.; if the actual distance be found to be the same within a few links, the work is finished, but if not (*let it be 29.85 chs. instead*)—

that is, one chain too much to the east), it becomes necessary to correct the bearing of the line by the compass, and run it over again.

To find the angle of correction.

As 226·65 chs.

is to sin. 90° or radius

so is 1·00 chs.

to the sin. opposite ∠

As 226·65 : 57° 3' :: 1·00 : x when x = length of the arc in degrees, and 57° 3' is the angle, whose length of arc equals radius.*

$$\begin{array}{r}
 57\cdot3 \\
 1\cdot00 \\
 \hline
 57\cdot3 \\
 \hline
 60 \\
 \hline
 226)343\cdot80(15'
 \end{array}
 \quad \begin{array}{l}
 \text{present bearing S. } 38^\circ 40' \text{ W.} \\
 \text{add } \underline{15'} \\
 \text{corrected bearing S. } 38^\circ 55' \text{ W.}
 \end{array}$$

PROBLEM VIII.

The bearings of three sides of a triangle being given, to cut off a given area by a line parallel to the base.

RULE 1st.—These three lines form a triangle, whose angles are all known, make one side = unity, and calculate the proportionate lengths of the other sides, making this side = x , and multiplying by the other two sides; all the sides are known in terms of x ; find the area, which will be equal to the given area; the solution of the equation gives the answer: or,

RULE 2nd.—Say, As the rectangle of the sines of the angles at the base : the rectangle of radius into the sine of the opposite angle :: twice the area : square of the side required. The other sides may be found in the same way.

EXAMPLE 1.—Let one of the boundary lines of a Gore, between two townships, bear N. 38° W., and the other N. 16° 30' E.; it is required to cut off 325 acres from the Gore end, by a line, bearing N. 88° 15' E.

$$\bullet 2 \pi \text{ rad.} = \text{circumference} = 360 \text{ degrees.}$$

$$\pi \text{ rad.} = 180 \text{ deg.} : \text{rad.} = \frac{180^\circ}{\pi} = 57^\circ 3'.$$

RULE 1st.—First, find the interior angles, and let $AB = 1$, or unity.

To find BC or AC

$$\begin{array}{lcl} \text{As sine } 53^\circ 45' & = & 9.90657 \\ \text{is to } 1^\circ & = & 0.00000 \\ \text{so is sine } 71^\circ 45' & = & 9.97759 \\ \text{to } 1.178 \text{ chs. BC} & = & 0.07102 \end{array}$$

$$\begin{array}{lcl} \text{As sine } 53^\circ 45' & = & 9.90657 \\ \text{is to } 1^\circ & = & 0.00000 \\ \text{so is sine } 54^\circ 30' & = & 9.91069 \\ \text{to } AC = 1.01 & = & 0.00412 \end{array}$$

$$\begin{array}{l} AB = 1.000 \\ AC = 1.010 \\ BC = 1.178 \end{array}$$

Let fall a perpendicular BD.

$$\text{Area} = \frac{AC \cdot AB \cdot \sin. \angle CAB}{2}$$

$$\begin{array}{lcl} \log. AB, \text{ or } 1.000 & = & 0.00000 \\ \log. AC, \text{ or } 1.010 & = & 0.00412 \\ \sin. \angle CAB & = & 9.97757 \\ -\log. rad. & = & 10.00000 \\ 2 | \underline{.9588} & = & 1.98171 \\ .4794 \text{ square chains.} & & \end{array}$$

Because similar figures are as the squares of their homologous sides.

$$\begin{array}{lcl} \text{As } .4794 \text{ sq. chs.} & = & 1.68070 \\ \text{is to } 3250 \text{ sq. chs.} & = & 3.51188 \\ \text{so is } 1^2 & = & 0.00000 \\ \text{to } x^2 & = & 2 | \underline{3.83118} \\ 82.34 = AB & = & 1.91559 \end{array}$$

which is the measure of the side taken as the unit side of the three.

Rule 2nd.—

| | | | | |
|--------------------------------------|------------|-------------|----------|----------|
| As sine A. sine B | { | 9.97759 | = | 19.88828 |
| | | 9.91069 | | <hr/> |
| is to rad. sine C. | { | 9.90657 | = | 19.90657 |
| | | 10.00000 | | <hr/> |
| so is twice the area (6500 sq. chs.) | = | 3.81291 | | |
| | | | 23.71948 | |
| | | | 19.88828 | |
| | to x^2 = | 2 3.83120 | | |
| 82.34 = AB. | = x = | | | 1.91560 |

the same as before.

To find AC and BC , the other two sides (by the first rule); multiply each of the sides of the unit triangle by this common unit of measurement, or marking $A =$ the actual area, and a , the unit area, and x , as before, the length of any other side, in the unit triangle a , which was taken; then we have, by similar triangles,

$$a : A :: x^i : y^i.$$

i. e. $\frac{Ax^*}{a} = y^*$ where y is the actual length of the same side.

$$x \sqrt{\frac{A}{a}} = y \text{ or } \log x + (\log \sqrt{\frac{A}{a}} = 1.91560) = \log y.$$

where $\sqrt{\frac{A}{a}}$ or 1.91560 is the actual length of the unit side (AE).

Let x ($= 1.010$) then $\log. x = 0.00412$

$$\log, \sqrt{\frac{A}{a}} = 1.91559$$

$$AC = 83.12 \text{ chs.} \quad \equiv 1.91971$$

Again,

$$\text{Let } x = 1.178 \quad \equiv 0.07102$$

$$BC = 96 \text{ chs. } 96 \text{ lks.} \quad \equiv \underline{\underline{1.98661}}$$

AB = 82.34 chs.

BC = 96·96 chs.

AC = 83·12 chs.

To find the side AC (by the second rule).

$$\text{As the } \sin. \angle A \sin. \angle C. \frac{9.97759}{9.90657} \} = 19.88419$$

$$\text{is to radius } \sin. \angle B. \frac{9.910692}{10.000000} \} = 19.91069$$

$$\begin{array}{rcl} \text{so is twice the area (6500)} & & \underline{3\cdot81291} \\ & & 28\cdot72360 \\ & & 19\cdot88416 \\ & \text{to } x^2 = \frac{2|3\cdot83994}{2} \\ \text{AC} = 83\cdot12 \text{ chs.} & & = x = 1\cdot91972 \end{array}$$

To find the side BC.

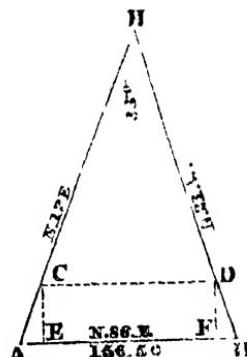
$$\begin{array}{rcl} \text{As sin. B sin. C} & & \frac{9\cdot91069}{9\cdot90657} \} = 19\cdot81724 \\ \text{is to rad. sin. A} & & \frac{9\cdot97759}{10\cdot00000} \} = 19\cdot97759 \\ \text{so is twice the area (6500) sq. chs.} & & = \frac{3\cdot81291}{23\cdot79050} \\ & & 19\cdot81724 \\ & \text{to } x^2 = \frac{2|3\cdot97324}{2} \\ \text{BC} = 96 \text{ chs. 96 lks.} & & = x = 1\cdot98662 \end{array}$$

the same as before.

EXAMPLE 2.—Given the bearing of a Gore of land, north 12 deg. east, and north 21 deg. west, and the base 156 chains, 50 links, bearing north 86 deg. east.

It is required to cut off 100 acres from the frustum end of the Gore.

$$\begin{array}{rcl} \text{As sin. } 33^\circ & = 9\cdot73611 \\ \text{is to } 156\cdot50 \text{ chs.} & = 2\cdot19451 \\ \text{so is sin. } 73^\circ & = \frac{9\cdot98060}{12\cdot17511} \\ & 9\cdot73611 \\ \text{to AE} & = 2\cdot43900 \end{array}$$



$$\begin{array}{l} \text{perpendicular of whole triangle} = AE \cdot \sin. A, \\ = 9\cdot98284 \\ = 2\cdot43900 \\ \log. 264\cdot20 = 2\cdot42184 \end{array}$$

$$\begin{array}{rcl}
 \log. 264\cdot20 & = & 2\cdot42184 \\
 + \log. 156\cdot50 & = & 2\cdot19451 \\
 \hline
 2|41330 & = & 4\cdot61635
 \end{array}$$

Area of triangle = 20665 square chains.

Subtract 1000 sq. chs., the area of the frustum to be cut off.
there results 19665 sq. chs. the portion to be left at the vertex end.

$$\begin{array}{rcl}
 \text{As } 20665 \text{ square chains} & = & 4\cdot31523 \\
 \text{is to } 19665 \text{ square chains} & = & 4\cdot29368 \\
 \text{so is } 274\cdot80 & = & \begin{array}{l} 4\cdot87800 \\ \hline 9\cdot17168 \\ 4\cdot31523 \end{array} \\
 \hline
 274\cdot80 & = & 2\overline{4\cdot85645} \\
 278\cdot05 \text{ chains} & = & \underline{= 2\cdot42823} \\
 \hline
 6\cdot75 \text{ chains} & = & AC
 \end{array}$$

$$\begin{array}{rcl}
 \text{As sin. } 74^\circ & = & BD, \sin. 73^\circ \\
 BD & = & AC, \sin. 74 \\
 \hline
 & & \sin. 73^\circ \\
 \log. 6\cdot75 & = & 0\cdot82930 \\
 \log. \sin. 74^\circ & = & \begin{array}{l} 9\cdot98284 \\ \hline 10\cdot81214 \end{array} \\
 -\log. \sin. 73 & = & -9\cdot98060 \\
 BD, 6\cdot785 \text{ chs.} & = & \begin{array}{l} 0\cdot83154 \\ \hline \end{array}
 \end{array}$$

To prove this work,

$$\begin{array}{rcl}
 \cos. 74^\circ. AC & = & AE \\
 \cos. 73^\circ. BD & = & BF \\
 \log. AC, 6\cdot75 \text{ chs.} & = & 0\cdot82930 \quad \text{and } EC = \sin. 74^\circ. AC \\
 \log. \cos. 74 & = & 9\cdot44034 \\
 \log. AE, 1\cdot8605 & = & 0\cdot26964 \\
 \log. BD, 6\cdot78 \text{ chs.} & = & 0\cdot83154 \\
 \log. \cos. 73^\circ & = & \begin{array}{l} 9\cdot46594 \\ \hline 0\cdot29748 \end{array}
 \end{array}$$

$$\begin{array}{rcl}
 1\cdot8605 & & 156\cdot50 \\
 1\cdot9837 & & 1\cdot92 \\
 \hline
 2|8\cdot8442 & & \log. 154\cdot58 = 2\cdot18915 \\
 & & \log. 6\cdot48 = 0\cdot81214 \\
 1\cdot9221 & & \\
 & & 100 1 8 = \text{sq. chs. } 1003 = 3\cdot00129 \\
 & & R. P. \\
 & & \text{or } 1 8 \text{ of excess in } 100 \text{ acres.}
 \end{array}$$

sufficiently near for practical purposes, as if the perpendicular EC had been 6 chains 47, instead of 6 chains 48, the result would have been exactly 100 acres.

CHAP. VII.

POLE STAR.

THE α of Ursa Minor, or as it is more commonly termed Polaris, is about $1\frac{1}{2}^{\circ}$ from the true pole, and revolves round it in 23 hours 56 minutes. When it is at its greatest distance, east or west, it is said to be at its eastern, or western elongation.

The true bearing of the pole star, that is, the angle made at the centre of the earth, between the true pole and the pole star, is called the polar azimuth; this, which should be taken at the time of its greatest elongation, depends upon the latitude of the place, and the distance of the star from the pole. This distance is called the polar distance, it is subject to a small annual diminution, called precession, which is 19·3 seconds annually. In the year 1830 this distance was $1^{\circ} 35' 50''$; by multiplying the number of years since by 19·3 seconds, and deducting the product, the actual polar distance can be obtained. Now the azimuth, or angle of variation of the pole star, can be always determined by the following proportion:—

As radius : sin. lat. :: sin. polar dist. : azimuth.

Having obtained the polar distance to any day and place, in order to ascertain the angle of variation, or polar azimuth, find the time in the Nautical Ephemeris, when the star is about its greatest eastern or western elongation, and, with a theodolite or circumferentor which is furnished with a telescope, observe, carefully,

when the star, having ceased to move in its first direction, begins to retrograde ; fix the telescope carefully in that place, and direct an assistant to take a stake, with a lighted candle upon it, and put it down in the same line, at some 8 or 10 chains distance. This is the line of elongation ; then say,—as radius is to the whole distance, which must be carefully measured, so is the tangent of the angle of variation, to the actual distance in feet, measured at right angles to the former ; the line connecting this new point, and the place where the instrument was, is the meridian line required. Take the bearing of this line, and the angle, found between this and the magnetic north, becomes the angle of variation of the compass ; should the needle point to the east of this meridian line, the variation is easterly ; should it point to the west, it is westerly.

CHAP. VIII.

ON LOCAL ATTRACTION.

THE needle being often, from various causes, diverted from its polarity, it becomes requisite, in running a line, to try the *backward* bearing at every station, and to see if it corresponds with the forward : if it does not, try the last forward station again, to see if any error may have been committed in taking it, should there be none, as the previous backward bearing, by assumption, must correspond with the preceding forward station, there can be no attraction there, and the attraction must be at the one where the backward bearing differed. Allow for the error or angle of attraction at the next station,

and proceed until this error be compensated. During the continuance of this angle of error, there is local attraction in the neighbourhood.

As at the first station, in starting, there is no backward bearing to prove the non-existence of attraction, it is impossible to say whether the error be at the first or second station, should the backward bearing of the second station not correspond to the forward bearing of the first; by taking, however, a third station, and taking therefrom and thereto, the bearings of both the first and second stations, the error will be discoverable immediately, as the backward and forward bearings of (from 2 to 3 and from 1 to 3) cannot both agree. Where the disagreement is, there is the local attraction.

Attraction may, however, commence with the first station, and not be discovered until some stations afterwards; should this be suspected, it becomes necessary to test the first bearing relatively, by a line making any angle with it, (*which angle has been measured by the chain,*) to such a distance, as may be considered beyond the sphere of the supposed attraction. There are many other *local* checks beside this; lines are seldom run in space unconnectedly in the woods, and, by measuring the distance from the next base line, its correctness may soon be determined.

CHAP. IX.

THIS page contains a small collection of those conventional signs that are most in use; the method of representation is such as is generally adopted.

CONVENTIONAL SIGNS.

| | | | |
|-----------|--------------------|--|--------------|
| | River | | Coal |
| | Canal | | Limekiln |
| | Bridges | | Stone Quarry |
| | Drawbridge | | Town |
| | Ford | | Village |
| | Horse Ferry | | with Church |
| | Rope Ferry | | Post House |
| ROADS. | | | Turnpike |
| | Turnpike Road | | Smithy |
| | Highway | | Telegraph |
| | Occ. Road | | |
| | Bridle Road | | |
| | Rail Road | | |
| | Cutting | | |
| | Embankment | | |
| MILLS. | | | |
| | Windmill | | |
| | Sawmill | | |
| | Watermill | | |
| MILITARY. | | | |
| | Redoubt | | |
| | Fort | | |
| | Artillery Position | | |
| | Battery | | |
| | Mortar Battery | | |

| MINES. | | BOUNDARIES. |
|--------|---------------------|-------------------------|
| ◎ | Gold | — — — — of a County |
| ▷ | Silver | — · · · — of a Parish |
| ↳ | Tin | — · · · — of a Township |
| ♀ | | |
| ‡ | Copper | |
| § | : : . . Lead | △ Station Point |
| | : : . . Quicksilver | |

CHAP. X.

ON PLOTTING, SCALING, &c.

BEFORE commencing to plot, it is always requisite to consider carefully the shape of the plan to be plotted, its size and character, and the most desirable position to place it upon the paper, so as to admit of the best vacant space for the insertion of the heading or title, with the usual specification, that should accompany it. It has generally been considered indispensable to place the plan, so as to have the north side of the plan on the top of the paper fronting you; but, I would recommend, that the position of the meridian line should be but a secondary consideration, and should, in every case, depend upon the size and shape of the plan; where these two desiderata can be combined, it is better, of course, to do so; though I certainly cannot deem it a matter of so much moment as it is often made.

Before commencing your plan, take care also to have the paper properly stretched upon a drawing board, if the size of the plan will admit of it, and finish the whole plan before taking the paper off the board. At the bottom of the paper, make a scale of the required proportion, carefully dividing it into tenths and hundredths as the case may be, and let all *long* lines upon the plan be measured off this scale. The *short* ones, that is, lines of offsets; lines of distances, less than tenths, may be taken off the ivory scale, from which the scale upon the paper was first obtained.

Paper contracts very much, so much so, that there is frequently a difference in the length of the same line between one year and another of two hundredths, when the line is 10 or 11 tenths long; a line one year may measure 20 or 30 chs., and the next be only 19 chs. 80 lks., or 29 chs. 75 lks., being a loss of a link or two in each chain. In distances, therefore, of less than a chain's length, there can be no perceptible error. These distances might always be taken by the common ivory scale of equal parts, and, in fact, should be, as these smaller subdivisions could never be so well divided upon a paper scale; and the divisions upon paper would, from constant using in the making of the plan, soon wear through one into the other. These remarks may appear superfluous to a novice, but experience will soon show him the value of them. The neglect of these considerations has been the source of many a day's loss of work to a beginner.

Having made the scale, lay down your base line very accurately, and draw it carefully in with lake, marking the various stations upon it and its total length. Then take, with the compasses, the various lengths of the

sides of the ~~several~~ triangles, of which the survey is composed, and lay off the different points of intersection, testing rigorously, as you proceed, the constructed, with the measured lengths of the respective *check-lines*.

Do all this *before* an offset is put in, unless the offset be afterwards used as the point of a more convenient base line for another triangle.

When this part of the plotting is found correct, draw the lines in very plainly with lake.

In marking off the several distances on the base line use one of the long scales, and, placing it close against the given line, prick off, with a fine needle, the proper distances, and round the points, as centres, draw a small circle, but on no account use any black lead pencil, however finely sharpened, in stations of this kind. Do this also, in determining the point of inclination of the sides of the triangle. And in cases where great accuracy is required, I should prefer striking the radii, not with a bow pen, but with the finely pointed compasses themselves.

This can be done so slightly, as accurately to show the intersecting arcs, without at all injuring the paper. The mere injury of the paper, however, should never be a subject of any consideration, in comparison with the accuracy of the drawing, as, in all cases, a correct and accurate office drawing must be made, as a plan of reference, in which accuracy is the only desideratum,—accuracy in the outline,—accuracy in the determining of the areas. In a finished plan it is totally different; a finished plan is seldom made, in fact, it ought never to be made, an authority for the extension or compilation of surveys, as accuracy of detail, and beauty of colouring, are seldom, and not easily, combined.

Having finished these subsidiary lines, as they may

with propriety be termed, proceed to the laying off the offset points.

The best method of doing this, is to place the long scale, above referred to, close against the given line, having the zero points of each coinciding, and get an assistant to read off the several distances thereon, whence offsets are taken, first going through the right offsets, then the left. An offset scale is now necessary, which is a small scale of about 3 inches, divided in the same way as the long one, but the zero points being either edge of the scale. This is placed against the long scale, and the lengths on the measured line are determined by the long line, while the distance of any point, or offset therefrom, is determined by the offset scale; this latter point is alone marked. Practice and care will ensure considerable rapidity, as well as accuracy, in this plan.

When the scale is 2 or 4 chains to the inch, any offset, less than 10 links, must be done by the hand.

With reference to the division of the scales—the scales used for horizontal delineations are generally 2 or 4 chains to the inch, or 20 chains, or 40 chains. The usual scale used in the plotting of the tithe commutation surveys, is that of 3 chains to the inch.

The division of the common and vertical sections, is that of 5 or 50 feet, 10 or 100 feet to the inch.

I have one caution to give beginners, in the purchasing of scales, never to purchase a scale having different divisions upon it, as it is productive of considerable and serious error.

The additional expense when you are purchasing—of purchasing one of a set—is soon made up by the saving of the frequent losses of time, that a difference of divisions on the same scale must inevitably occasion.



CHAP. XI.

TOPOGRAPHICAL SURVEYING.

In surveying a tract of country, it is frequently not only necessary that a correct delineation should be obtained of the various outline of hill and dale, of river and forest, but that some method should be adopted of conveying upon the same plan a pretty fairly correct representation of the relative heights of its different parts. The positions of the various boundaries in the plan are, or always should be, the horizontal spaces they would occupy on the earth's surface, totally independent of their height; this height must, therefore, be obtained in some other way; straight lines drawn from the summit, diverging to the bottom, more or less close, as the hill is steep or otherwise, are used in the one case: this is called the vertical method; in the other, the irregularities of the earth are represented by waving horizontal lines, approaching or receding in proportion to the steepness of the ground, or its graduated ascent. The two plates, Nos. 6 and 7, are examples of the first method. There is a boldness of style, and a faithfulness of representation, which I am disposed to consider must give it the preference. This style of drawing is called military drawing, probably from the special demand for these

topographical features of country for military purposes. By this, an officer is enabled, at a glance, to ascertain whether a commanding point, he is desirous of occupying, is accessible to cavalry, or unapproachable by infantry; and he is thereby enabled to decide, whether he can venture to attempt the dislodgement of an enemy from one hill, or can permanently occupy another, in spite of odds that may be sent against him.

This kind of drawing has its advantages even to the civil Surveyor; if well done, the introduction of the features of the country is a great improvement to the plan, abstractedly; and, to the proprietor of an estate, the topographical details are often useful in assisting him in the laying of it out.

I need scarcely refer to the great assistance, that the engineers of the present day have derived from the faithfulness of the topographical delineations of this country in the ORDNANCE MAPS, in the selection of the best lines of routes for railways; every professional man must have experienced this himself.

Plate 8



7 Mar. 17 '00

L E V E L L I N G,

ITS NATURE AND OBJECTS.

CHAP. I.

LEVELLING is the art of representing the inequalities of the earth's surface, and of determining the relative heights of any number of points above or below a line, equidistant, at every point, from the centre of the earth. This line is what is understood by the term—a level line ; it is that line which is assumed by water when at rest.

The instrument, used for the purpose of levelling, is called a spirit level.

T H E Y S P I R I T L E V E L.

Its Description.

This instrument is merely one portion of almost every other instrument, carried out to its greatest practical perfection. The bubble, which in most instruments forms only a subordinate part in the construction, is in this the chief, the only object of the instrument being to obtain a practical tangent to the earth's surface, or to place

the line of collimation of the telescope in a truly horizontal line. Hence it is termed THE SPIRIT LEVEL—*par excellence*.

This instrument consists, like the others, of its parallel plates, with their two pairs of conjugate screws, of its telescopes, and its spirit level beneath. The telescope stands upon Y's, (I have selected the Y level in preference to the rest for description,) as in the case of the theodolite; and has, also, like that instrument, its milled-head adjusting-screw for the object-glass; and the moveable eye piece for neutralizing the parallax. The cross wires, however, are not always arranged the same way as in the theodolite; sometimes one horizontal and two perpendicular wires are used instead. By the latter contrivance you are enabled to secure the perpendicularity of the staff, in one direction, which you have no means of doing by the former.

The spirit level here, also, is furnished with its capstan-headed screws, for making it parallel to the axis of the telescope, vertically and laterally.

But in this instrument there is one contrivance which the other instruments do not possess—of raising or depressing one of the Y's, or supports, on which the telescope rests, so as to have the axis of the telescope always at right angles to the axis of the instrument.

ADJUSTMENTS.

I need only repeat, that, for the line of collimation,* the telescope must be turned round on its axis, that the intersection of its wires may always intersect one point, this adjustment will be found fully explained in

* The two first adjustments are similar to those of the theodolite.

the description of the theodolite. The adjustment of the second liability to error, in the non-parallelism of the level and the telescope, is obtained as before, by reversing the telescope in its Y's. These two adjustments must be completed first. Afterwards,

To make the axis of the telescope always at right angles to the axis of the instrument; in other words, to secure the line of collimation being perfectly level in any portion of a complete revolution of the instrument.

Set the telescope over any pair of conjugate screws, and make the bubble level; turn the instrument, till the telescope be over the conjugate pair: level it in this position; then turn it back to the first pair, and correct for any error that may have arisen from the last levelling, and continue till the bubble be central over the two pairs of conjugate screws; then turn the instrument one half revolution round, and, if the bubble still remain in the centre, the instrument is in adjustment; if not, the error can only be occasioned by the axis of the bubble, or, which is the same thing, the axis of the telescope not being truly perpendicular to the centering of the instrument.

To correct this error, raise or depress the moveable (Y) support by the milled-headed screw beneath, until the bubble be brought half-way to its proper position, and correct for the other half by the parallel screws. By repeating the correction two or three times, the greatest accuracy will be obtained.

It is necessary to examine the adjustment every morning before starting, and it should be seen to at every observation, though it will scarcely require re-adjusting the same day. I should observe, that there is, or ought to be, a cap over this adjusting (Y) screw which should be carefully kept on.

This level is a very delicate instrument, and soon liable to get out of adjustment without great care.

There are several other kinds of levels—Troughton's, Gravatt's, &c.: all good of their kind, and each, perhaps, more fitted than the rest for some peculiar kind of levelling.

In trial and check-levels, I would recommend Gravatt's or Troughton's, being calculated by their lightness and non-tendency of disarrangement, to get rapidly over the ground.

For the main sections, at every two chains, I should prefer the Y level; and for the putting down the rails, the formation of roads, and all work where accuracy, and not expedition, are required, I should decidedly give it the preference.

There is one fault I have found with most levels, that the tube of the telescope is not long enough to admit of reading off the staff within short distances, few reading within half a chain. Having had placed, for myself, in addition to the extending tube at the object end, another at the eye-glass to remedy this defect, I have been enabled to read within three yards; to this inner tube, of course, was attached the diaphragm of the cross wires and the lengthening eye-piece.

LEVELLING STAVES.

These are generally made twelve feet high, divided into feet, and again into tenths of a foot, and subdivided for facility of computation, into hundredths. The method of arranging this subdivision, constitutes the difference between the several staves in use. Some persons prefer one, some another, and I must leave the reader to select for himself. There are

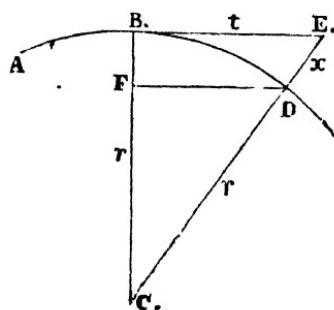
Sopswith's, Gravatt's, Cowper's, Stephenson's, &c. The one I myself prefer is, what I believe goes by the name of Stephenson's : and was first used on the London and Birmingham ; the hundredths are obtained in the same way as in the common ivory protractor ; the tenths of a foot through the whole length of the staff are bisected, making the two divisions "twentieths ;" and these division lines extend the whole breadth across the staff ; the opposite ends of these lines are connected by diagonal lines, each one with its preceding, viz.,—the left of No. 1 with the right of No. 2, the right of No. 2 with the left of No. 3, and so on. And five vertical lines are drawn along the whole of the staff, which thus divides each of these diagonal lines into five equal parts, each being the fifth part of the twentieth, or the hundredth of a foot.

The feet are distinguished by large red figures ; the tenths in black, with a large full point at every .5.

The lines obtained by this means are only level lines for very short distances, being *tangents* to the earth at the several points of observation, and not their corresponding *arcs*, which are the true levels.

The value of this error and the practical methods adopted had better be at once explained.

Let ABD be a portion of the earth's circumference, whose centre is C. Let BE be any level distance of the instrument (t), the true level line will be BD, and the error between the apparent and true level will be the versed sine BF. When the distance BE is great, this versed sine must be calculated by trigonometry.



nometry. But, for the usual distances of observation by the spirit level, ED, or sec.—rad., can be safely taken instead.

Now, CBC is a right-angled triangle, and therefore

$$\begin{aligned}\overline{r+x}^2 &= r^2 + t^2 \\ r^2 + 2rx + x^2 &= r^2 + t^2 \\ 2rx + x^2 &= t^2\end{aligned}$$

throwing x^2 away, as indefinitely small, in relation to $2rx$, we have

$$\begin{aligned}2rx &= t^2 \\ \text{and } x &= \frac{t^2}{2r}, \text{ or}\end{aligned}$$

the error x = the square of the tangent, divided by twice the radius; and, as this divisor is a constant quantity, this error is proportioned to the squares of the distances.

The mean diameter of the earth is 7,916 miles; for one mile distance, therefore, we shall have $x = \frac{1}{7916}$ miles, or 8,004 inches; for two miles distance, four times that quantity; for three miles, nine times; throwing away the root of an inch as immaterial, the error of one mile's distance is 8 inches or $\frac{1}{3}$ of a foot; for two miles, $\frac{4}{3}$ feet; for three miles, $\frac{9}{3}$ feet, &c.; or x in feet $= \frac{1}{3}$ (distance in miles) 2 ; which formula may be easily remembered. This value of x would not be sufficiently correct for many miles. Referring to the equation $\overline{r+x}^2 = r^2 + t^2$,

the value would then be $x = \sqrt{r^2 + t^2} - r$.

Example of applying this correction.

Placed a spirit level at any point B, (see fig. page 255,) on the earth's surface, and found the point E, at 3 miles off, to be on an apparent level with the point B. What is the comparative height of the object E?

Now, BE is the apparent level, and BD the true level, B and D being points equidistant from the earth's centre. DE (x) is the height of E above B, which, in feet, equals the distance (DE, in miles) $3^2 \times \frac{1}{3} = 9 \times \frac{1}{3} = 6$ feet, the height of the object E above B.

The apparent height, therefore, of every distant object, as observed by a spirit level from any point, is always *less* than the true height, by this value of x , for the curvature of the earth.

CHAP. II.

REFRACTION.

THERE is also, in long observations, another correction necessary, arising from the effects of the density of the atmosphere, in refracting the rays from the object, which makes the apparent greater than the true height. The correction, therefore, must be subtracted from its apparent height.

Refraction increases the distance at which objects can be seen (*cæteris paribus*), in a proportion of 14 to 13, and raises the apparent height one-seventh of its correction for curvature.

Thus, in the last example, the object observed at a distance of three miles was apparently level with the instrument, but the correction for curvature being six feet, and the height (independently of the subsequent correction for refraction) was 6 feet, $\frac{1}{7}$ of $\frac{6}{7}$ = $\frac{6}{49}$ feet = $1\frac{1}{7}$ inches, and therefore 5 feet $1\frac{1}{7}$ inches = the true height of the object, allowing for both corrections.

EXAMPLES.

1st.—The observed heights of three objects, at a distance of 4, 6, and 8 miles, (calculated from observations taken by the theodolite,) were found to be respectively, 24, 25, and 28 feet. What are their true heights?

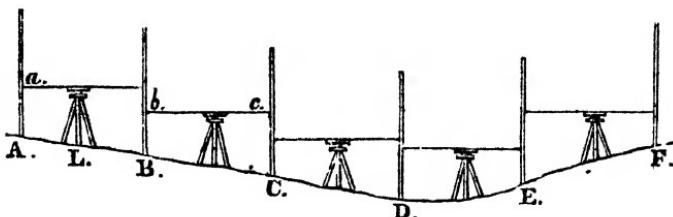
2nd.—Found the angle of elevation of the spire of a church, which was 420 chains 75 links off, to be $1^{\circ} 10'$. What is its real height above the point of observation?

CHAP. III.

To find the difference of levels between several points, or to trace a sectional line of the inequalities of the earth's surface.

Let ABCDE be the line to be traced. Set the level (L) between the object, and read off the height Aa and that of Bb, the difference between Aa and Bb will be the number of feet that B is higher or lower than Aa; if Bb be greater than Aa, the point B will be

lower (by this difference) than Aa; for the height, read off by the level staff, is the number of feet that each point observed is *beneath* the level of the line of collimation of the telescope—hence, where there is a number of points beneath the same level line, the greater the reading of the staff, the lower this point must be.



Then, because, in the first observation, the height at B (read by the level staff) is greater than that at A, the point B is lower than the point A. Again, in the second example, where, it must be observed, that another line of collimation is taken, because the height by the staff at C is greater than at B, the point C is lower than B. In the third observation, also, D is lower than C, and C being lower than B, and B than A, the ground falls thus far. At the fourth observation, however, because the height at D is greater than that at E, the point D is lower than E, and, therefore, E being higher than D, the ground rises to E, and as the reading at E is greater than at F, it goes on rising to F. The relative heights of the two ends of the line, at A and F, depend upon whether the ground falls, more or less, from A to D, than it rises from D to F.

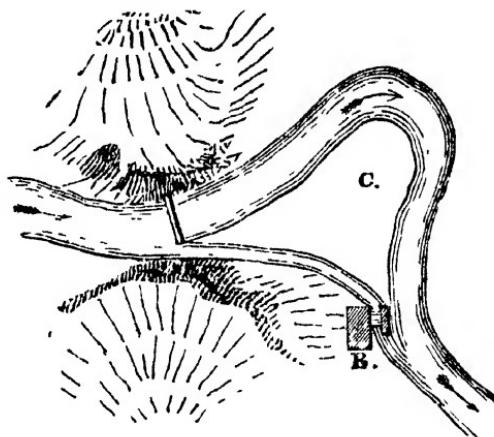
Now, the difference between the reading at B and A, in the first observation, added to the difference of readings at C and B, in the second observation, plus this difference between D and C, in the third, as there is a continued descent to the point D, will give the actual fall from A to D, or the number of feet, that the point D is lower than A. In the same way, the sum of the difference of readings of D and E, and of E and F, in their respective observations, will be the number of feet F is higher than D; if, therefore, the fall from A to D, be greater than the ascent from D to F, the difference will be the actual fall from A to F, or the number of feet that the point A is higher than the point F.

This is the principal object of levelling. It is very simple in theory, but in the carrying out of the practical operations, great care is

necessary. In this, as in most things which are of a simple character and which do not admit of checks in the course of the work, errors are very like to creep in imperceptibly. There are certain mechanical contrivances to guard against them, which will be subsequently explained.

This taking of several stations is called compound levelling. Simple levelling is merely the determining, by one observation, the relative heights of two given points.

Thus, supposing that it is required to ascertain, in the accompanying diagram, whether there was sufficient fall of water between that part of the river at A, and that, at B, to turn a grist and saw-mill at B. Placed the level at C, between the two stations, and found the first or back reading 0·45 feet, and the forward 11·52 feet; the back distance was 2 chains, and the forward distance 20 chains; required the fall.



The back reading is 0·45 feet, at the short distance of 2 chains, which will require no correction for curvation or refraction.

The forward reading, being at a distance of 18·50 chains, must be corrected.

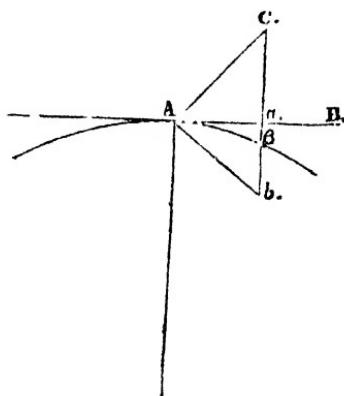
Now, x (for curvature) = $\frac{20^2}{7916} = 5$ inches = .416 feet. This must be subtracted from the observed reading, and you obtain the true reading (independent of the refraction),

$$\begin{array}{r}
 11\cdot52 \text{ feet, observed reading} \\
 - 0\cdot416 \\
 \hline
 11\cdot104 \text{ true distance beneath the first point.}
 \end{array}$$

Add, for refraction, $\frac{1}{4}$ of .416, or .059, and the true height of the water at A, above that at B, is 11.163 feet.

As in this case, to obtain the true reading, the correction for the curvation has been subtracted, and that for refraction added, which is the reverse of the preceding rule at page 257, we will investigate the subject a little more fully.

Let AB be a tangent line to the earth, at the point A, being the apparent level line; let Aa be the true level, and β the corresponding point to a. Let C be any object above the apparent level line, and b any object below the same level line. The object above would be observed by the theodolite, those beneath (for short distances and for the common purpose of levelling) by the level staves. Let aC be the distance above the positive height ($+h$), and ab the distance beneath, which will then be ($-h$). Now aC is the height above the line AB, βaC is the true height, therefore $\beta a + aC$ is the true height of the point C, above A.



On the other hand, ab , the reading by the level staff, is the distance beneath the apparent level line AB, but the true line is $A\beta$, and βb is the true difference of levels between A and b, which is, therefore, equal to ($-h + b$), or the difference between the two.

Again, for the refraction.

As refraction raises the apparent above the true height of an object, its correction must always lower the apparent height (it is therefore negative $-R$).

If the object be above the apparent level, the error for refraction must be subtracted from the apparent height, thus ($h + C - r$), where $+C$ is the correction, for curvature, and r , is that for refraction, when these values are positive.

But if the object be below the apparent level, we have ($-h + c - r$), that is, the curvature (which is the only positive value) must be subtracted, and the refraction must be added, to the negative value of h , which still remains negative, or beneath the apparent level.

Generally, therefore, let the reader bear in mind, that the correction for the curvature raises the object, and that for refraction depresses it, reducing or increasing its distance from the apparent level, according as the object is above or below it, *i.e.*, has been observed by the theodolite, or read off by the level staves.

As the necessary calculations for curvature and refraction would be exceedingly tedious, in extensive operations, the following method renders them altogether unnecessary.

Set the level in the centre of the object, as nearly as *the eye can tell*, and these corrections for both objects become equal and opposite, and therefore neutralize each other. The apparent level, being always parallel to the chord, that connects two objects, equidistant from the place of observation, must, therefore, have the same versed sine.

Before proceeding to the main level, we will briefly go over the method of conducting the trial and check levels, as they are termed, before taking the final level.

CHAP. IV.

TRIAL LEVELS.

HAVING determined upon the general line of route, the line is marked down upon the Ordnance map, and the several points, where the roads are crossed, are carefully measured from the scale, and determined upon the ground.

The portions of the line between these points are roughly picketed, *if time will permit*, or, if not, their general direction, by the compass, is ascertained, on the plan. The levels are then taken, as near as possible to this direction—the error of deviation being always confined to the intervals between the roads—the relative heights of these points of the roads being always ascertained, and bench-marks taken near. The inclination of the ground, on the right and the left of the line, is also, in the first trial level, carefully marked, so that the engineer may know on which side of the

levelled line to deviate, when he is in want of a piece of cutting, or an embankment.

The trial level, however, is, after all, but very rough work, and serves only as a general check upon the correctness of the subsequent levels.

The heights of the several points upon the roads are useful, as checking their heights by the next level, but the intervals are generally all *out of line*, and never to be depended upon; certainly not, when the direction of the line is not picketed. The only use, then, of this first trial level is, in addition to its furnishing the general face of the country, in a certain direction, its acting as a check upon the height of the roads, and upon the relative heights of the two ends of the line, so as to prevent any serious error in the final Section.

Such being the nature of the first trial level, it may easily be understood how easily errors may creep in in the parliamentary plans, when these level trials and the ORDNANCE MAP are often the only data, that the indecision of companies and the pressure of time, place within reach of the engineer.

CHECK LINES

Are levels taken for the purpose of determining, a second time, the heights between two distant points, which, agreeing with the first, becomes a strong proof of the correctness of the work in detail.

METHOD OF LEVELLING ON A LINE OF RAILWAY.

Having determined upon the point of commencement, select some fixed point, as near to it as possible, whose height being taken, becomes a mark of reference for any subsequent level. This is called a *bench mark*. Place your level about half way between this point and the next onward station, and fixing the legs

firmly in the ground, set the instrument level, and observe the height read off by the staff at the back station: this observation is called a back-sight, or back-set. Turn the instrument round to the bench mark and read off the height there. This is an intermediate, as not being a connecting-link in the consecutive series of backs and foresets. Lastly, read off the height at the forward station, which is termed the foreset, taking care, before each of these observations, to see that the bubble is duly in the centre of the tube. Now take up the level, and place it, as before, between the next two stations, and so on, observing the back and forward readings in every case, and taking, in the way, such bench marks as may appear desirable for the purposes of reference hereafter. These bench marks should be of a permanent character, *near* to the line, and in conspicuous places.

These observations should, in all cases, be carefully taken, and due attention paid, on the part of the Surveyor, to the non-existence of parallax.

The staff-holder has, also, considerable responsibility reposed in him, as all the care on the Surveyor's part would be neutralized by inattention in the placing or moving of the staff. At every station the staff has to be read off twice, in opposite directions, and great care is requisite in turning it round. A peg, or board, or a penny, is sometimes used by the staff-holder for that purpose, so as to keep the staff always in the same point. Much error also frequently arises from the staff not being held perfectly upright. There is some difficulty in keeping it so, and men are apt to become tired and careless. The wind too will often disturb it.

Personally, I have found the greatest difficulty in getting men to keep it upright; they do not understand that it can be out of the perpendicular in two ways; they will often keep it straight, as to it not being too much to their left or right, while, frontwards, it will be considerably out.

This inconvenience is avoided in some level staves by a small plummet in the side of it, which enables the staff-holder to see whether

it is perpendicular in one direction ; and its lateral perpendicularity the Surveyor can judge of for himself.

Method of keeping the Field Book.

Divide the page into the several columns, as in the example, page 268, which are the field notes of a portion of a line of railway from Wandsworth to Croydon, commencing near the Wandsworth station, at a point which is carefully described, and can at any time be easily ascertained, being on the surface of the rail at $\frac{1}{4}$ mile, or 20 chains, *below* the second bridge from the Wandsworth station.

This point, which was in a cutting, was assumed to be 140 feet above an imaginary level, to which the heights, at the several points on the line, bear reference, which is called a Datum Line.

This point might have been considered zero, but as whenever any other points were below this, their height would be negative, the constant changing of, from positive to negative, would be productive of considerable trouble and probable error, especially to those of my readers who might not be quite *au fait* at Algebraical calculation.

The height of the point of starting is put in the column of reduced levels, 140, and the 0·00, in the column of distance, shows that it is the starting point. The instrument is now placed between this point and the next favourable station (not exceeding, in any case, eight or ten chains from the level to the station) in this, from the deepness of the cutting, only 50 links between the *two* stations, and the back reading was found to be 11·04 feet, and the forward reading 1·49. These are placed in their respective columns. Now, as the foreset reads less than the backset, the ground

rises, and the difference between the two readings (9·55) is placed in the column of *Rises*. The height of the ground, where the instrument was placed, of course is not known. The instrument was then removed, to between the second and third station, and the staff turned round, when a second reading of each was taken, and placed in their proper columns. The last reading being less, the ground still rises, and the difference (11·47) is again placed in the column of *Rises*, and its distance (*always from the starting point*) is marked down, viz.:—86 links. Thus we proceed till, at 1 chain 66 links, we get to the top of the cutting, always rising.

Before continuing, we will see how the reduced levels are obtained. These reduced levels are the actual height of each point respectively above the assumed datum or standard line of height

The rises and falls are the differences at each point of its own and the preceding height, and denote whether the ground has risen or fallen between any two consecutive points. By adding, therefore, each rise to the preceding actual height, we obtain the actual height or reduced level of the point, and by deducting this difference where there is a fall, we also obtain the same result.

At the next station the ground falls, the foreset being greater than the intermediate, and 2·61 is placed in the column of falls, which, subtracted from 182·52, the reduced level at the preceding point, gives 179·91, the reduced level at this point. The height being that of the field immediately beyond the ditch on the top of the cutting, the distance was not wanted again, and it was, therefore, made an intermediate, the foreset at

the same station being at a distance of 10·89 from the starting point.

It must be observed generally that there should be no *foreset* without its distance. All B. M. of every kind—the heights of water,—in rivers and drains,—in fields,—on banks,—and every height taken for the purposes of illustration, should be made intermediate.

Having filled the page with the observed backsets, intermediates and foresets, add up the foresets and backsets, their difference will be the difference of height between the starting point and the last point on the page, and it will be equal to the difference between the sums of the rises and falls; it should be also equal to the difference between the assumed datum line and the last reduced height.

Having these three checks, there is no possibility of error when they all agree, and *in all cases every page must be so tested.*

Before calculating the reduced levels, as any error in the rises and falls must induce the same error in the reduced levels, observe first, whether the difference of the rises and falls is the same as that of the sums of the backsets and foresets, you can then safely proceed to the calculations of the reduced levels.

Observe, in placing the different readings, that, in the first line in the column of reduced levels, must be placed the assumed height of the starting point above the datum line; collateral with this must be 0·00 of distance, accompanied with a full and clear description of the exact locality of the starting point. (Consider that you are describing the place to a perfect stranger, who has never been on the spot before.)

On the next lower line, the first back reading must be placed, and, collaterally with it, *in the same line*, the next reading in the *intermediate* column, if it be that of a *bench mark* without a given distance, or of a station, which is not intended to be the backward station to the next observation. If it is, place it in the column of foresets, collaterally with the backset, in the same line. Observe, that every line in the reduced level must have a given height, and that to each of these heights, in the column of distances, there must be a distance given, or an observation of individuality.

The reason of placing the foreset in the same line as the backset is, that the height of each backset point is the height of the preceding foreset, and that it is the difference between the actual backset and foreset upon the same line, that gives the height of the latter.

In calculating the rises and falls, take the difference between the

two readings in the same line, when the one is a back reading, and the other, intermediate or foreset, and place the difference in its proper place, in the same line. Then, if the next reading be an intermediate, it will be placed under the preceding intermediate ; the difference between these two intermediates must be placed in the same line as the latter, and so on throughout any number of intermediates. After these intermediates must, of course, come a foreset point, before the instrument can be removed, which foreset becomes a backset point of observation at the next placing of the level. The difference must be taken between this foreset and last preceding intermediate, and placed collaterally in the same line with the foreset.

This closes one portion of the line of section, and another is recommenced that would be totally unconnected with the former were it not that the last point in that becomes the first point in this,

Collaterally with the reduced levels, if these levels refer to benchmarks, write B. M. in large characters in the column of distances, and the nature of it under the head of remarks. If the reduced levels refer to points taken out of the line, which is frequently the case in trial levels to avoid obstacles, write "out of line;" also, in the column of distances, carry the distances on always from the starting point to 80 chains, and, if an observation be taken at that point, which should be done if possible, write 1 mile against it, and begin the chaining afresh,—continue to another 80 chains, marking this point 2 miles ; begin here again also, and proceed as before.

CHAP. V.

THE first page of the following notes has been completed, the rises and falls calculated, and the reduced levels put in. The other pages contain only the several readings at the different stations, and the respective distances or observations to each ; the rest must be filled in by the student.

TRIAL LEVELS

On a proposed line of Railway between Wandsworth and Croydon.

| Backset | Inter. | Foreset | Rise | Fall | Reductn. Levels | Dist. | Remarks |
|---------|--------|---------|------|-------|--------------------|--------------------------------|---|
| 11.04 | | 1.49 | 9.55 | | 140.00 | 0.00 | Datum line being 140 ft. below the level of the sea. |
| 11.60 | 0.23 | 11.37 | | | 149.55 | .50 | Commencing at the surface of the rails opposite the centre of the watch box, $\frac{1}{4}$ mile below the second bridge, from the Wandsworth station. |
| 11.50 | 0.39 | 11.11 | | | 160.92 | .86 | |
| | | | | | 172.03 | 1.23 | |
| 11.61 | 1.12 | 10.49 | | | 182.52 | 1.66 | top of cutting of railway |
| 6.04 | 8.65 | | | 2.61 | 179.91 | | in field |
| | | 4.79 | 3.86 | | 183.77 | 10.89 | |
| 5.18 | 7.05 | | | 1.87 | 181.90 | | within field |
| | | 8.20 | | 1.15 | 180.75 | 18.80 | in field beyond |
| 3.31 | 11.97 | | | 8.66 | 172.09 | 26.08 | |
| 1.23 | 9.95 | | | 8.72 | 163.37 | 29.84 | |
| 2.92 | 7.28 | | | 4.36 | 159.01 | | in field |
| | | 9.84 | | 2.56 | 156.45 | 35.68 | in field beyond |
| 0.42 | 10.55 | | | 10.13 | 146.32 | 43.91 | |
| 0.55 | 6.94 | | | 6.39 | 139.93 | 52.39 | |
| 4.40 | 5.10 | | | 0.70 | 139.23 | 63.31 | |
| 3.66 | 2.97 | 0.69 | | | 139.92 | B M | being hook of lower hinge of gate in lane |
| 2.53 | 5.40 | | | 2.87 | 137.05 | 63.83 | centre of road No. 1 BURNTWOOD LANE |
| | | 2.42 | 2.98 | | 140.03 | 64.33 | in field beyond hedge and ditch 64.10 and 64.20 |
| 4.90 | 8.73 | | | 3.83 | 136.20 | | in gravel pit |
| | | 4.69 | 4.04 | | 140.24 | 71.70 | 71.60 path crosses, 71.76 hedge crosses |
| 5.40 | 4.84 | 1.56 | | | 141.80 | 0.15 | 1 MILE. 76.40 & 79.70 hedges cross |
| 4.82 | 3.72 | 1.10 | | | 142.90 | 11.28 | 520 hedge, 11.03 ditch |
| 1.00 | 2.35 | | | 1.35 | 141.55 | B.M. on rail by tree near shed | |
| | | 6.27 | | | 3.92 | 137.63 | 18.70 18.78 and 21.74 hedge and fence |
| 4.77 | 9.54 | | | 4.77 | 132.86 | 26.10 | 30.08 and 30.70 garden fence |

TRIAL LEVELS CONTINUED.

| Backset | Inter. | Foreset | Dist. | Remarks |
|---------|--------|---------|-------|--|
| 4·11 | | 2·09 | 31·02 | |
| 4·95 | 3·15 | | B. M. | By brick wall, Occupation-road, same level |
| | | 3·22 | | on lower hinge of gate of blue rails |
| 5·20 | | 2·45 | | out of line |
| 7·37 | | 4·18 | | out of line |
| 3·48 | 6·03 | | 47·53 | Centre of road No. 2, London and Tooting road |
| | | 4·50 | B. M. | nut of lower hinge of white gate |
| 5·06 | | 2·51 | 57·48 | 52·53 ditch crosses |
| 7·67 | | 0·45 | | out of line 5914, and 6296, fence & ditch cross |
| 8·83 | | 0·68 | 75·00 | 69·70, and 76·30, ditch and fence cross |
| 9·95 | | 0·70 | 19·75 | 2 MILES |
| 9·86 | | 0·60 | 2·80 | 325 and 380, fences of road cross |
| 8·97 | 11·78 | | | Centre of road No. 3 Church-lane |
| | | 0·55 | 5·80 | in field beyond |
| 10·32 | | 0·96 | 9·30 | within field near fence of lane |
| 2·01 | 5·92 | | 9·80 | Centre of road No. 4, Back-lane |
| | | 0·00 | B. M. | on tree in Back-lane blazed (with nail) |
| 8·73 | 0·21 | | B. M. | on top of post near drain on left of line & road |
| | | 0·25 | | out of line |
| 4·34 | | 1·60 | | out of line |
| 0·40 | | 11·77 | | out of line |
| 0·40 | | 9·65 | | out of line |
| 2·72 | 6·10 | | B. M. | on top of post, on right, near corner of wall |
| | | 8·25 | | out of line |
| 5·50 | | 4·96 | | out of line |
| 5·40 | | 3·70 | | out of line |
| 5·09 | 2·81 | | B. M. | on stump near gate at culvert |
| | | 2·15 | | out of line |
| 5·00 | | 1·00 | | out of line |
| 8·05 | | 7·02 | | out of line |
| 1·30 | | 11·35 | | out of line |
| 1·86 | 2·08 | | B. M. | on top of further side of gate, other side of road |
| | | 6·85 | 48·30 | Centre of road No. 5 |
| 5·56 | 1·48 | | 48·12 | 4812 and 48·48 fences of road cross |
| | | 4·52 | 48·52 | in field beyond |
| | | 6·55 | 49·82 | |
| 1·36 | | 10·93 | 57·58 | 59·30, ditch crosses |
| 1·57 | 6·41 | | 59·63 | 60·70, ditch crosses |
| | | 6·19 | 61·03 | |
| 10·65 | 5·53 | | 64 00 | |
| | | 4·15 | 71·25 | 6900 fence, 71·86 ditch |
| 5·70 | | 4·03 | 79·18 | |
| 5·80 | 3·72 | | 3·78 | 3 MILES, 390 and 430 fences |
| | | 4·30 | | Centre of road No. 6 |
| | 4·00 | | B.M. | top of near post of black gate on left of line |

CHAP. VI.

PLOTTING OF THE MAIN SECTION LINE.

DRAW first the DATUM LINE in ink, carefully and finely ; then lay off the several miles upon it, by means of a 12-foot ivory rule. The usual scale for sections of railways, deposited for Parliament, is 20 chains to the inch, or 4 inches to the mile. These distances must be carefully gone over once or twice to guard against error, and the number of miles must be marked against each. The method of chaining on a line from the beginning, and dividing the length into miles on the ground, confines every error of plotting within that distance, as the distances are always taken from the last mile point—so many miles + (plus) so many chains, till the next mile mark, when it begins again.

At every third mile, as well as at the beginning and ending, erect perpendiculars to the line, geometrically by the compasses, to about twice the height of the datum line, and draw a line through the distance parallel to the datum line ; measure off upon this line the intermediate miles, and connect them with the corresponding points in the datum line.

Now, lay off, upon the datum line, the several distances on the column of distance for one mile, and draw very fine lines therefrom, parallel to the perpendiculars. These parallels, or supposed perpendicular lines, being confined within every mile, cannot be far wrong. In fact, the correctness of the section line depends upon their being truly perpendicular, or the distances in the section line, though correctly measured on the datum line, might be incorrectly projected.

Upon these perpendiculars measure carefully off, with a fine needle, on a scale of 100 feet to an inch, the several heights in the reduced levels corresponding to the distances, carefully distinguishing the roads and rivers, the position of hedges, &c., as in the accompanying plan, and connect their several heights ; *put this in ink at once*, while the plotting is fresh in the memory, marking off in pencil the “reduced level” of the centre, against every road, the surface of the water of rivers, &c., and also the height of the ground at every half mile.

One mile being now finished, and not before, proceed to the next ; finish that before you commence the following, and so on throughout.

CHAP. VII.

CROSS LEVELS.

Cross Section on line of London and Southampton Railway.

| Backset | Inter. | Foreset | Dist. | Remarks |
|---------|--------|---------|-------|---|
| 7.80 | 7.24 | | 0.00 | Commenced at surface of rails opposite the centre of the watch box, $\frac{1}{4}$ mile below the 2nd bridge, from Wandsworth Station. |
| | 6.98 | | 3.00 | |
| | 6.85 | | 4.00 | |
| | 6.97 | | 6.00 | |
| | | | 8.00 | |
| | | 7.40 | 10.00 | |
| 4.74 | 5.03 | | 12.00 | |
| | 5.30 | | 13.00 | |
| | 5.49 | | 16.00 | |
| | 5.75 | | 18.00 | |
| | | 6.05 | 20.00 | Ground level with rail line hereabouts. |
| 3.57 | 3.78 | | 22.00 | Being surface of rail at the chain opposite the intersection of a drain, with railway on the right side, downwards. |
| | 3.92 | | 24.00 | |
| | | 4.18 | 26.00 | |
| | | | B. M. | |

*Cross Section No. 1, Burntwood Lane**Cross Section No. 2, London to Tooting.*

| Backset | Inter. | Foreset | Dist. | Backset | Inter. | Foreset | Dist. |
|---------|--------|---------|--------|---------|--------|---------|--------|
| 0.83 | | | 0.00 | 0.15 | | | 0.00 |
| | 1.84 | | 1.00 | | 1.04 | | 1.00 |
| | 2.87 | | 2.00 | | 1.86 | | 2.00 |
| | 3.27 | | 3.00 | | 2.79 | | 3.00 |
| | 3.79 | | 4.00 | | 5.87 | | 4.00 |
| | 4.82 | | 6.00 | | 6.00 | | 6.00 |
| | 5.35 | | 7.00 | | 6.83 | | 7.00 |
| | 6.04 | | 8.00 | | 7.54 | | 8.00 |
| | 6.66 | | 9.00 | | 8.15 | | 9.00 |
| | | 7.55 | *10.00 | | 8.50 | | +10.00 |
| | 1.43 | | 11.00 | 2.14 | 6.90 | +B. M. | 11.00 |
| | 2.60 | | 12.00 | | | | |

* Line crosses.
Reduced level, 137.05.

+ Line crosses. Reduced level,
130.98.
† On nut of lower hinge of white
gate.

CROSS SECTIONS CONTINUED.

No. 3, Church Lane.

| Backset | Inter. | Foreset | Dist. |
|---------|--------|---------|-------|
| 0·56 | | | 0·00 |
| | 2·31 | | 1·00 |
| | 7·23 | | 3·00 |
| | | 10·53 | 4·00 |
| 0·14 | 7·45 | | 6·00 |
| | | 11·45 | 7·00 |
| 0·72 | | 10·51 | 9·00 |
| 0·43 | 4·36 | * | 9·82 |
| | | 9·97 | 11·00 |
| 0·12 | 7·68 | | 13·00 |
| | | 10·37 | 14·00 |
| 0·07 | 2·32 | | 15·09 |
| | 6·27 | | 17·00 |
| | 8·02 | | 18·00 |
| | 9·22 | | 19·90 |
| | | 10·05 | 20·00 |

* Line crosses 32 down hill from fence on left of line. Reduced level, 175·35

No. 4, Back Lane.

| Backset | Inter. | Foreset | Dist. |
|---------|--------|---------|---------|
| 1·07 | | | 0·00 |
| | | 1·81 | 1·00 |
| | | 2·80 | 2·00 |
| | | 6·58 | 4·00 |
| | | 8·76 | 5·00 |
| 0·91 | | 10·55 | 6·00 |
| | | 7·61 | 8·00 |
| 2·75 | | 10·67 | 9·00 |
| | | 6·51 | 10·00 |
| | | 7·22 | † 10·25 |
| | | 1·30 | ‡ B.M. |
| 0·53 | | 9·48 | 11·00 |
| | | 0·41 | 8·24 |
| | | 0·56 | 15·00 |
| 1·53 | | 7·84 | 17·00 |
| | | 6·59 | 19·00 |
| | | 7·95 | 20·00 |

† Line crosses. Reduced level, 192·30.

‡ On blazed tree

Cross Section No. 5.

| Backset | Inter. | Foreset | Dist. |
|---------|--------|---------|----------|
| 6·04 | | | 0·00 |
| | 5·70 | | 1·00 |
| | 4·20 | | 3·00 |
| | 3·10 | | 4·00 |
| | | 2·34 | 5·00 |
| 5·15 | 0·71 | | B. M. |
| | 3·85 | | 7·00 |
| | | 4·21 | 8·00 |
| 4·95 | 0·42 | | B. M. |
| | 6·80 | | 11·00 |
| | 8·47 | | 12·00 |
| | | 10·27 | 13·00 |
| 1·21 | 5·77 | | 14·00 |
| | | 8·13 | 15·00 |
| 4·75 | 1·50 | | \$ B. M. |
| | | 7·79 | 17·00 |
| 0·64 | | 8·15 | 19·00 |
| 0·47 | | 8·27 | 21·00 |
| 0·69 | | 7·39 | 23·00 |
| 2·76 | | 5·75 | 25·00 |

§ Line crosses at 16·63 chs. Reduced level, 184·39.

CHAP. VIII.

LEVELLING AND PLOTTING OF THE CROSS
SECTIONS.

HAVING, in the course of proceeding with the main section, carefully marked upon the ground the exact spot in the centre of the road, where the line crosses, and taken the height of the nearest bench mark to it, about ten chains on either side of this spot are measured, in chains' lengths, along the road, for the purpose of obtaining the height of the road at every chain.

In taking the cross levels, be careful always to level one way, *from left to right*, that is, measure always from the left side of the line, (looking in the direction the main section has been levelled, or towards the termination of the line,) to the centre, where the line crosses, onwards to the right side of the line. This will prevent error. Unless the number of chains levelled on either side, of where the line crosses, be always the same, which, depending upon the inequality of the section line of the road, is seldom the case, the consequences of neglecting this might be serious, as the line would cross in its wrong place, and the calculations for lowering or raising the surface, for the bridge across it, would be altogether incorrect.

Call these chains' lengths, therefore, in your level book (*beginning as far, on the left of the line, as from the nature of the ground may be necessary, and numbering to the centre*), 1 chain, 2 chains, and so on.

Place your level opposite to the centre stake, between 0 chains and where the line crosses, and take the several readings at every chain's full length, and also at the spot where the line crosses, and should the bench mark, before alluded to, be on that side, take that also; then remove your level to midway between the centre, where the line crosses, and the extreme point you intend levelling to, and take the several readings.

Enter these in your book, according to the examples given, making the reading at 0 chains your first back set, at 1 chain your first intermediate, placing it in the next lower line, so that you may be enabled to ascertain the height of the starting point, 0'00 in the column of distances. Mark all the subsequent readings, taken at the same place, *intermediate*, excepting that where the line crosses, which should be first *foreset*, saying in every case, in the column of remarks, line X. The B. M. will be an intermediate, as usual.

Having now obtained all the field data necessary, calculate carefully the rises and falls, using the ordinary checks to ensure accuracy. Then turn to the notes of your main section, and observe there the height of the B.M., and of that part of the road where the line crosses; enter these in the column of reduced levels, as the reduced levels of the same points, taken in the cross section, and, determining the subsequent portion of the section as usual, *reverse the calculations of the upper portion*. By this means, you obtain the several heights of the road, relatively to the whole section, and as the height of where the line crosses, obtained by the section levels, should be the same as that by the main section, you have a certain check upon the general accuracy of the levelling, as well as a special proof that the *correct* bench mark, and point of crossing of the road, have been taken.

Now plot the sections of the several cross roads, numbering them, carefully, No. 1, No. 2, &c., to agree to the corresponding roads in the main section, marking where the line crosses.

The scale, generally used for these cross sections, is 5 chains horizontal, and 50 feet vertical.

The section being now completed, the field book is no longer wanted, and the subsequent proceedings depend upon the skill and judgment of the engineer, to overcome, in the selection of his gradients, such local difficulties of hills or hollows—of roads and rivers—as the face of the section may present.

In determining the lines of gradients, practice and experience are the best masters, and I refer my readers to them.

CHAP. IX.

Method of determining the heights, that the several roads should be raised or lowered, and whether they should be passed over or under.

When a road approach is *passed over* by the line, the usual distance allowed, from the surface of the road to the surface of the rails, is 20 feet; when the approach is *passed under*, the allowance is 19 feet.

This being premised, we will proceed to the calculations of roads, Nos. 1, 2, and 3, in the given section, and leave the remainder to the reader for practice.

The heights of the several gradients selected, were, 140 feet at starting, which running level for a quarter of a mile, then ascended at a rate of 1 in 354, to the centre of road No. 2, (at a distance of 1 mile, 47 chs. 53 links, from the starting point,) where it is 160 feet above the *datum* line; the gradient then changes into one of 1 in 413 to the 3 mile, where its height is 178 feet. The gradient line in the plan, it must be remembered, is *the line of the surface of the rails*, and not what is called the *balance* line, or ground line of the ballasting, which is so called, from its being the line adopted in the calculations, of both the cuttings and embankments; the earth, in the former, being dug out, and in the latter, filled up, as far as A; upon this, afterwards, is placed the ballasting, which, in the calculations of earthwork, is reckoned at 2 feet.

The distance of the first road (cross section No. 1), by turning to the main field book, will be found to be 63·83 chains, it falls, therefore, in the second division of gradients, being 140 feet at one end, and 160 at the other. Find the length of this gradient, which begins at 20 chains, and ends at 127·53 chains, being, therefore, 107·53 chains long. Knowing the length and excess of height at the end, the excess of height, at any point upon it, can be calculated by similar triangles, thus—

As the whole distance, 107·53 chains : 20 feet :: 23·83 chains,

the distance of the road No. 1, from the commencement of the gradient: its corresponding excess at that point, viz., 5·45 feet, which added to 140 feet, its height at starting, gives 145·45, the height of the surface of the rails, where the line crosses the centre of the road No. 1. By turning again to the field book (to the reduced levels), the height of the surface of the road is 137·05 feet, making the rail line 8·45 feet higher than the road line, or surface of approach.

If the road can be conveniently lowered, lower it, so as to make it 20 feet beneath the rail line; to effect this, it must be lowered 20 feet—8·15 feet or 8 feet 15 inches, and passed over.

In the same way, it may be found, that at road No. 3, the surface of rails is 165·75 feet, while that of the approach is 175·35, making the approach 9 feet 7 inches *higher* than the rail. The road, therefore, must be raised 19 feet—9 feet 7 inches, or 9 feet 5 inches, and passed *under*.

When, as in the case of road No. 2, the surface of the rails crosses a road, 20 feet, or more than 20 feet above it, the road remains the same, *the level is unaltered*.

EXAMPLES.

The reader is now recommended to calculate for himself, roads Nos. 2, 4, and 5, and see if he makes them agree with the plan.

At No. 2, the line is 20 feet higher than the road; at No. 4, the road is 25½ feet above the line; and at No. 5, the road is to be raised 8 feet 10 inches, and passed under.

CHAP. X.

Calculations of the superficial areas, occupied by the railroad, and of the solid contents of the cuttings and embankments.

SUPERFICIAL AREAS.

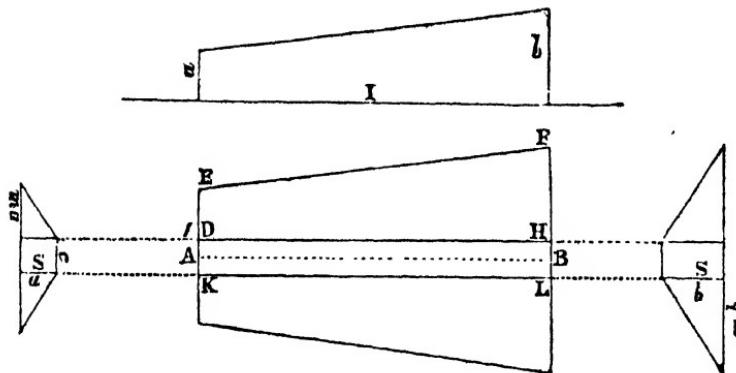
HAVING determined the effects of the line in the altering of the approaches, the next thing to be ascertained, is the superficial area that it will occupy, so as to estimate the expence of purchasing the ground. This will, of course, depend upon the heights or depths of the cuttings and embankments, upon the width of the line, and the proportion of the slopes.

In making these calculations, much depends upon the object in view and the pressure of time. In preparing for deposit, the difference of heights between the section line and the *balance* line are taken

by the compasses ; while, in preparing for the contracts, it is requisite they should be calculated from the field book, and the rates of inclination in the section. Again, for deposit, these differences are only taken at any apparent change of inclination in the ground line, while for the contracts they must be made for every two chains.

Being able to obtain these differences of heights, whether by compasses (from the section), or by calculations (from the field notes), as the case may require, we will take some general case, and explain the best method of calculating the areas of the several blocks.

Let a , b , be the several heights ; let c be the width of the railway ; and let the slopes be as m to 1 (by the term "slopes" is meant, technically, that the base of the slope is to the height of the slope as $m : 1$) ; therefore, if the heights be a , b , the base of the slopes will be ma mb .



Let AB be any length of line, whose height at A is (a), and at B is (b). Then, if $ADHB$ be taken as the half of the line, DE and HF will be the extent of the slopes on the one side, and DE and HF will be respectively equal to ma and mb ; and, therefore, the area of $AEFB$ will be $\frac{c+ma+mb}{2} \times \pi$, where π equals the length, but $AEFB$ is only half the area ; therefore the whole area of the block equals $(c+ma+mb) \cdot \pi$.

EXAMPLE.—Required the superficial area of a portion of railways, 20 chains long and 12 feet high at one end, and 10 feet at the other, the slopes being 2 to 1 (the width of line being 33 feet); now $c=33$, $a=12$, $b=10$, and $m=2$, and $\pi=20$ chs. ; therefore,

$$\frac{33+23+20}{66} \times 20 = 23.333 \text{ sq. chains} = \begin{array}{r} \text{A. R. P.} \\ 2 \ 1 \ 13. \end{array}$$

EXAMPLE.—What will be the superficial area of a cutting of a railroad, whose depths at every chain are 0, 2, 3, 5, 7, 9, 8, 6, 4, 2, 1, 0 feet; the other data as in last example?

The formula for the area is $(c+ma+mb)\pi$; in the two end areas, in the present example, where $a=0$, and $b=0$, ma and mb would be 0; and the areas will be $(c+mb)\pi$; in the one ease, and $(c+ma)\pi$, in the other.

Now the whole area = $\frac{\pi}{66} \left\{ (33+4) + (33+4+6) + \&c. \right\}$; or, by taking separately the whole of the *central area*, which will be (11 chains $\times \frac{33}{66}$), and using the formula (given in the first part, for calculating the areas of offsets, page 75), for the areas of the *slopes*, we have for their area $2\pi \frac{(4+6+10+14+18, \&c.)}{66}$ sq. chs.; therefore, the whole area = $\frac{1}{66} \times \left\{ \frac{33 \times 11 + 2\pi(4+6+10+\&c.)}{A. R. P} \right\}$
 $= 8.34$ square chains = 0 3 13.

EXAMPLE.—What will be the area of a mile of railway, whose heights of embankment for the first quarter of a mile, at every 5 chains, are 12, *25, 50, and 20 feet; for the next 25 chains, the line is level; and then, to the end, runs through a cutting, which rises gradually to the height of 40 feet, at half way, and descends to the end, having the other data as the preceding?

| |
|---------------------|
| A. R. P. |
| <i>Ans.</i> 15 3 15 |

* In this case, area of each slope equals $d \left(\frac{bg}{2} + ch + dl + \&c. \right)$ see page 76, where, in the given formula, the perpendicular at the starting point = 0. In the section of a cutting or embankment, the area of the slopes would then be $d \left(\frac{bg}{2} + ch + dl + em + \frac{fn}{2} \right)$

CHAP. XI.

CUTTINGS AND EMBANKMENTS.

THERE are two or three methods adopted in practice, in the calculation of these quantities, viz., the Prismoidal Formula, Bidder's Tables, M'Niel's, &c.

Prismoidal Formula.

Let c be any cutting, having, at one end, the height $= a$, at the other $= b$; length of cutting $= \pi$, slope 2 to 1, or generally m to 1.

Now the solid, thus cut off, assumes the form of an imperfect prism, which may properly be divided into three divisions; the *central* one, being a solid, generated by a plane of the given heights and length; moving along a plane, at right angles to it, to a distance, equal to the width of the railway; and the two slopes, equal to each other, being strictly frusta of pyramids; the height of the frusta being equal to the length of the cutting, and the sides of either end, being the sides of a right-angled triangle, whose base $=$ the height of the cutting at that end; and perpendicular, the proportion of the given slope to it.

Having the same data as above, (*See Diagram, Page 277.*)

the central contents $= \pi \frac{c}{2} (a+b)$, where $c =$ width of railway

$$\text{slopes} = \frac{\text{areas of the two ends} + \text{the mean area}}{3} \times \text{the length}$$

$$= \frac{\pi}{3}(ma^2 + mb^2 + mab)$$

$$\therefore \text{The whole contents} = \frac{\pi}{2} c (a+b) + \frac{m}{3}(a^2 + b^2 + ab)$$

$$= \frac{\pi}{6}(3ac + 3bc + 2a^2m + 2b^2m + 2abm)$$

$$= \frac{\pi}{6}(ac + a^2m + bc + b^2m + 2ac + 2bc + m(a+b)^2)$$

$$= \frac{\pi}{6}(c+ma. a + c+mb. b + 4(\frac{a+b}{2}).(c+\frac{m}{2}. a+b))$$

which is the *prismoidal formula*; where $\overline{c+ma}$, a and $\overline{c+mb}$. b are areas of the two ends, and $4(\frac{a+b}{2})(c+\frac{m}{2}. \overline{a+b})$ is 4 times the area of a section midway; the whole area being equal to the sum of these three into $\frac{1}{6}$ the length, whether of cutting or embankment.

2. Moseley's Formula.*

$$\text{The central area} = \pi \frac{c}{2} (a+b) = \frac{\pi m}{6} \left(\frac{3c}{m} \cdot a+b \right)$$

$$\text{slopes} = \frac{\pi}{3} m (a^2 + b^2 + ab) = \frac{\pi m}{6} (2a^2 + 2b^2 + 2ab)$$

$$\therefore \text{whole contents} = \frac{\pi m}{6} \left(\frac{3c}{m} (a+b) + 2a^2 + 2b^2 + 2ab \right)$$

$$\text{Now let } a = y_1 \text{ and } b = y_2, \text{ we have 1st contents} = \frac{\pi m}{6} \left(\frac{3c}{m} (y_1 + y_2) + 2y_1^2 + 2y_2^2 + 2y_1 y_2 \right).$$

Now, as the adjoining cutting must commence with the same height as this terminates, supposing the other height of the second cutting = y_3 , and next height y_4 , we have—

$$\text{area of 1st cutting} = \frac{\pi m}{6} \left(\frac{3c}{m} (y_1 + y_2) + 2y_1^2 + 2y_2^2 + 2y_1 y_2 \right)$$

$$\text{2nd cutting} = \frac{\pi m}{6} \left(\frac{3c}{m} (y_2 + y_3) + 2y_2^2 + 2y_3^2 + 2y_2 y_3 \right)$$

and the series = $\frac{\pi m}{6} \left(\frac{3c}{m} (y_1 + 2y_2 + 2y_3 + \dots + 2y_{n-1} + y_n) + 2(y_1^2 + 2y_2^2 + 2y_3^2 + \dots + 2y_{n-1}^2 + y_n^2) + 2(y_1 y_2 + y_2 y_3 + y_3 y_4 + \dots + y_{n-1} y_n) \right)$ or B,

$$\text{and } \therefore = \frac{\pi m}{6} \left(2 \sum y_n \cdots (y_1 + y_n) + 2(2 \sum y_n^2 - (y_1^2 + y_n^2)) + 2(B) \right).$$

Now, when y_1 and y_n are = 0, which is the case in a complete cutting or embankment; we have—

$$\text{Solidity} = \frac{1}{6} \pi m \left(\frac{3c}{m} \sum y_n + 4 \sum y_n^2 + 2(B) \right),$$

when $\sum y_n$ = the sum of the simple quantities,

and $\sum y_n^2$ = the sum of their squares,

and B = the sum of the continued products of the simple quantities.

* This formula, for which I am indebted to Professor Moseley, of King's College, I have had several opportunities of testing, and I have found it exceedingly useful. The calculations from the Prismoidal Formula, which is, geometrically deduced, are strictly accurate; but they are too tedious for general purposes. Bidder's tables, on the other hand, though convenient for common practice, are still, from being only calculated to full feet and limited to 50 feet, unfitted for contract estimates. Whereas Moseley's formula, being applicable to any height and any subdivision, and embracing any extent of cutting, whose heights are taken at any equal distances whatever, becomes especially useful for the final calculations; which are usually made at every two chains of distance, and to heights of one hundredth of a foot.

3. *Bidder's Formula,*

for computing the solid contents, is—

$$\text{for the slopes } = \frac{22}{27} ((a+b)^2 - ab) \quad \left. \begin{array}{l} \\ \end{array} \right\} \text{in yards.}$$

$$\text{for the centre } = \frac{11}{9} (a+b)$$

This is for a chain in length, and in the centre for a foot wide ; the slopes being one to one.

Now, it has been previously shown that—

$$\frac{\pi c}{2} (a+b) = \text{contents of centre.}$$

Let $c = 1$ foot,

$$\text{and } \pi = \text{one chain or 66 feet, } \therefore \frac{66}{2} (a+b) \text{ in cubic feet } = \text{centre.}$$

$$\frac{66}{2} \text{ of } \frac{1}{27} \text{ or } \frac{11}{9} (a+b) \text{ in cubic yards } = \text{centre.}$$

$$\text{And the slopes } = \frac{\pi m}{3} (a^2 + b^2 + ab).$$

Let $m = 1$, and $\pi = 66$ feet.

$$\therefore \text{slopes } = \frac{66}{3} \text{ feet, or } \frac{22}{27} (a^2 + b^2 + ab) \text{ in yards,}$$

$$\text{and } \therefore = \frac{22}{27} (\overline{a+b}^2 - ab) \text{ cubic yards.}$$

The central contents, thus obtained, would have to be multiplied by the length of the sections in chains, and the width of the line in feet; and the contents of the slopes, by the rates of m to 1 ; and by the length of the section in chains.

M'Neil's Tables are also for slopes of 1 to 1, and for base of 1 foot; but not for lengths of 1 chain.

The quantities obtained, therefore, in M'Neil's tables, would have to be multiplied, for the centre, by the width of railway in feet ; and, for the slopes, by the length π (in feet).

These two tables, however, are only calculated for long sections, and for full feet, (for which they are invaluable,) principally, for computing the whole contents of a line previous to going to Parliament, to form the first estimate of the expense.

EXAMPLE.—Required the solid contents of a cutting or embankment, whose several heights at each chain's length are, 0, 5, 10, 15, 20, 25, 30, 30, 24, 18, 15, 10, 0—slopes 2 to 1, and the width of railway, 30 feet.

BY BIDDER'S TABLES.

| Lengths | Heights at either end. | Central Contents | Contents of Slope |
|---------|------------------------|------------------|-------------------|
| 1'00 | 2 | 6·1 | 20 |
| 1'00 | 5 | 18·3 | 143 |
| 1'00 | 10 | 30·6 | 387 |
| 1'00 | 15 | 42·8 | 754 |
| 1'00 | 20 | 55·0 | 1243 |
| 1'00 | 25 | 67·2 | 1854 |
| 1'00 | 30 | 73·3 | 2200 |
| 1'00 | 35 | 66·0 | 1789 |
| 1'00 | 40 | 51·3 | 1085 |
| 1'00 | 45 | 40·3 | 667 |
| 1'00 | 50 | 30·6 | 387 |
| 1'00 | 55 | 12·2 | 82 |
| | | 493·7 | 10611 |
| | | 30 | 2 |
| | | 14811·1 | 21222 |
| | | | 14811 |
| | | | 36033 |

493·7 being the number of cubic feet in the central column, for the width of one foot, which must, therefore, be multiplied by 30 for the whole width, and 10611 being the contents of the slopes for the ratio of one to one, which must, in this case, at the ratio of two to one, be doubled; the sum of the two, 36033, will be the number of cubic yards required.

BY MOSELEY'S FORMULA.

The sum of the simple quantities = 202 = $\frac{1}{2} y_n$

the sum of their squares = 4400 = $\frac{1}{2} y_n^2$

the sum of their products

(of each with its preceding) = 4222 = B

Now the formula = $\frac{\pi m}{6} \left\{ \frac{8c}{m} 2 \frac{1}{2} y_n + 4 \frac{1}{2} y_n^2 + 2 B \right\}$

$2 \frac{1}{2} y_n = 404, \frac{8c}{m} = 45; 4 \frac{1}{2} y_n^2 = 176000, 2 B = 8444$

$\therefore \frac{1}{2} 66 (404 \times 45 + 176000 + 8444) =$ cubical contents.

$404 \times 45 = 18180$

17600

8444

$\frac{42224 \times 22}{1000} = 972928$ cubic feet = 36034 cubic yards,

differing by one cubic yard only, from the previous result by Bidder's tables. The calculation of this by the *Prismoidal Formula* would be too tedious an operation. Of the two methods given, Bidder's tables have the advantage where the numbers are high, while the accuracy of calculation required for contract work, when the heights at every chain may be decimals of a foot, can only be done by Moseley's, Bidder's being calculated to feet only.

EXAMPLE.—Calculate, by Moseley's formula and Bidder's tables, the cubical contents of an embankment, whose heights at every chain are 0, 2, 3, 5, 7, 9, 8, 6, 4, 2, 1, 0; slopes 2 to 1; width of centre, 33 feet.

Ans. 5178 cubic yards.

EXAMPLE BY THE PRISMOIDAL FORMULA.—What are the contents in cubic yards of a cutting, where the heights at every 2 chains are, 0, 3, 5, 7·5, 8, 9·5, 8, 6, 4, 0, the width of the line being 33 feet, and the slopes 2 to 1.

Now $c + \frac{ma}{2}$. a = area at one end.

$c + \frac{mb}{2}$. b = area at other.

$$\frac{a+b}{2} (c + \frac{m}{2} a + b) = \text{area midway.}$$

$33 + 6 \times 3$, $33 + 10 \times 5$, $33 + 15 \times 7\cdot5$, $33 + 16 \times 8$ &c., are the several end areas, and

$$\frac{3}{2}(33 + 3)$$
, $\frac{3+5}{2}(33+3+5)$, $\frac{5+7\cdot5}{2}(33+5+7\cdot5)$, &c.,

are the several areas midway.

Now the sum of the end areas = 2404 square feet, and that of the middle area = 2380.

Four times the middle area, *plus* twice the end areas (as the areas at every height serve for end areas to two sections), multiplied into $\frac{1}{3}$ the common length, will be the contents in cubic feet.

$$\frac{2(2404) + 4(2380) \times 132}{3} = 315216 \text{ cubic feet} = 11675 \text{ cubic}$$

yards, the solid contents required.

Instead of the preceding, however, there are two methods in common use among contractors, which, though apparently correct, are really far from being so.

The first is by taking the MEAN OF THE TWO END AREAS, which make the results *too much*.

The second, by taking THE AREA OF THE MEAN HEIGHT, which, on the other hand, makes it *too little*.

Now, in the proof of Bidder's formula, it has been shown that the contents of slopes $= \frac{\pi}{3} (m(a+b)^2 - ab)$, and of the centre $= \frac{\pi c}{2} \cdot a + b$

therefore the whole contents are $= \pi (\frac{m}{3} (a+b)^2 - ab) + \frac{a+b}{2} \cdot c$

and by assuming x and y , as the unknown values of the respective excess and deficiency in the two erroneous methods above, and equating them with the true formula, we can obtain their respective values.

Of these two incorrect formulæ

$$\text{the first } \pi (\frac{c+ma}{2} \cdot a + \frac{c+mb}{2} \cdot b) = \frac{3ac + 3ma^2 + 3bc + 3mb^2}{6}$$

$$\text{the second } = \pi \cdot \frac{a+b}{2} (c + \frac{m(a+b)}{2}) = \frac{6ac + 6bc + 3ma^2 + 6mab + 3mb^2}{6}$$

Finding the differences between these and the correct formula, which are the values of x and y respectively, we obtain

$$\text{an excess for the first of } \frac{m}{6} \cdot \frac{a-b}{a+b}^2$$

$$\text{a deficiency for the second of } \frac{m}{12} \cdot \frac{a-b}{a+b}^2$$

where a and b , as usual, represent the heights at the ends, and m the ratio of the slopes.

These corrections have to be multiplied by π , the length of the section.

THE END.

CORRECTED AND OMITTED ANSWERS.

| Page. | Ex. | |
|-------|-----|---|
| 24 | 2 | 49 acres, 0 roods, and 10 perches. |
| 25 | 4 | 231384 square yards. |
| 29 | 4 | For AF read AD. |
| | 8 | For DE read AF. |
| | 3 | For BC, 6·50, read FC, 6·50. |
| | 3 | For DB read AC. |
| 32 | | For 6 read 0. |
| 34 | 2 | 57 acres, 1 rood, 10 perches. |
| | 3 | 9 acres, 2 roods, 0 perches. |
| 35 | 3 | 10 acres, 1 rood, 26 perches. |
| 46 | 3 | Comes under Case 2. |
| 48 | 2 | Base = 73·99 chains. { \angle at B = $33^{\circ} 4'$ { \angle at C = $29^{\circ} 6'$ |
| | 3 | In the question, should be 70 chains and 50 chains. |
| 120 | 4 | 14 acres, 1 rood, 6 perches. |
| 122 | 2 | 16 acres, 2 roods, $12\frac{1}{3}$ perches. |
| | 3 | $79\cdot2\cdot7\frac{1}{100}$ acres Scotch = 100 English. By 11200 sq. ft. |
| 123 | 1 | 41 acres, 3 roods, 9 perches. |
| | 2 | 61 acres, 2 roods, $36\frac{1}{3}$ perches. |
| | 3 | 1944 English acres. $7\frac{1}{6}$ miles. |
| | 4 | 65 acres, 0 roods, 35 perches. |
| 124 | 1 | 280 sq. gads + $72\frac{1}{4}$ sq. feet, or 0 acres, 2 roods, 23 perches statute. |
| | 2 | 56162711. |
| 146 | 1 | DE, 13 chains 17 links; \angle DEF, $103^{\circ} 48' 20''$. |
| 148 | 1 | 8 miles 3 furlongs and a half. |
| 149 | 1 | 17 chains 62 links. |
| 150 | 1 | 249 feet. |
| 151 | 1 | 188 feet high; 90 feet wide. |
| 152 | 1 | Difference of height, 202 feet. |
| 153 | 1 | In the question, for $14^{\circ} 15'$ read $14^{\circ} 45'$. 249 feet. |
| 181 | 2 | $42^{\circ} 3' 25''$. |
| 216 | 1 | New Road should bear S. $24^{\circ} 38'$ W., 60 chains 82 links. |
| 218 | 2 | 5 chains 42 links, and 6 chains 79 links. |
| 227 | 1 | 12 acres, 3 roods, 9 perches. |
| | 2 | 22 acres, 3 roods, 13 perches. |
| | 2 | 7·04 chains. |
| 230 | 2 | { A, 117 acres, and 12·72 chains frontage. { B, 67 acres, and 7·28 chains frontage. |
| 257 | 1 | $33\frac{1}{2}, 45\frac{1}{2}, 64\frac{1}{2}$ feet. |
| | 2 | 581 $\frac{1}{4}$ feet. |

ERRATA.

In page 40 for $\frac{1}{\cos. A}$ read $\frac{1}{-\cos. A}$
for sec. A read —sec. A.

165 line 24, for cosec. DG read cosec. d. DG.
205 line 3, for them read these
for is read as.
..

A P P E N D I X.

TABLES

OF

L O G A R I T H M S,

OF

S I N E S, C O S I N E S, A N D T A N G E N T S.

A N D

, A T R A V E R S E T A B L E,

S I X P L A C E S O F D E C I M^U A L S.

TABLE OF LOGARITHMS.

THIS table extends to six places of logarithms, for all numbers from 1 to 10,000, which is amply sufficient for all ordinary practical purposes.

In looking for the logarithm of any number, it must be remembered, that,—as in natural numbers, the *position* of the decimal point alone determines the value of the numbers,—so, in logarithms, that value is represented by the number of digits in the index, *prefixed* to the logarithm, which is the logarithmic decimal point or characteristic.

Logarithms of natural numbers, therefore, are the same, whether those numbers be decimal or not; so that to find the logarithm of any given number, you must first find the logarithm of the figures independently of their integral value, and then prefix the proper Index, to represent the position of the decimal point.

To find the Logarithms.

Look for the first three significant figures in the columns of numbers, on the left of the page; then follow, *laterally*, through the columns of logarithms, till you come to the one, which is headed by the next figure in the given number. The logarithm thus found, which is the logarithm, *collateral*, with the first three significant figures, and, *vertically*, under the next figure, will be the required logarithm of the given number.

EXAMPLE to find the logarithm of 4285. Look down the column of numbers for 428, then laterally to the column headed 5; then collaterally with 428, and, vertically, under 5, you will find the required logarithm 631951, which is the logarithm, whether the number be 4285 or 42.85 or .0004285.

For the index, which represents the position of the decimal point, see chapter on logarithms, in the Introduction.

The method of finding the logarithms of numbers, *greater* than 10,000, or having more than four significant figures, will be found also fully explained there.

LOGARITHM, OF NUMBERS

To find the number, corresponding to a given Logarithm.

Look in the columns of logarithms, for the nearest logarithm to the given one (omitting the index); then, looking, collaterally, to the column of numbers for the first three figures, and vertically, to the number on the top of the column for the fourth, you will obtain the corresponding number to four places of figures, which will be whole numbers, or decimals, according to the index of the logarithm.

For determining the position of the decimal point, as represented by this index, or for carrying out the number to more places of figures, see the chapter on the subject in the Introduction.

EXAMPLE—What is the natural number corresponding to the given logarithm 1.252130?

Looking in the columns of logarithms, you will find 252125 is the nearest logarithm; collaterally, in the column of numbers you will find 178, the first three figures; and vertically, at the head of the column 7, the fourth figure, which will give the required number 1787; having its integral value regulated by the given index 1, which will make 1787, all decimals, viz., .1787.

LOGARITHMS OF NUMBERS FROM 1 TO 10,000.

| No. | Log. |
|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|
| 1 | 000000 | 21 | 322219 | 41 | 612784 | 61 | 785330 | 81 | 908185 |
| 2 | 301030 | 22 | 342423 | 42 | 623249 | 62 | 792392 | 82 | 913814 |
| 3 | 477121 | 23 | 361728 | 43 | 633468 | 63 | 799341 | 83 | 919078 |
| 4 | 602060 | 24 | 380211 | 44 | 643453 | 64 | 806180 | 84 | 924279 |
| 5 | 698970 | 25 | 397940 | 45 | 653213 | 65 | 812913 | 85 | 929419 |
| 6 | 778151 | 26 | 414973 | 46 | 662758 | 66 | 819344 | 86 | 934496 |
| 7 | 8450 8 | 27 | 431364 | 47 | 672098 | 67 | 826075 | 87 | 939519 |
| 8 | 903090 | 28 | 447158 | 48 | 681241 | 68 | 832509 | 88 | 944483 |
| 9 | 934243 | 29 | 462398 | 49 | 690196 | 69 | 838849 | 89 | 949390 |
| 10 | 000000 | 30 | 477121 | 50 | 698970 | 70 | 845098 | 90 | 954243 |
| 11 | 041393 | 31 | 491362 | 51 | 707570 | 71 | 851258 | 91 | 959041 |
| 12 | 079181 | 32 | 505150 | 52 | 716003 | 72 | 857383 | 92 | 963788 |
| 13 | 113943 | 33 | 518514 | 53 | 724276 | 73 | 863323 | 93 | 968483 |
| 14 | 146128 | 34 | 531479 | 54 | 732394 | 74 | 869292 | 94 | 973128 |
| 15 | 176091 | 35 | 544068 | 55 | 740363 | 75 | 875061 | 95 | 977724 |
| 16 | 204120 | 36 | 556303 | 56 | 748188 | 76 | 880814 | 96 | 982271 |
| 17 | 230449 | 37 | 568202 | 57 | 755875 | 77 | 886491 | 97 | 986772 |
| 18 | 255273 | 38 | 579784 | 58 | 763428 | 78 | 892095 | 98 | 991226 |
| 19 | 278754 | 39 | 591065 | 59 | 770852 | 79 | 897697 | 99 | 995685 |
| 20 | 301030 | 40 | 602060 | 60 | 778151 | 80 | 903090 | 100 | 000000 |

| No. | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
|-----|--------|--------|--------|--------|--------|---------|--------|--------|--------|--------|
| 100 | 000000 | 000434 | 000818 | 001301 | 001734 | 002166 | 002598 | 003029 | 003461 | 003891 |
| 101 | 004821 | 004751 | 005181 | 005609 | 006038 | 006468 | 006894 | 007321 | 007748 | 008174 |
| 102 | 008600 | 009026 | 009451 | 009876 | 010300 | 010724 | 011147 | 011570 | 011993 | 012415 |
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| 836 | 922206 | 922258 | 922310 | 922362 | 922414 | 922466 | 922518 | 922570 | 922622 | 922674 |
| 837 | 922725 | 922777 | 922829 | 922881 | 922933 | 922985 | 923037 | 923089 | 923140 | 923192 |
| 838 | 923244 | 923296 | 923348 | 923399 | 923451 | 923503 | 923555 | 923607 | 923653 | 923710 |
| 839 | 923762 | 923814 | 923865 | 923917 | 923969 | 924021 | 924072 | 924124 | 924176 | 924228 |
| 840 | 924279 | 924331 | 924383 | 924434 | 924486 | 924538 | 924589 | 924641 | 924693 | 924744 |
| 841 | 924796 | 924848 | 924899 | 924951 | 925003 | 925054 | 925106 | 925157 | 925209 | 925261 |
| 842 | 925312 | 925364 | 925415 | 925467 | 925518 | 925570 | 925621 | 925673 | 925725 | 925776 |
| 843 | 925828 | 925879 | 925931 | 925982 | 926034 | 926085 | 926137 | 926188 | 926240 | 926291 |
| 844 | 926342 | 926394 | 926445 | 926497 | 926548 | 926600 | 926651 | 926702 | 926754 | 926805 |
| 845 | 926857 | 926908 | 926959 | 927011 | 927062 | 927114 | 927165 | 927216 | 927268 | 927319 |
| 846 | 927370 | 927422 | 927473 | 927524 | 927576 | 927627 | 927678 | 927730 | 927781 | 927832 |
| 847 | 927883 | 927935 | 927986 | 928037 | 928088 | 928140 | 928191 | 928242 | 928293 | 928345 |
| 848 | 928896 | 928447 | 928498 | 928549 | 928601 | 928652 | 928703 | 928754 | 928803 | 928857 |
| 849 | 928908 | 928959 | 929010 | 929061 | 929112 | 929163 | 929215 | 929266 | 929317 | 929368 |

| No. | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
|-----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 850 | 929419 | 929470 | 929521 | 929572 | 929623 | 929674 | 929725 | 929776 | 929827 | 929879 |
| 851 | 929930 | 929981 | 930032 | 930083 | 930134 | 930185 | 930236 | 930287 | 930338 | 930389 |
| 852 | 930440 | 930491 | 930542 | 930592 | 930643 | 930694 | 930745 | 930796 | 930847 | 930898 |
| 853 | 930949 | 931000 | 931051 | 931102 | 931153 | 931204 | 931254 | 931305 | 931356 | 931407 |
| 854 | 931458 | 931509 | 931560 | 931610 | 931661 | 931712 | 931763 | 931814 | 931865 | 931915 |
| 855 | 931965 | 932017 | 932068 | 932118 | 932169 | 932220 | 932271 | 932322 | 932372 | 932423 |
| 856 | 932474 | 932524 | 932575 | 932626 | 932677 | 932727 | 932778 | 932829 | 932879 | 932930 |
| 857 | 932981 | 933031 | 933082 | 933133 | 933183 | 933234 | 933285 | 933335 | 933386 | 933437 |
| 858 | 933487 | 933538 | 933589 | 933639 | 933690 | 933740 | 933791 | 933841 | 933892 | 933913 |
| 859 | 933993 | 934044 | 934094 | 934145 | 934195 | 934246 | 934296 | 931347 | 934394 | 934448 |
| 860 | 934498 | 934549 | 934599 | 934650 | 934700 | 934751 | 934801 | 934852 | 934902 | 934953 |
| 861 | 935003 | 935054 | 935104 | 935154 | 935205 | 935255 | 935306 | 935356 | 935406 | 935457 |
| 862 | 935507 | 935558 | 935608 | 935658 | 935709 | 935759 | 935809 | 935860 | 935910 | 935960 |
| 863 | 936011 | 936061 | 936111 | 936162 | 936212 | 936262 | 936313 | 936363 | 936413 | 936463 |
| 864 | 936514 | 936564 | 936614 | 936663 | 936715 | 936765 | 936815 | 936865 | 936916 | 936966 |
| 865 | 937016 | 937066 | 937117 | 937167 | 937217 | 937267 | 937317 | 937367 | 937418 | 937468 |
| 866 | 937518 | 937568 | 937618 | 937668 | 937718 | 937769 | 937819 | 937869 | 937919 | 937969 |
| 867 | 938019 | 938069 | 938119 | 938169 | 938219 | 938269 | 938320 | 938370 | 938420 | 938470 |
| 868 | 938520 | 938570 | 938620 | 938670 | 938720 | 938770 | 938820 | 938870 | 938919 | 938970 |
| 869 | 939020 | 939070 | 939120 | 939170 | 939220 | 939270 | 939320 | 939369 | 939419 | 939469 |
| 870 | 939519 | 939569 | 939619 | 939669 | 939719 | 939769 | 939819 | 939869 | 939918 | 939968 |
| 871 | 940018 | 940068 | 940118 | 940168 | 940218 | 940267 | 940317 | 940367 | 940417 | 940467 |
| 872 | 949516 | 940506 | 940616 | 940666 | 940716 | 940765 | 940815 | 940865 | 940915 | 940961 |
| 873 | 941014 | 941061 | 941114 | 941163 | 941213 | 941263 | 941313 | 941362 | 941412 | 941462 |
| 874 | 941511 | 941561 | 941611 | 941660 | 941710 | 941760 | 941809 | 941859 | 941909 | 941958 |
| 875 | 942008 | 942058 | 942107 | 942157 | 942207 | 942256 | 942306 | 942355 | 942403 | 942455 |
| 876 | 942504 | 942554 | 942603 | 942653 | 942702 | 942752 | 942801 | 942851 | 942901 | 942950 |
| 877 | 943000 | 943049 | 943099 | 943148 | 943198 | 943247 | 943297 | 943346 | 943393 | 943445 |
| 878 | 943495 | 943514 | 943593 | 943643 | 943692 | 943742 | 943791 | 943841 | 943890 | 943939 |
| 879 | 943989 | 944038 | 944088 | 944137 | 944186 | 944236 | 944285 | 944335 | 944384 | 944433 |
| 880 | 944183 | 944532 | 944581 | 944631 | 944680 | 944729 | 944779 | 944828 | 944877 | 944927 |
| 881 | 944976 | 945025 | 945074 | 945124 | 945173 | 945222 | 945272 | 945321 | 945370 | 945419 |
| 882 | 945469 | 945518 | 945567 | 945616 | 945665 | 945715 | 945764 | 945813 | 945862 | 945912 |
| 883 | 945961 | 946010 | 946059 | 946108 | 946157 | 946207 | 946256 | 946305 | 946354 | 946403 |
| 884 | 946452 | 946501 | 946551 | 946600 | 946649 | 946698 | 946747 | 946796 | 946845 | 946894 |
| 885 | 946943 | 946992 | 947041 | 947090 | 947140 | 947189 | 947238 | 947287 | 947336 | 947385 |
| 886 | 947134 | 947183 | 947232 | 947281 | 947330 | 947379 | 947728 | 947777 | 947826 | 947875 |
| 887 | 947924 | 947973 | 948022 | 948070 | 948119 | 948168 | 948217 | 948266 | 948315 | 948364 |
| 888 | 948413 | 948462 | 948511 | 948560 | 948609 | 948657 | 948706 | 948755 | 948804 | 948853 |
| 889 | 948902 | 948951 | 948999 | 949018 | 949097 | 949146 | 949193 | 949244 | 949292 | 949341 |
| 890 | 949390 | 949439 | 949488 | 949536 | 949585 | 949634 | 949683 | 949731 | 949780 | 949829 |
| 891 | 949878 | 949926 | 949975 | 950024 | 950073 | 950121 | 950170 | 950219 | 950267 | 950316 |
| 892 | 950365 | 950414 | 950462 | 950511 | 950560 | 950608 | 950657 | 950706 | 950754 | 950803 |
| 893 | 950851 | 950900 | 950949 | 950997 | 951046 | 951095 | 951143 | 951192 | 951240 | 951289 |
| 894 | 951338 | 951386 | 951435 | 951483 | 951532 | 951580 | 951629 | 951677 | 951726 | 951775 |
| 895 | 951823 | 951872 | 951920 | 951969 | 952017 | 952066 | 952114 | 952163 | 952211 | 952260 |
| 896 | 952308 | 952356 | 952405 | 952453 | 952502 | 952550 | 952599 | 952647 | 952696 | 952744 |
| 897 | 952792 | 952841 | 952889 | 952938 | 952986 | 953034 | 953083 | 953131 | 953180 | 953228 |
| 898 | 953276 | 953325 | 953373 | 953421 | 953470 | 953518 | 953566 | 953615 | 953663 | 953711 |
| 899 | 953870 | 953808 | 953856 | 953905 | 953953 | 954001 | 954098 | 954146 | 954194 | |

| No. | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
|-----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 900 | 954243 | 954291 | 954339 | 954387 | 954435 | 954484 | 954532 | 954580 | 954628 | 954677 |
| 901 | 954725 | 954773 | 954821 | 954869 | 954918 | 954966 | 955014 | 955062 | 955110 | 955158 |
| 902 | 955207 | 955255 | 955303 | 955351 | 955399 | 955447 | 955495 | 955543 | 955592 | 955640 |
| 903 | 955688 | 955736 | 955784 | 955832 | 955880 | 955928 | 955976 | 956024 | 956072 | 956120 |
| 904 | 956168 | 956216 | 956265 | 956313 | 956361 | 956409 | 956457 | 956505 | 956553 | 956601 |
| 905 | 956649 | 956697 | 956745 | 956793 | 956840 | 956888 | 956936 | 956984 | 957032 | 957080 |
| 906 | 957128 | 957176 | 957224 | 957272 | 957320 | 957368 | 957416 | 957464 | 957512 | 957559 |
| 907 | 957607 | 957655 | 957703 | 957751 | 957799 | 957847 | 957894 | 957942 | 957990 | 958038 |
| 908 | 958086 | 958134 | 958181 | 958229 | 958277 | 958325 | 958373 | 958421 | 958468 | 958516 |
| 909 | 958364 | 958612 | 958659 | 958707 | 958755 | 958803 | 958850 | 958898 | 958946 | 958994 |
| 910 | 959041 | 959089 | 959137 | 959185 | 959232 | 959280 | 959328 | 959375 | 959423 | 959471 |
| 911 | 959518 | 959566 | 959614 | 959661 | 959709 | 959757 | 959804 | 959852 | 959900 | 959947 |
| 912 | 960995 | 960042 | 960090 | 960138 | 960185 | 960233 | 960280 | 960328 | 960376 | 960423 |
| 913 | 960471 | 960518 | 960566 | 960613 | 960661 | 960709 | 960756 | 960804 | 960851 | 960899 |
| 914 | 960946 | 960994 | 961041 | 961089 | 961136 | 961184 | 961231 | 961279 | 961326 | 961374 |
| 915 | 961421 | 961469 | 961516 | 961563 | 961611 | 961658 | 961706 | 961753 | 961801 | 961848 |
| 916 | 961895 | 961943 | 961990 | 962038 | 962085 | 962132 | 962180 | 962227 | 962275 | 962322 |
| 917 | 962369 | 962417 | 962464 | 962511 | 962559 | 962606 | 962653 | 962701 | 962748 | 962795 |
| 918 | 962813 | 962860 | 962937 | 962985 | 963032 | 963079 | 963126 | 963174 | 963221 | 963268 |
| 919 | 963316 | 963363 | 963410 | 963457 | 963504 | 963552 | 963599 | 963646 | 963693 | 963741 |
| 920 | 963788 | 963835 | 963882 | 963929 | 963977 | 964024 | 964071 | 964118 | 964165 | 964212 |
| 921 | 964260 | 964307 | 964354 | 964401 | 964448 | 964495 | 964542 | 964590 | 964637 | 964684 |
| 922 | 964731 | 964778 | 964825 | 964872 | 964919 | 964966 | 965013 | 965061 | 965108 | 965155 |
| 923 | 965202 | 965249 | 965296 | 965343 | 965390 | 965437 | 965481 | 965531 | 965578 | 965625 |
| 924 | 965672 | 965719 | 965766 | 965813 | 965860 | 965907 | 965954 | 966001 | 966048 | 966095 |
| 925 | 966142 | 966189 | 966236 | 966283 | 966329 | 966376 | 966423 | 966470 | 966517 | 966564 |
| 926 | 966611 | 966658 | 966705 | 966752 | 966799 | 966845 | 966892 | 966939 | 966986 | 967033 |
| 927 | 967080 | 967127 | 967173 | 967220 | 967267 | 967314 | 967361 | 967408 | 967454 | 967501 |
| 928 | 967548 | 967595 | 967642 | 967688 | 967735 | 967782 | 967829 | 967875 | 967922 | 967969 |
| 929 | 968016 | 968062 | 968109 | 968156 | 968203 | 968249 | 968296 | 968343 | 968390 | 968436 |
| 930 | 968483 | 968530 | 968576 | 968623 | 968670 | 968716 | 968763 | 968810 | 968856 | 968903 |
| 931 | 968950 | 968996 | 969043 | 969090 | 969136 | 969183 | 969229 | 969276 | 969323 | 969369 |
| 932 | 969416 | 969463 | 969509 | 969556 | 969602 | 969649 | 969695 | 969742 | 969789 | 969835 |
| 933 | 969882 | 969928 | 969975 | 970021 | 970068 | 970114 | 970161 | 970207 | 970254 | 970300 |
| 934 | 970347 | 970393 | 970440 | 970486 | 970533 | 970579 | 970626 | 970672 | 970719 | 970765 |
| 935 | 970812 | 970858 | 970904 | 970951 | 970997 | 971044 | 971090 | 971137 | 971183 | 971229 |
| 936 | 971276 | 971322 | 971369 | 971415 | 971461 | 971508 | 971554 | 971601 | 971647 | 971693 |
| 937 | 971740 | 971786 | 971832 | 971879 | 971925 | 971971 | 972018 | 972064 | 972110 | 972157 |
| 938 | 972203 | 972249 | 972295 | 972342 | 972388 | 972434 | 972481 | 972527 | 972573 | 972619 |
| 939 | 972666 | 972712 | 972758 | 972804 | 972851 | 972897 | 972943 | 972989 | 973035 | 973082 |
| 940 | 973128 | 973174 | 973220 | 973266 | 973313 | 973359 | 973405 | 973451 | 973497 | 973548 |
| 941 | 973590 | 973636 | 973682 | 973728 | 973774 | 973820 | 973866 | 973913 | 973959 | 974005 |
| 942 | 974051 | 974097 | 974143 | 974189 | 974235 | 974281 | 974327 | 974374 | 974420 | 974466 |
| 943 | 974512 | 974558 | 974604 | 974650 | 974696 | 974742 | 974788 | 974834 | 974880 | 974926 |
| 944 | 974972 | 975018 | 975064 | 975110 | 975156 | 975202 | 975248 | 975294 | 975340 | 975386 |
| 945 | 975432 | 975478 | 975524 | 975570 | 975616 | 975662 | 975707 | 975753 | 975799 | 975845 |
| 946 | 975891 | 975937 | 975983 | 976029 | 976075 | 976121 | 976167 | 976212 | 976258 | 976304 |
| 947 | 976350 | 976396 | 976442 | 976488 | 976533 | 976579 | 976625 | 976671 | 976717 | 976763 |
| 948 | 976808 | 976854 | 976900 | 976946 | 976992 | 977037 | 977083 | 977129 | 977175 | 977220 |
| 949 | 977266 | 977312 | 977358 | 977403 | 977449 | 977495 | 977541 | 977586 | 977632 | 977678 |

| No. | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
|-----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 950 | 977724 | 977769 | 977815 | 977861 | 977906 | 977952 | 977998 | 978043 | 978089 | 978135 |
| 951 | 978181 | 978226 | 978272 | 978317 | 978363 | 978409 | 978454 | 978500 | 978546 | 978591 |
| 952 | 978637 | 978683 | 978728 | 978774 | 978819 | 978865 | 978911 | 978956 | 979022 | 979047 |
| 953 | 979093 | 979138 | 979184 | 979230 | 979275 | 979321 | 979366 | 979412 | 979457 | 979503 |
| 954 | 979548 | 979594 | 979639 | 979685 | 979730 | 979776 | 979821 | 979867 | 979912 | 979958 |
| 955 | 980003 | 980049 | 980094 | 980140 | 980185 | 980231 | 980276 | 980322 | 980367 | 980412 |
| 956 | 980458 | 980503 | 980549 | 980594 | 980640 | 980685 | 980730 | 980776 | 980821 | 980867 |
| 957 | 980912 | 980957 | 981003 | 981048 | 981093 | 981139 | 981184 | 981229 | 981275 | 981320 |
| 958 | 981366 | 981411 | 981456 | 981501 | 981547 | 981592 | 981637 | 981683 | 981728 | 981773 |
| 959 | 981819 | 981864 | 981909 | 981954 | 982000 | 982045 | 982090 | 982135 | 982181 | 982226 |
| 960 | 982271 | 982316 | 982362 | 982407 | 982452 | 982497 | 982543 | 982588 | 982633 | 982678 |
| 961 | 982723 | 982769 | 982814 | 982859 | 982904 | 982949 | 982994 | 983040 | 983085 | 983130 |
| 962 | 983175 | 983220 | 983265 | 983310 | 983356 | 983401 | 983446 | 983491 | 983536 | 983581 |
| 963 | 983626 | 983671 | 983716 | 983762 | 983807 | 983852 | 983897 | 983942 | 983987 | 984032 |
| 964 | 984077 | 984122 | 984167 | 984212 | 984257 | 984302 | 984347 | 984392 | 984437 | 984482 |
| 965 | 984527 | 984572 | 984617 | 984662 | 984707 | 984752 | 984797 | 984842 | 984887 | 984932 |
| 966 | 984977 | 985022 | 985067 | 985112 | 985157 | 985202 | 985247 | 985292 | 985337 | 985382 |
| 967 | 985426 | 985471 | 985516 | 985561 | 985606 | 985651 | 985696 | 985741 | 985786 | 985830 |
| 968 | 985875 | 985920 | 985965 | 986010 | 986055 | 986100 | 986144 | 986189 | 986234 | 986279 |
| 969 | 986324 | 986369 | 986413 | 986458 | 986503 | 986548 | 986593 | 986637 | 986682 | 986727 |
| 970 | 986772 | 986817 | 986861 | 986906 | 986951 | 986996 | 987040 | 987085 | 987130 | 987175 |
| 971 | 987219 | 987264 | 987309 | 987353 | 987398 | 987443 | 987488 | 987532 | 987577 | 987622 |
| 972 | 987666 | 987711 | 987756 | 987800 | 987845 | 987890 | 987934 | 987979 | 988024 | 988068 |
| 973 | 988113 | 988157 | 988202 | 988247 | 988291 | 988336 | 988381 | 988425 | 988470 | 988514 |
| 974 | 988559 | 988604 | 988648 | 988693 | 988737 | 988782 | 988826 | 988871 | 988916 | 988960 |
| 975 | 989005 | 989049 | 989094 | 989138 | 989183 | 989227 | 989272 | 989316 | 989361 | 989405 |
| 976 | 989450 | 989494 | 989539 | 989583 | 989628 | 989672 | 989717 | 989761 | 989806 | 989850 |
| 977 | 989895 | 989939 | 989983 | 990028 | 990072 | 990117 | 990161 | 990206 | 990250 | 990294 |
| 978 | 990339 | 990383 | 990428 | 990472 | 990516 | 990561 | 990605 | 990650 | 990694 | 990738 |
| 979 | 990783 | 990827 | 990871 | 990916 | 990960 | 991004 | 991049 | 991093 | 991137 | 991182 |
| 980 | 991226 | 991270 | 991315 | 991359 | 991403 | 991448 | 991492 | 991536 | 991580 | 991625 |
| 981 | 991669 | 991713 | 991758 | 991802 | 991846 | 991890 | 991935 | 991979 | 992023 | 992067 |
| 982 | 992111 | 992156 | 992200 | 992244 | 992288 | 992333 | 992377 | 992421 | 992465 | 992509 |
| 983 | 992554 | 992598 | 992642 | 992686 | 992730 | 992774 | 992819 | 992863 | 992907 | 992951 |
| 984 | 992995 | 993039 | 993083 | 993127 | 993172 | 993216 | 993260 | 993304 | 993348 | 993392 |
| 985 | 993436 | 993480 | 993524 | 993568 | 993613 | 993657 | 993701 | 993745 | 993789 | 993833 |
| 986 | 993877 | 993921 | 993965 | 994009 | 994053 | 994097 | 994141 | 994185 | 994229 | 994273 |
| 987 | 994317 | 994361 | 994405 | 994449 | 994493 | 994537 | 994581 | 994625 | 994669 | 994713 |
| 988 | 994757 | 994801 | 994845 | 994889 | 994933 | 994977 | 995021 | 995065 | 995108 | 995152 |
| 989 | 995196 | 995240 | 995284 | 995328 | 995372 | 995416 | 995460 | 995504 | 995547 | 995591 |
| 990 | 995635 | 995679 | 995723 | 995767 | 995811 | 995854 | 995898 | 995942 | 995986 | 996030 |
| 991 | 996074 | 996117 | 996161 | 996205 | 996249 | 996293 | 996337 | 996380 | 996424 | 996468 |
| 992 | 996512 | 996555 | 996599 | 996643 | 996687 | 996731 | 996774 | 996818 | 996862 | 996906 |
| 993 | 996949 | 996993 | 997037 | 997080 | 997124 | 997168 | 997212 | 997255 | 997299 | 997343 |
| 994 | 997386 | 997430 | 997474 | 997517 | 997561 | 997605 | 997648 | 997692 | 997736 | 997779 |
| 995 | 997823 | 997867 | 997910 | 997954 | 997998 | 998041 | 998085 | 998129 | 998172 | 998216 |
| 996 | 998259 | 998303 | 998347 | 998390 | 998434 | 998477 | 998521 | 998564 | 998608 | 998652 |
| 997 | 998695 | 998739 | 998782 | 998826 | 998869 | 998913 | 998956 | 999000 | 999043 | 999087 |
| 998 | 999181 | 999174 | 999218 | 999261 | 999305 | 999348 | 999392 | 999435 | 999474 | 999522 |
| 999 | 999565 | 999609 | 999652 | 999696 | 999739 | 999783 | 999826 | 999870 | 999918 | 999957 |

TABLE II.

LOGARITHMIC SINES, COSINES, TANGENTS, AND COTANGENTS,

*Calculated to the natural radius of 10,000 millions,
or the logarithmic radius of 10 index.*

THE secant and cosecant will be at once obtained from the tables, by the following formula, which will be found in the chapter on Trigonometry, page 38.

$$\sec. = \frac{\text{rad.}^2}{\cos.} \text{ therefore, log. sec.} = 20 - \log. \cos.$$

$$\text{cosec.} = \frac{\text{rad.}^2}{\sin.} \text{ log. cosec.} = 20 - \log. \text{sine.}$$

PROBLEM I.

To find the sine, tangent, &c., of a given arc.

If the degrees in the arc be *less than 45 degrees*, look at the top of the page for the degrees, reading downwards in the *left hand* column for the minutes, then, collaterally with the minutes, in the column headed sines, tangents, &c., at the *top* of the page, will be found the logarithmic sine or tangent required.

If the degrees be *more than 45°* look at the bottom of the page for the degrees, reading upwards, in the right hand column, for the minutes; then, collaterally, in the column, which has the proper heading, at the *bottom* of the page, will be found the logarithmic sine, or tangent required.

EXAMPLE 1.—It is required to find the sine of the arc of 29 degrees 45 minutes.

This arc being less than 45 degrees, look for 29 degrees at the top of the page, then reading downwards in the left hand, find 45 minutes; collaterally with this, in the column of sines, will be found 9.695671, the logarithmic sine required.

EXAMPLE 2.—It is required to find the tangent of the arc of 54 degrees 30 minutes.

This being above 45 degrees, look for 54 degrees, in the bottom of the page, and for 30 minutes, in the right hand column; then, in the column, headed tangents, at the *bottom* of the page, will be found, collaterally with the 30 minutes, the required tangent 10·146732.

When the given angle is in seconds, look out in the tables for the logarithmic sines, tangents, &c., of the next lower and next higher minutes; then say,—

As the difference between the next lower and next greater arc, in seconds: the difference between their corresponding logarithmic sines or tangents: the excess, in seconds, of the given arc above the next lower in the tables: the logarithmic excess of the sine or tangent of the given angle, above that of the next lower angle in the tables.

Add this excess to the next lower, in the case of the sine, tangent, and secant, subtracting it for their complements, and you obtain the logarithmic sine, or tangent required.

Or, multiply the difference of the next lower and next higher logarithmic sines and tangents by the seconds in the given angle, and divide by 60, for the excess aforesaid.

EXAMPLE—What is the sine of the angle of $15^{\circ} 45' 30''$?

$$\text{log. sin. } 15\cdot46 = 9\cdot434122$$

$$\text{log. sin. } 15\cdot45 = \underline{9\cdot433675}$$

$$\text{diff. } 1' = \underline{\quad\quad\quad} 417$$

$$\text{As } 1' \text{ or } 60'' : 30'' : 447 : x$$

$$447 \times \frac{30}{60} = 223 = \text{excess}$$

$$\text{log. sin. } 15\cdot45 = 9\cdot433675$$

$$\text{excess} = \underline{\quad\quad\quad} 223$$

$$\text{log. sin. } 15^{\circ} 45' 30'' = 9\cdot433898$$

PROBLEM II.

To find the number of degrees and minutes of an arc, when the logarithmic sine or tangent is given.

If the given logarithmic sine or tangent be found, or nearly so, in the tables, observe, whether the column, in which it is found, is headed sine or tangent, at the bottom or the top of the page; if at the top, look there for the number of degrees, and downwards in the left

hand column, for the minutes; if, on the contrary, at the *bottom*, the degrees will be found there, and, reading upwards in the right hand column, will be found the number of minutes required, placed collaterally with the given logarithmic sine or tangent.

EXAMPLE—What are the degrees and minutes of an arc, whose sine is 9.937895?

This will be found in the second column of logarithms, being headed sines, at the *bottom* of the page, where will be found 60 degrees; then tracing, collaterally, to the right hand column of minutes, will be found 5, making the required angle 60 degrees 5 minutes.

When the given logarithmic sine is not to be found exactly in the tables, and *the calculations are carried out to seconds* say,—As the difference between the next lower and next higher logarithmic sine or tangent, : the difference between their corresponding angles in the tables, in seconds, :: the difference between the next lower logarithmic sine or tangent and the given one, : excess in seconds, of the required angle.

Add this, in the case of the sine, tangent, or secant, and *vice versa*, to the next lower angle in the table, and you obtain the required angle in seconds.

Or, multiply the logarithmic excess by 60 seconds, and divide by the logarithmic difference for the actual excess in seconds.

EXAMPLE 2.—How many degrees, minutes, and seconds, are there in the arc, whose tangent is 10.246305?

next higher tan. 10.246174 = log. $60^\circ 27'$. given tan. 10.246305
next lower tan. 10.216180 = log. $60^\circ 26'$ = 10.246180

diff. 294 $60''$ log. excess = 125

As 294 : $60''$: 125 : x = actual excess

$$x = \frac{125 \times 60}{294} = 25 \text{ seconds which added to}$$

the lower, makes the required angle = $60^\circ 26' 25''$.

| | 0° | | | | 1° | | | | |
|----|-----------|-----------|----------|-----------|-----------|----------|----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 10·000000 | 10·000000 | | | 8·2411855 | 9·999934 | 8·241921 | 11·758079 | 60 |
| 1 | 6·463726 | 10·000000 | 6·403726 | 13·536274 | 8·249033 | 9·999932 | 8·249102 | 11·750898 | 59 |
| 2 | 6·764756 | 10·000000 | 6·764756 | 13·235344 | 8·256094 | 9·999929 | 8·256165 | 11·743835 | 58 |
| 3 | 6·940847 | 10·000000 | 6·940847 | 13·059153 | 8·263042 | 9·999927 | 8·263115 | 11·736885 | 57 |
| 4 | 7·065786 | 10·000000 | 7·065786 | 12·034214 | 8·269881 | 9·999925 | 8·269956 | 11·730044 | 56 |
| 5 | 7·162696 | 10·000000 | 7·162696 | 12·837304 | 8·276614 | 9·999922 | 8·276691 | 11·723309 | 55 |
| 6 | 7·241877 | 9·999999 | 7·241878 | 12·758122 | 8·283243 | 9·999920 | 8·283323 | 11·716677 | 54 |
| 7 | 7·308824 | 9·999999 | 7·308825 | 12·691175 | 8·289773 | 9·999918 | 8·289856 | 11·710144 | 53 |
| 8 | 7·366816 | 9·999999 | 7·366817 | 12·633183 | 8·296207 | 9·999915 | 8·296292 | 11·703708 | 52 |
| 9 | 7·417968 | 9·999999 | 7·417970 | 12·582030 | 8·302546 | 9·999913 | 8·302634 | 11·697366 | 51 |
| 10 | 7·463726 | 9·999998 | 7·463727 | 12·536273 | 8·308794 | 9·999910 | 8·308884 | 11·691116 | 50 |
| 11 | 7·506118 | 9·999998 | 7·506120 | 12·494880 | 8·314954 | 9·999907 | 8·315046 | 11·684954 | 49 |
| 12 | 7·542906 | 9·999997 | 7·542909 | 12·457091 | 8·321027 | 9·999905 | 8·321122 | 11·678878 | 48 |
| 13 | 7·577668 | 9·999997 | 7·577672 | 12·422328 | 8·327016 | 9·999902 | 8·327114 | 11·672896 | 47 |
| 14 | 7·609853 | 9·999996 | 7·609857 | 12·390143 | 8·332924 | 9·999899 | 8·333025 | 11·669753 | 46 |
| 15 | 7·639816 | 9·999996 | 7·639820 | 12·360180 | 8·338573 | 9·999897 | 8·338856 | 11·661144 | 45 |
| 16 | 7·667845 | 9·999995 | 7·667849 | 12·332151 | 8·344504 | 9·999894 | 8·344610 | 11·655390 | 44 |
| 17 | 7·694173 | 9·999995 | 7·694179 | 12·306821 | 8·350181 | 9·999897 | 8·350289 | 11·649711 | 43 |
| 18 | 7·718997 | 9·999994 | 7·719003 | 12·280907 | 8·355783 | 9·999888 | 8·355895 | 11·644105 | 42 |
| 19 | 7·742478 | 9·999993 | 7·742484 | 12·257516 | 8·361315 | 9·999885 | 8·361430 | 11·638570 | 41 |
| 20 | 7·764754 | 9·999993 | 7·764761 | 12·235239 | 8·366777 | 9·999882 | 8·366895 | 11·633103 | 40 |
| 21 | 7·785943 | 9·999992 | 7·785951 | 12·214049 | 8·372171 | 9·999879 | 8·372292 | 11·627708 | 39 |
| 22 | 7·806146 | 9·999991 | 7·806155 | 12·193845 | 8·377409 | 9·999876 | 8·377622 | 11·622738 | 38 |
| 23 | 7·825451 | 9·999990 | 7·825460 | 12·174540 | 8·382762 | 9·999873 | 8·382889 | 11·617111 | 37 |
| 24 | 7·843934 | 9·999989 | 7·843944 | 12·156056 | 8·387962 | 9·999870 | 8·388692 | 11·611908 | 36 |
| 25 | 7·861662 | 9·999989 | 7·861674 | 12·138326 | 8·393101 | 9·999867 | 8·393234 | 11·606766 | 35 |
| 26 | 7·878695 | 9·999988 | 7·878708 | 12·121292 | 8·398179 | 9·999864 | 8·398315 | 11·601685 | 34 |
| 27 | 7·893045 | 9·999987 | 7·893059 | 12·104901 | 8·403199 | 9·999861 | 8·403338 | 11·590662 | 33 |
| 28 | 7·910879 | 9·999986 | 7·910894 | 12·089106 | 8·408161 | 9·999858 | 8·408304 | 11·591636 | 32 |
| 29 | 7·926119 | 9·999985 | 7·926134 | 12·073806 | 8·413063 | 9·999854 | 8·413213 | 11·586787 | 31 |
| 30 | 7·940842 | 9·999983 | 7·940858 | 12·050142 | 8·417919 | 9·999851 | 8·418006 | 11·581932 | 30 |
| 31 | 7·955682 | 9·999982 | 7·955100 | 12·044900 | 8·422717 | 9·999848 | 8·422869 | 11·577131 | 29 |
| 32 | 7·968870 | 9·999981 | 7·968889 | 12·031111 | 8·427402 | 9·999845 | 8·427618 | 11·572382 | 28 |
| 33 | 7·982233 | 9·999980 | 7·982253 | 12·017747 | 8·432156 | 9·999841 | 8·432315 | 11·567685 | 27 |
| 34 | 7·995198 | 9·999979 | 7·995219 | 12·004681 | 8·436800 | 9·999838 | 8·436962 | 11·563038 | 26 |
| 35 | 8·007787 | 9·999977 | 8·007809 | 11·992191 | 8·441394 | 9·999834 | 8·441560 | 11·558440 | 25 |
| 36 | 8·020021 | 9·999976 | 8·020044 | 11·979956 | 8·445941 | 9·999831 | 8·446110 | 11·553890 | 24 |
| 37 | 8·031919 | 9·999975 | 8·031945 | 11·968055 | 8·450440 | 9·999827 | 8·450613 | 11·549387 | 23 |
| 38 | 8·043501 | 9·999973 | 8·043527 | 11·956473 | 8·454893 | 9·999824 | 8·455070 | 11·544930 | 22 |
| 39 | 8·054781 | 9·999972 | 8·054809 | 11·945191 | 8·459301 | 9·999820 | 8·459481 | 11·540519 | 21 |
| 40 | 8·065776 | 9·999971 | 8·065806 | 11·934194 | 8·463605 | 9·999816 | 8·463449 | 11·536151 | 20 |
| 41 | 8·076500 | 9·999969 | 8·076531 | 11·923469 | 8·467965 | 9·999813 | 8·468172 | 11·531828 | 19 |
| 42 | 8·080965 | 9·999968 | 8·080697 | 11·913003 | 8·472263 | 9·999809 | 8·472454 | 11·527446 | 18 |
| 43 | 8·097183 | 9·999966 | 8·097217 | 11·902783 | 8·476494 | 9·999805 | 8·476693 | 11·523807 | 17 |
| 44 | 8·107167 | 9·999964 | 8·107203 | 11·893797 | 8·480693 | 9·999801 | 8·480892 | 11·519108 | 16 |
| 45 | 8·116926 | 9·999963 | 8·116903 | 11·886303 | 8·484848 | 9·999797 | 8·485050 | 11·514930 | 15 |
| 46 | 8·126471 | 9·999961 | 8·126510 | 11·873490 | 8·488663 | 9·999794 | 8·489170 | 11·510830 | 14 |
| 47 | 8·135810 | 9·999959 | 8·135851 | 11·864149 | 8·493040 | 9·999790 | 8·493250 | 11·506730 | 13 |
| 48 | 8·144953 | 9·999958 | 8·144996 | 11·855004 | 8·497078 | 9·999786 | 8·497293 | 11·502707 | 12 |
| 49 | 8·153907 | 9·999956 | 8·153952 | 11·846048 | 8·501080 | 9·999782 | 8·501298 | 11·498702 | 11 |
| 50 | 8·162681 | 9·999954 | 8·162727 | 11·837273 | 8·505045 | 9·999778 | 8·505267 | 11·494733 | 10 |
| 51 | 8·171290 | 9·999952 | 8·171328 | 11·828672 | 8·509874 | 9·999774 | 8·509200 | 11·490800 | 9 |
| 52 | 8·179213 | 9·999950 | 8·179763 | 11·820237 | 8·512667 | 9·999769 | 8·513098 | 11·486902 | 8 |
| 53 | 8·187966 | 9·999948 | 8·188036 | 11·812904 | 8·510726 | 9·999765 | 8·516961 | 11·483099 | 7 |
| 54 | 8·196103 | 9·999946 | 8·196156 | 11·803844 | 8·520551 | 9·999761 | 8·520790 | 11·479210 | 6 |
| 55 | 8·204070 | 9·999944 | 8·204126 | 11·795874 | 8·524143 | 9·999757 | 8·524586 | 11·475414 | 5 |
| 56 | 8·211995 | 9·999942 | 8·211053 | 11·788047 | 8·528102 | 9·999753 | 8·528349 | 11·471651 | 4 |
| 57 | 8·219582 | 9·999940 | 8·219641 | 11·780359 | 8·531828 | 9·999748 | 8·532080 | 11·467920 | 3 |
| 58 | 8·227134 | 9·999938 | 8·227195 | 11·772805 | 8·535528 | 9·999744 | 8·535779 | 11·464221 | 2 |
| 59 | 8·234557 | 9·999936 | 8·234621 | 11·765379 | 8·539186 | 9·999740 | 8·539447 | 11·460553 | 1 |
| 60 | 8·241855 | 9·999934 | 8·241921 | 11·758079 | 8·542819 | 9·999735 | 8·543084 | 11·456916 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | |

| | 2° | | | | 3° | | | | |
|----|-----------|----------|----------|-----------|----------|----------|----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan | |
| 0 | 8'542819 | 9'999735 | 8'543084 | 11'456916 | 8'718600 | 9'999404 | 8'719306 | 11'250604 | 60 |
| 1 | 8'546122 | 9'999731 | 8'550691 | 11'453309 | 8'721204 | 9'999398 | 8'721806 | 11'278194 | 59 |
| 2 | 8'549004 | 9'999726 | 8'550268 | 11'459732 | 8'724595 | 9'999391 | 8'724204 | 11'275796 | 58 |
| 3 | 8'553539 | 9'999722 | 8'553617 | 11'46183 | 8'725972 | 9'999384 | 8'726588 | 11'273412 | 57 |
| 4 | 8'557054 | 9'999717 | 8'557336 | 11'442664 | 8'728337 | 9'999378 | 8'728959 | 11'271041 | 56 |
| 5 | 8'560340 | 9'999713 | 8'560828 | 11'430179 | 8'730388 | 9'999371 | 8'731317 | 11'268685 | 55 |
| 6 | 8'563099 | 9'999708 | 8'564291 | 11'435709 | 8'733027 | 9'999364 | 8'733663 | 11'266337 | 54 |
| 7 | 8'567434 | 9'999704 | 8'567277 | 11'432273 | 8'735351 | 9'999357 | 8'735996 | 11'264004 | 53 |
| 8 | 8'570836 | 9'999699 | 8'571137 | 11'428463 | 8'737667 | 9'999350 | 8'738317 | 11'261183 | 52 |
| 9 | 8'574214 | 9'999694 | 8'574520 | 11'425480 | 8'739966 | 9'999343 | 8'740026 | 11'259374 | 51 |
| 10 | 8'577500 | 9'999689 | 8'577877 | 11'422123 | 8'742259 | 9'999336 | 8'742922 | 11'257078 | 50 |
| 11 | 8'580892 | 9'999685 | 8'581208 | 11'418792 | 8'741536 | 9'999329 | 8'743207 | 11'254793 | 49 |
| 12 | 8'584193 | 9'999680 | 8'584514 | 11'415486 | 8'740802 | 9'999322 | 8'747479 | 11'252521 | 48 |
| 13 | 8'587469 | 9'999675 | 8'587705 | 11'412205 | 8'749055 | 9'999315 | 8'749740 | 11'250260 | 47 |
| 14 | 8'590721 | 9'999670 | 8'591051 | 11'408949 | 8'751297 | 9'999308 | 8'751989 | 11'248011 | 46 |
| 15 | 8'593948 | 9'999665 | 8'594233 | 11'405717 | 8'753528 | 9'999301 | 8'754227 | 11'245773 | 45 |
| 16 | 8'597152 | 9'999660 | 8'597492 | 11'402508 | 8'755747 | 9'999294 | 8'756453 | 11'243547 | 44 |
| 17 | 8'600332 | 9'999655 | 8'600677 | 11'399323 | 8'757955 | 9'999287 | 8'758668 | 11'241332 | 43 |
| 18 | 8'603489 | 9'999650 | 8'603839 | 11'396161 | 8'760154 | 9'999279 | 8'760872 | 11'239128 | 42 |
| 19 | 8'606623 | 9'999645 | 8'606978 | 11'393022 | 8'762337 | 9'999272 | 8'763065 | 11'236935 | 41 |
| 20 | 8'609734 | 9'999640 | 8'610094 | 11'389099 | 8'764511 | 9'999265 | 8'765246 | 11'234754 | 40 |
| 21 | 8'612823 | 9'999635 | 8'613189 | 11'386811 | 8'766075 | 9'999257 | 8'767417 | 11'232583 | 39 |
| 22 | 8'615891 | 9'999629 | 8'616202 | 11'383738 | 8'768828 | 9'999250 | 8'769578 | 11'230422 | 38 |
| 23 | 8'618937 | 9'999624 | 8'619313 | 11'380687 | 8'770970 | 9'999242 | 8'771727 | 11'228273 | 37 |
| 24 | 8'621962 | 9'999619 | 8'622343 | 11'377637 | 8'773101 | 9'999235 | 8'773866 | 11'226131 | 36 |
| 25 | 8'624195 | 9'999614 | 8'625352 | 11'374648 | 8'775223 | 9'999227 | 8'775995 | 11'224005 | 35 |
| 26 | 8'627948 | 9'999608 | 8'628340 | 11'371660 | 8'777333 | 9'999220 | 8'778114 | 11'221886 | 34 |
| 27 | 8'630911 | 9'999603 | 8'631308 | 11'368602 | 8'779431 | 9'999212 | 8'780222 | 11'219775 | 33 |
| 28 | 8'633854 | 9'999597 | 8'634256 | 11'365741 | 8'781524 | 9'999205 | 8'782320 | 11'217680 | 32 |
| 29 | 8'636776 | 9'999592 | 8'637184 | 11'362816 | 8'783605 | 9'999197 | 8'784408 | 11'215592 | 31 |
| 30 | 8'639680 | 9'999586 | 8'640093 | 11'359907 | 8'785673 | 9'999189 | 8'786486 | 11'213574 | 30 |
| 31 | 8'642563 | 9'999581 | 8'642062 | 11'357018 | 8'787736 | 9'999181 | 8'788554 | 11'211446 | 29 |
| 32 | 8'645428 | 9'999575 | 8'645853 | 11'354147 | 8'789078 | 9'999174 | 8'790613 | 11'209387 | 28 |
| 33 | 8'648274 | 9'999570 | 8'648704 | 11'351296 | 8'791182 | 9'999166 | 8'792662 | 11'207338 | 27 |
| 34 | 8'651102 | 9'999564 | 8'651537 | 11'348163 | 8'793859 | 9'999158 | 8'794701 | 11'205299 | 26 |
| 35 | 8'653911 | 9'999556 | 8'654352 | 11'345648 | 8'79581 | 9'999150 | 8'796731 | 11'203269 | 25 |
| 36 | 8'656702 | 9'999553 | 8'657149 | 11'342851 | 8'797894 | 9'999142 | 8'798752 | 11'201248 | 24 |
| 37 | 8'659475 | 9'999517 | 8'659928 | 11'340072 | 8'799897 | 9'999134 | 8'800763 | 11'199237 | 23 |
| 38 | 8'662230 | 9'999511 | 8'662689 | 11'337311 | 8'801892 | 9'999126 | 8'802765 | 11'197235 | 22 |
| 39 | 8'664968 | 9'999535 | 8'665433 | 11'334567 | 8'803876 | 9'999118 | 8'804756 | 11'195242 | 21 |
| 40 | 8'667689 | 9'999529 | 8'668160 | 11'331840 | 8'805552 | 9'999110 | 8'806742 | 11'193258 | 20 |
| 41 | 8'670393 | 9'999524 | 8'670670 | 11'329130 | 8'807619 | 9'999102 | 8'808717 | 11'191283 | 19 |
| 42 | 8'673080 | 9'999518 | 8'673563 | 11'326437 | 8'809077 | 9'999094 | 8'810683 | 11'189317 | 18 |
| 43 | 8'673751 | 9'999512 | 8'672399 | 11'323761 | 8'811726 | 9'999086 | 8'812641 | 11'187359 | 17 |
| 44 | 8'678405 | 9'999506 | 8'678900 | 11'321100 | 8'813667 | 9'999077 | 8'814589 | 11'185411 | 16 |
| 45 | 8'681043 | 9'999500 | 8'681544 | 11'318456 | 8'815599 | 9'999069 | 8'816529 | 11'183471 | 15 |
| 46 | 8'683665 | 9'999493 | 8'684172 | 11'315628 | 8'817522 | 9'999061 | 8'818461 | 11'181539 | 14 |
| 47 | 8'6860272 | 9'999487 | 8'686784 | 11'312316 | 8'819436 | 9'999053 | 8'820384 | 11'179616 | 13 |
| 48 | 8'688863 | 9'999481 | 8'689381 | 11'310619 | 8'821343 | 9'999044 | 8'822298 | 11'177702 | 12 |
| 49 | 8'691438 | 9'999475 | 8'691963 | 11'308037 | 8'823240 | 9'999036 | 8'824205 | 11'175795 | 11 |
| 50 | 8'693908 | 9'999469 | 8'694529 | 11'305471 | 8'825130 | 9'999027 | 8'826103 | 11'173897 | 10 |
| 51 | 8'696543 | 9'999463 | 8'697081 | 11'302919 | 8'827011 | 9'999019 | 8'827992 | 11'172008 | 9 |
| 52 | 8'699073 | 9'999456 | 8'699617 | 11'300383 | 8'828884 | 9'999010 | 8'829874 | 11'170126 | 8 |
| 53 | 8'701589 | 9'999450 | 8'702139 | 11'297861 | 8'830749 | 9'999002 | 8'831745 | 11'168252 | 7 |
| 54 | 8'704090 | 9'999443 | 8'704646 | 11'295354 | 8'832607 | 9'999003 | 8'833613 | 11'166387 | 6 |
| 55 | 8'706577 | 9'999437 | 8'707140 | 11'292860 | 8'834456 | 9'999004 | 8'835471 | 11'164529 | 5 |
| 56 | 8'709049 | 9'999431 | 8'709618 | 11'290382 | 8'836207 | 9'999076 | 8'837321 | 11'162679 | 4 |
| 57 | 8'711507 | 9'999424 | 8'712083 | 11'287917 | 8'838130 | 9'999067 | 8'839163 | 11'160837 | 3 |
| 58 | 8'713952 | 9'999418 | 8'714334 | 11'285466 | 8'839056 | 9'999058 | 8'840998 | 11'159002 | 2 |
| 59 | 8'716083 | 9'999411 | 8'716072 | 11'283028 | 8'841774 | 9'999050 | 8'842425 | 11'157175 | 1 |
| 60 | 8'718680 | 9'999404 | 8'719306 | 11'260604 | 8'843585 | 9'999041 | 8'844644 | 11'155350 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | |

| | 4° | | | | 5° | | | | |
|----|----------|----------|----------|-----------|----------|----------|----------|-----------|----|
| / | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | / |
| 0 | 8'843565 | 9'998941 | 8'844644 | 11'155356 | 8'940296 | 9'998344 | 8'941952 | 11'058048 | 60 |
| 1 | 8'845387 | 9'998932 | 8'846455 | 11'153545 | 8'941738 | 9'998333 | 8'943104 | 11'050596 | 59 |
| 2 | 8'847183 | 9'996923 | 8'848260 | 11'151746 | 8'943174 | 9'996322 | 8'944652 | 11'053148 | 58 |
| 3 | 8'848971 | 9'996914 | 8'850057 | 11'149943 | 8'944606 | 9'998311 | 8'946295 | 11'053705 | 57 |
| 4 | 8'850751 | 9'996905 | 8'851846 | 11'148154 | 8'946034 | 9'996300 | 8'947734 | 11'052266 | 56 |
| 5 | 8'852525 | 9'996896 | 8'853628 | 11'146372 | 8'947456 | 9'998289 | 8'949168 | 11'050832 | 55 |
| 6 | 8'854291 | 9'996887 | 8'855403 | 11'144597 | 8'948874 | 9'998277 | 8'950597 | 11'049403 | 54 |
| 7 | 8'856049 | 9'996878 | 8'857171 | 11'142829 | 8'950287 | 9'998266 | 8'952021 | 11'047979 | 53 |
| 8 | 8'857801 | 9'996869 | 8'858932 | 11'141068 | 8'951696 | 9'998255 | 8'953441 | 11'046559 | 52 |
| 9 | 8'859546 | 9'996860 | 8'860666 | 11'139314 | 8'953100 | 9'998243 | 8'954856 | 11'045144 | 51 |
| 10 | 8'861283 | 9'998851 | 8'862433 | 11'137507 | 8'954199 | 9'998232 | 8'956267 | 11'043733 | 50 |
| 11 | 8'863014 | 9'998841 | 8'864173 | 11'135827 | 8'955394 | 9'998220 | 8'957651 | 11'042326 | 49 |
| 12 | 8'864734 | 9'998832 | 8'865906 | 11'131094 | 8'957281 | 9'998209 | 8'959075 | 11'040025 | 48 |
| 13 | 8'866415 | 9'998823 | 8'867632 | 11'123268 | 8'958670 | 9'998197 | 8'960473 | 11'039527 | 47 |
| 14 | 8'866165 | 9'998813 | 8'869351 | 11'130649 | 8'960052 | 9'998186 | 8'961866 | 11'038134 | 46 |
| 15 | 8'869868 | 9'998803 | 8'871064 | 11'129336 | 8'961429 | 9'998174 | 8'963255 | 11'036745 | 45 |
| 16 | 8'871565 | 9'998795 | 8'872770 | 11'127230 | 8'962801 | 9'998163 | 8'964639 | 11'035361 | 44 |
| 17 | 8'873255 | 9'998785 | 8'874469 | 11'125531 | 8'964170 | 9'998152 | 8'966019 | 11'033981 | 43 |
| 18 | 8'874938 | 9'998776 | 8'876162 | 11'123-38 | 8'965534 | 9'998139 | 8'967394 | 11'032606 | 42 |
| 19 | 8'876615 | 9'998766 | 8'877849 | 11'122151 | 8'966893 | 9'998128 | 8'968760 | 11'031234 | 41 |
| 20 | 8'878285 | 9'998757 | 8'879529 | 11'120171 | 8'968249 | 9'998116 | 8'970133 | 11'029807 | 40 |
| 21 | 8'879949 | 9'998747 | 8'881202 | 11'118798 | 8'969600 | 9'998104 | 8'971496 | 11'028504 | 39 |
| 22 | 8'881607 | 9'998738 | 8'882693 | 11'117131 | 8'970347 | 9'998092 | 8'972855 | 11'027145 | 38 |
| 23 | 8'883258 | 9'998728 | 8'884530 | 11'115470 | 8'972289 | 9'998080 | 8'974209 | 11'025791 | 37 |
| 24 | 8'884903 | 9'998718 | 8'886185 | 11'113815 | 8'973628 | 9'998068 | 8'975560 | 11'024440 | 36 |
| 25 | 8'886442 | 9'998708 | 8'887833 | 11'112167 | 8'974962 | 9'998056 | 8'976906 | 11'023094 | 35 |
| 26 | 8'888174 | 9'998699 | 8'889476 | 11'110524 | 8'976293 | 9'998044 | 8'978248 | 11'021752 | 34 |
| 27 | 8'889801 | 9'998689 | 8'891112 | 11'108888 | 8'977619 | 9'998032 | 8'979586 | 11'020414 | 33 |
| 28 | 8'891421 | 9'998679 | 8'892742 | 11'107258 | 8'978941 | 9'998020 | 8'980921 | 11'019079 | 32 |
| 29 | 8'893035 | 9'998669 | 8'894366 | 11'105634 | 8'980259 | 9'998008 | 8'982251 | 11'017749 | 31 |
| 30 | 8'894643 | 9'998659 | 8'895984 | 11'10416 | 8'981573 | 9'998996 | 8'983377 | 11'016423 | 30 |
| 31 | 8'896246 | 9'998649 | 8'897596 | 11'102404 | 8'982983 | 9'997394 | 8'984899 | 11'015101 | 29 |
| 32 | 8'897842 | 9'998639 | 8'899203 | 11'100797 | 8'984169 | 9'997972 | 8'986217 | 11'013783 | 28 |
| 33 | 8'899432 | 9'998629 | 8'900803 | 11'099197 | 8'985349 | 9'997959 | 8'987532 | 11'012468 | 27 |
| 34 | 8'901917 | 9'998619 | 8'902394 | 11'097602 | 8'986789 | 9'997947 | 8'988642 | 11'011158 | 26 |
| 35 | 8'902596 | 9'998609 | 8'903087 | 11'096013 | 8'988083 | 9'997935 | 8'990149 | 11'009851 | 25 |
| 36 | 8'904169 | 9'998599 | 8'905379 | 11'094430 | 8'989374 | 9'997922 | 8'991551 | 11'008549 | 24 |
| 37 | 8'905736 | 9'998589 | 8'907147 | 11'092833 | 8'990660 | 9'997910 | 8'992750 | 11'007250 | 23 |
| 38 | 8'907297 | 9'998578 | 8'908719 | 11'091281 | 8'991913 | 9'997897 | 8'994015 | 11'005955 | 22 |
| 39 | 8'908853 | 9'998568 | 8'910285 | 11'090715 | 8'993225 | 9'997885 | 8'995337 | 11'004663 | 21 |
| 40 | 8'910404 | 9'998558 | 8'911846 | 11'088154 | 8'994917 | 9'997872 | 8'996624 | 11'003376 | 20 |
| 41 | 8'911949 | 9'998548 | 8'913101 | 11'086399 | 8'995768 | 9'997860 | 8'997908 | 11'002092 | 19 |
| 42 | 8'913488 | 9'998537 | 8'914561 | 11'085049 | 8'997036 | 9'997847 | 8'999188 | 11'000812 | 18 |
| 43 | 8'915022 | 9'998527 | 8'916495 | 11'083505 | 8'998299 | 9'997837 | 9'000465 | 10'999535 | 17 |
| 44 | 8'916550 | 9'998516 | 8'918034 | 11'081966 | 8'999560 | 9'997522 | 9'001738 | 10'998262 | 16 |
| 45 | 8'918073 | 9'998506 | 8'919568 | 11'080422 | 9'000816 | 9'997409 | 9'003007 | 10'996903 | 15 |
| 46 | 8'919591 | 9'998495 | 8'921096 | 11'078904 | 9'002069 | 9'997797 | 9'004272 | 10'995728 | 14 |
| 47 | 8'921103 | 9'998485 | 8'922619 | 11'077381 | 9'003318 | 9'997784 | 9'005534 | 10'994466 | 13 |
| 48 | 8'922610 | 9'998474 | 8'924136 | 11'075864 | 9'004503 | 9'997771 | 9'006792 | 10'993208 | 12 |
| 49 | 8'924112 | 9'998464 | 8'925649 | 11'074351 | 9'005805 | 9'997758 | 9'008047 | 10'991953 | 11 |
| 50 | 8'925609 | 9'998453 | 8'927156 | 11'072844 | 9'007044 | 9'997745 | 9'009293 | 10'990702 | 10 |
| 51 | 8'927100 | 9'998442 | 8'928658 | 11'071342 | 9'008278 | 9'997732 | 9'010546 | 10'989454 | 9 |
| 52 | 8'928587 | 9'998431 | 8'930155 | 11'069815 | 9'009510 | 9'997719 | 9'011790 | 10'988210 | 8 |
| 53 | 8'930068 | 9'998421 | 8'931647 | 11'068553 | 9'010737 | 9'997706 | 9'013031 | 10'986969 | 7 |
| 54 | 8'931544 | 9'998410 | 8'933131 | 11'066866 | 9'011962 | 9'997693 | 9'014268 | 10'985732 | 6 |
| 55 | 8'933015 | 9'998399 | 8'934616 | 11'065384 | 9'013182 | 9'997680 | 9'015502 | 10'984498 | 5 |
| 56 | 8'934481 | 9'998389 | 8'936093 | 11'063307 | 9'014400 | 9'997667 | 9'016732 | 10'983268 | 4 |
| 57 | 8'935942 | 9'998377 | 8'937565 | 11'062135 | 9'015013 | 9'997654 | 9'017859 | 10'982041 | 3 |
| 58 | 8'937394 | 9'998366 | 8'939032 | 11'060468 | 9'016824 | 9'997641 | 9'019193 | 10'980817 | 2 |
| 59 | 8'938850 | 9'998355 | 8'940191 | 11'059506 | 9'018031 | 9'997628 | 9'020403 | 10'979597 | 1 |
| 60 | 8'940296 | 9'998344 | 8'941952 | 11'055848 | 9'019235 | 9'997614 | 9'021620 | 10'978880 | 0 |

| 6° | | | | 7° | | | | | |
|----|----------|----------|----------|-----------|----------|----------|-----------|------------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 9'019235 | 9'997614 | 9'021620 | 10'978380 | 9'085894 | 9'936751 | 9'089144 | 10'510856 | 60 |
| 1 | 9'020435 | 9'997601 | 9'022834 | 10'977166 | 9'086922 | 9'936735 | 9'089183 | 10'509813 | 59 |
| 2 | 9'021632 | 9'997588 | 9'024044 | 10'976956 | 9'087947 | 9'936720 | 9'0891228 | 10'508772 | 58 |
| 3 | 9'022823 | 9'997574 | 9'025251 | 10'974749 | 9'088970 | 9'936704 | 9'0892266 | 10'507734 | 57 |
| 4 | 9'024016 | 9'997561 | 9'026455 | 10'973545 | 9'089990 | 9'936686 | 9'093302 | 10'506638 | 56 |
| 5 | 9'025203 | 9'997547 | 9'027655 | 10'972345 | 9'091008 | 9'936673 | 9'094336 | 10'505664 | 55 |
| 6 | 9'026386 | 9'997534 | 9'028852 | 10'971146 | 9'092024 | 9'936657 | 9'145367 | 10'504633 | 54 |
| 7 | 9'027567 | 9'997520 | 9'030046 | 10'969954 | 9'093037 | 9'936641 | 9'093635 | 10'503605 | 53 |
| 8 | 9'028744 | 9'997507 | 9'031237 | 10'968763 | 9'094047 | 9'936625 | 9'097422 | 10'502578 | 52 |
| 9 | 9'029918 | 9'997493 | 9'032425 | 10'967575 | 9'095056 | 9'936610 | 9'098446 | 10'501554 | 51 |
| 10 | 9'031089 | 9'997480 | 9'033600 | 10'966391 | 9'096062 | 9'936594 | 9'999468 | 10'500532 | 50 |
| 11 | 9'032257 | 9'997466 | 9'034791 | 10'965209 | 9'097065 | 9'936578 | 9'090487 | 10'500513 | 49 |
| 12 | 9'033421 | 9'997452 | 9'035969 | 10'964031 | 9'098066 | 9'936562 | 9'091504 | 10'500496 | 48 |
| 13 | 9'034582 | 9'997439 | 9'037114 | 10'962856 | 9'099065 | 9'936546 | 9'102519 | 10'507481 | 47 |
| 14 | 9'035741 | 9'997425 | 9'038316 | 10'961684 | 9'100062 | 9'936530 | 9'103532 | 10'506468 | 46 |
| 15 | 9'036890 | 9'997411 | 9'039485 | 10'960515 | 9'101056 | 9'936514 | 9'104542 | 10'505458 | 45 |
| 16 | 9'038048 | 9'997397 | 9'040651 | 10'959349 | 9'102048 | 9'936498 | 9'105550 | 10'504450 | 44 |
| 17 | 9'039197 | 9'997383 | 9'041813 | 10'958187 | 9'103037 | 9'936482 | 9'106556 | 10'503444 | 43 |
| 18 | 9'040342 | 9'997369 | 9'042973 | 10'957087 | 9'104025 | 9'936465 | 9'107559 | 10'502441 | 42 |
| 19 | 9'041485 | 9'997355 | 9'044130 | 10'955870 | 9'105010 | 9'936449 | 9'108560 | 10'501440 | 41 |
| 20 | 9'042625 | 9'997341 | 9'045284 | 10'954716 | 9'105992 | 9'936433 | 9'109559 | 10'500441 | 40 |
| 21 | 9'043702 | 9'997327 | 9'046434 | 10'953566 | 9'106973 | 9'936417 | 9'110556 | 10'500444 | 39 |
| 22 | 9'044895 | 9'997313 | 9'047582 | 10'952418 | 9'107951 | 9'936400 | 9'111551 | 10'500449 | 38 |
| 23 | 9'046026 | 9'997299 | 9'048727 | 10'951273 | 9'108927 | 9'936384 | 9'112543 | 10'500457 | 37 |
| 24 | 9'047154 | 9'997285 | 9'049869 | 10'950131 | 9'109901 | 9'936368 | 9'113533 | 10'500467 | 36 |
| 25 | 9'048277 | 9'997271 | 9'051006 | 10'948992 | 9'110873 | 9'936351 | 9'114521 | 10'505479 | 35 |
| 26 | 9'049400 | 9'997257 | 9'052144 | 10'947856 | 9'111842 | 9'936335 | 9'115507 | 10'504493 | 34 |
| 27 | 9'050519 | 9'997242 | 9'053277 | 10'946723 | 9'112809 | 9'936318 | 9'116491 | 10'503509 | 33 |
| 28 | 9'051635 | 9'997228 | 9'054407 | 10'945593 | 9'113774 | 9'936302 | 9'117472 | 10'502528 | 32 |
| 29 | 9'052749 | 9'997214 | 9'055535 | 10'944465 | 9'114737 | 9'936285 | 9'118352 | 10'501548 | 31 |
| 30 | 9'053859 | 9'997199 | 9'056659 | 10'943341 | 9'115698 | 9'936269 | 9'119429 | 10'500571 | 30 |
| 31 | 9'054966 | 9'997185 | 9'057781 | 10'942219 | 9'116656 | 9'936252 | 9'120404 | 10'500576 | 29 |
| 32 | 9'056071 | 9'997170 | 9'058900 | 10'941100 | 9'117613 | 9'936235 | 9'121377 | 10'500582 | 28 |
| 33 | 9'057172 | 9'997156 | 9'060016 | 10'939984 | 9'118567 | 9'936219 | 9'122348 | 10'500582 | 27 |
| 34 | 9'058271 | 9'997141 | 9'061130 | 10'938870 | 9'119519 | 9'936202 | 9'123317 | 10'500603 | 26 |
| 35 | 9'059367 | 9'997127 | 9'062244 | 10'937760 | 9'120469 | 9'936185 | 9'124284 | 10'500716 | 25 |
| 36 | 9'060460 | 9'997112 | 9'063348 | 10'936652 | 9'121417 | 9'936168 | 9'125249 | 10'500751 | 24 |
| 37 | 9'061551 | 9'997098 | 9'064453 | 10'935547 | 9'122362 | 9'936151 | 9'126211 | 10'507378 | 23 |
| 38 | 9'062639 | 9'997083 | 9'065556 | 10'934444 | 9'123306 | 9'936134 | 9'127172 | 10'507282 | 22 |
| 39 | 9'063724 | 9'997068 | 9'066655 | 10'933343 | 9'124248 | 9'936117 | 9'128130 | 10'507180 | 21 |
| 40 | 9'064806 | 9'997053 | 9'067752 | 10'932248 | 9'125187 | 9'936100 | 9'129087 | 10'507093 | 20 |
| 41 | 9'065885 | 9'997039 | 9'068840 | 10'931154 | 9'126125 | 9'936083 | 9'130041 | 10'506959 | 19 |
| 42 | 9'066962 | 9'997024 | 9'069938 | 10'930062 | 9'127060 | 9'936066 | 9'130994 | 10'506906 | 18 |
| 43 | 9'068036 | 9'997009 | 9'071027 | 10'928073 | 9'127993 | 9'936049 | 9'131944 | 10'506806 | 17 |
| 44 | 9'069107 | 9'996994 | 9'072113 | 10'927857 | 9'128925 | 9'936032 | 9'132893 | 10'5067107 | 16 |
| 45 | 9'070176 | 9'996979 | 9'073197 | 10'926503 | 9'129854 | 9'936015 | 9'133839 | 10'5066161 | 15 |
| 46 | 9'071242 | 9'996964 | 9'074278 | 10'925722 | 9'130781 | 9'935985 | 9'134784 | 10'5065216 | 14 |
| 47 | 9'072306 | 9'996949 | 9'075356 | 10'924644 | 9'131706 | 9'935980 | 9'135726 | 10'5064274 | 13 |
| 48 | 9'073306 | 9'996934 | 9'076432 | 10'923508 | 9'132630 | 9'935963 | 9'136667 | 10'5063333 | 12 |
| 49 | 9'074424 | 9'996910 | 9'077505 | 10'922495 | 9'133551 | 9'935946 | 9'137605 | 10'502395 | 11 |
| 50 | 9'075480 | 9'996904 | 9'078576 | 10'921424 | 9'134470 | 9'935928 | 9'138542 | 10'501458 | 10 |
| 51 | 9'076533 | 9'996889 | 9'079644 | 10'920356 | 9'135387 | 9'935911 | 9'139476 | 10'500524 | 9 |
| 52 | 9'077583 | 9'996874 | 9'080710 | 10'919290 | 9'136303 | 9'935894 | 9'140409 | 10'500591 | 8 |
| 53 | 9'078631 | 9'996868 | 9'081773 | 10'918227 | 9'137216 | 9'935876 | 9'141340 | 10'500660 | 7 |
| 54 | 9'079676 | 9'996843 | 9'082833 | 10'917167 | 9'138128 | 9'935859 | 9'142269 | 10'500731 | 6 |
| 55 | 9'080719 | 9'996828 | 9'083801 | 10'916109 | 9'139037 | 9'935841 | 9'143196 | 10'500684 | 5 |
| 56 | 9'081759 | 9'996812 | 9'084947 | 10'915053 | 9'139944 | 9'935823 | 9'144121 | 10'5005879 | 4 |
| 57 | 9'082707 | 9'996797 | 9'086000 | 10'914000 | 9'140850 | 9'935806 | 9'145044 | 10'5004956 | 3 |
| 58 | 9'083832 | 9'996782 | 9'087050 | 10'912950 | 9'141754 | 9'935788 | 9'145966 | 10'504034 | 2 |
| 59 | 9'084864 | 9'996766 | 9'088098 | 10'911902 | 9'142655 | 9'935771 | 9'146885 | 10'503115 | 1 |
| 60 | 9'085894 | 9'996751 | 9'089144 | 10'910856 | 9'143555 | 9'935753 | 9'147803 | 10'502197 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | |

| | 8° | | | | 9° | | | | |
|----|----------|----------|----------|-----------|----------|----------|----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 6 | 9'143555 | 9'995753 | 9'147803 | 10'852197 | 9'194332 | 9'994620 | 9'199713 | 10'800287 | 60 |
| 1 | 9'144453 | 9'995736 | 9'148718 | 10'851282 | 9'195129 | 9'994600 | 9'200519 | 10'799471 | 59 |
| 2 | 9'145849 | 9'995719 | 9'149632 | 10'850368 | 9'196025 | 9'994580 | 9'201345 | 10'798655 | 58 |
| 3 | 9'146213 | 9'995699 | 9'150544 | 10'849456 | 9'196719 | 9'994560 | 9'202159 | 10'797841 | 57 |
| 4 | 9'147136 | 9'995681 | 9'151454 | 10'848446 | 9'197511 | 9'994540 | 9'202971 | 10'797029 | 56 |
| 5 | 9'148026 | 9'995664 | 9'152363 | 10'847697 | 9'198302 | 9'994519 | 9'203782 | 10'796218 | 55 |
| 6 | 9'148915 | 9'995646 | 9'153269 | 10'846731 | 9'199091 | 9'994499 | 9'204592 | 10'795408 | 54 |
| 7 | 9'149802 | 9'995628 | 9'154174 | 10'845826 | 9'199879 | 9'994479 | 9'205400 | 10'794400 | 53 |
| 8 | 9'150686 | 9'995610 | 9'155077 | 10'844923 | 9'200666 | 9'994459 | 9'206207 | 10'793793 | 52 |
| 9 | 9'151569 | 9'995591 | 9'155978 | 10'844922 | 9'201451 | 9'994438 | 9'207013 | 10'792987 | 51 |
| 10 | 9'152451 | 9'995573 | 9'156877 | 10'843123 | 9'202234 | 9'994418 | 9'207817 | 10'792183 | 50 |
| 11 | 9'153330 | 9'995553 | 9'157775 | 10'842225 | 9'203017 | 9'994396 | 9'208619 | 10'791881 | 49 |
| 12 | 9'154206 | 9'995537 | 9'180671 | 10'841329 | 9'203797 | 9'994377 | 9'209420 | 10'790580 | 48 |
| 13 | 9'155063 | 9'995519 | 9'159565 | 10'840435 | 9'204577 | 9'994357 | 9'210220 | 10'789780 | 47 |
| 14 | 9'155957 | 9'995501 | 9'160457 | 10'839543 | 9'205354 | 9'994336 | 9'211018 | 10'788882 | 46 |
| 15 | 9'156830 | 9'995482 | 9'161347 | 10'838653 | 9'206131 | 9'994316 | 9'211815 | 10'788185 | 45 |
| 16 | 9'157700 | 9'995464 | 9'162236 | 10'837764 | 9'206906 | 9'994295 | 9'212611 | 10'787389 | 44 |
| 17 | 9'158569 | 9'995446 | 9'163123 | 10'836877 | 9'207679 | 9'994274 | 9'213105 | 10'786595 | 43 |
| 18 | 9'159435 | 9'995427 | 9'164008 | 10'835992 | 9'208452 | 9'994254 | 9'213198 | 10'785802 | 42 |
| 19 | 9'160301 | 9'995409 | 9'164892 | 10'835105 | 9'209222 | 9'994233 | 9'214989 | 10'785011 | 41 |
| 20 | 9'161164 | 9'995390 | 9'165774 | 10'834226 | 9'209992 | 9'994212 | 9'215780 | 10'784220 | 40 |
| 21 | 9'162025 | 9'995372 | 9'166654 | 10'833346 | 9'210760 | 9'994191 | 9'216668 | 10'783432 | 39 |
| 22 | 9'162865 | 9'995353 | 9'167532 | 10'832466 | 9'211526 | 9'994171 | 9'217356 | 10'782644 | 38 |
| 23 | 9'163743 | 9'995334 | 9'168409 | 10'831591 | 9'212291 | 9'994150 | 9'218142 | 10'781868 | 37 |
| 24 | 9'164600 | 9'995316 | 9'169284 | 10'830716 | 9'213055 | 9'994129 | 9'218926 | 10'781074 | 36 |
| 25 | 9'165454 | 9'995297 | 9'170157 | 10'829843 | 9'213818 | 9'994108 | 9'219710 | 10'780290 | 35 |
| 26 | 9'166337 | 9'995278 | 9'171029 | 10'828971 | 9'214579 | 9'994087 | 9'220492 | 10'779508 | 34 |
| 27 | 9'167159 | 9'995260 | 9'171899 | 10'828101 | 9'215338 | 9'994066 | 9'221272 | 10'778728 | 33 |
| 28 | 9'168008 | 9'995241 | 9'172767 | 10'827233 | 9'216097 | 9'994045 | 9'222052 | 10'777948 | 32 |
| 29 | 9'168856 | 9'995222 | 9'173634 | 10'826366 | 9'216854 | 9'994024 | 9'222830 | 10'777170 | 31 |
| 30 | 9'169769 | 9'995203 | 9'174499 | 10'825501 | 9'217009 | 9'994003 | 9'223607 | 10'776393 | 30 |
| 31 | 9'170547 | 9'995181 | 9'175362 | 10'824638 | 9'218163 | 9'993989 | 9'224382 | 10'775618 | 29 |
| 32 | 9'171389 | 9'995165 | 9'176224 | 10'823776 | 9'219116 | 9'993960 | 9'225156 | 10'774844 | 28 |
| 33 | 9'172230 | 9'995146 | 9'177064 | 10'822916 | 9'219868 | 9'993939 | 9'225929 | 10'774071 | 27 |
| 34 | 9'173070 | 9'995127 | 9'177942 | 10'822058 | 9'220618 | 9'993918 | 9'226700 | 10'773300 | 26 |
| 35 | 9'173906 | 9'995098 | 9'178799 | 10'821201 | 9'221307 | 9'993897 | 9'227471 | 10'772529 | 25 |
| 36 | 9'174744 | 9'995089 | 9'179655 | 10'820345 | 9'222115 | 9'993675 | 9'228239 | 10'771761 | 24 |
| 37 | 9'175578 | 9'995070 | 9'180508 | 10'819492 | 9'222861 | 9'993854 | 9'229007 | 10'770993 | 23 |
| 38 | 9'176411 | 9'995051 | 9'181360 | 10'818646 | 9'223606 | 9'993832 | 9'229773 | 10'770227 | 22 |
| 39 | 9'177242 | 9'995032 | 9'182211 | 10'817789 | 9'224349 | 9'993811 | 9'230539 | 10'769461 | 21 |
| 40 | 9'178072 | 9'995013 | 9'183059 | 10'816941 | 9'225092 | 9'993789 | 9'231302 | 10'768698 | 20 |
| 41 | 9'178900 | 9'994993 | 9'183897 | 10'816003 | 9'225833 | 9'993768 | 9'232065 | 10'767935 | 19 |
| 42 | 9'179726 | 9'994974 | 9'184752 | 10'815248 | 9'226573 | 9'993746 | 9'232826 | 10'767174 | 18 |
| 43 | 9'180531 | 9'994955 | 9'185597 | 10'814403 | 9'227311 | 9'993725 | 9'233586 | 10'766414 | 17 |
| 44 | 9'181374 | 9'994935 | 9'186439 | 10'813561 | 9'228048 | 9'993703 | 9'234345 | 10'765665 | 16 |
| 45 | 9'182196 | 9'994916 | 9'187280 | 10'812720 | 9'228784 | 9'993681 | 9'235103 | 10'764897 | 15 |
| 46 | 9'183016 | 9'994886 | 9'188120 | 10'811880 | 9'229518 | 9'993660 | 9'235855 | 10'764141 | 14 |
| 47 | 9'183884 | 9'994877 | 9'188958 | 10'811042 | 9'230262 | 9'993638 | 9'236614 | 10'763386 | 13 |
| 48 | 9'184651 | 9'994857 | 9'189794 | 10'810200 | 9'230984 | 9'993616 | 9'237308 | 10'762682 | 12 |
| 49 | 9'185406 | 9'994838 | 9'190629 | 10'809371 | 9'231715 | 9'993594 | 9'238120 | 10'761880 | 11 |
| 50 | 9'186280 | 9'994818 | 9'191462 | 10'808539 | 9'232444 | 9'993572 | 9'238582 | 10'761128 | 10 |
| 51 | 9'187092 | 9'994798 | 9'192294 | 10'807706 | 9'233172 | 9'993550 | 9'239422 | 10'760378 | 9 |
| 52 | 9'187903 | 9'994779 | 9'193124 | 10'806876 | 9'233899 | 9'993528 | 9'240371 | 10'759629 | 8 |
| 53 | 9'188712 | 9'994759 | 9'193953 | 10'806047 | 9'234426 | 9'993506 | 9'241118 | 10'758882 | 7 |
| 54 | 9'189519 | 9'994739 | 9'194780 | 10'805220 | 9'235349 | 9'993484 | 9'241866 | 10'758135 | 6 |
| 55 | 9'190325 | 9'994720 | 9'195606 | 10'804394 | 9'236073 | 9'993462 | 9'242610 | 10'757890 | 5 |
| 56 | 9'191130 | 9'994700 | 9'196430 | 10'803570 | 9'236795 | 9'993440 | 9'243354 | 10'756464 | 4 |
| 57 | 9'191933 | 9'994680 | 9'197253 | 10'802747 | 9'237515 | 9'993418 | 9'244097 | 10'755908 | 3 |
| 58 | 9'192734 | 9'994660 | 9'198074 | 10'801926 | 9'238235 | 9'993396 | 9'244839 | 10'755161 | 2 |
| 59 | 9'193184 | 9'994640 | 9'198894 | 10'801106 | 9'238953 | 9'993374 | 9'245579 | 10'754421 | 1 |
| 60 | 9'194322 | 9'994620 | 9'199713 | 10'800287 | 9'239670 | 9'993351 | 9'246319 | 10'753681 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | / |

| 10° | | | | 11° | | | |
|-------------|-----------|----------|-----------|----------|----------|----------|--------------|
| Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. |
| 0 9 239070 | 9 999361 | 9 246819 | 10 753081 | 9 280509 | 9 991047 | 9 288652 | 10 711348 60 |
| 1 9 240386 | 9 999339 | 9 247057 | 10 752943 | 9 281248 | 9 991922 | 9 289326 | 10 710674 59 |
| 2 9 241101 | 9 999307 | 9 247794 | 10 752200 | 9 281897 | 9 991897 | 9 280999 | 10 710001 58 |
| 3 9 241814 | 9 999284 | 9 248580 | 10 751470 | 9 282544 | 9 991878 | 9 290671 | 10 709329 57 |
| 4 9 242526 | 9 999262 | 9 249204 | 10 750736 | 9 283190 | 9 991848 | 9 291342 | 10 708658 56 |
| 5 9 243237 | 9 999240 | 9 249998 | 10 750002 | 9 283836 | 9 991823 | 9 292013 | 10 707987 55 |
| 6 9 243947 | 9 999217 | 9 250730 | 10 749270 | 9 284480 | 9 991799 | 9 292682 | 10 707318 54 |
| 7 9 244650 | 9 999195 | 9 251461 | 10 748539 | 9 285124 | 9 991774 | 9 293350 | 10 706650 53 |
| 8 9 245363 | 9 999172 | 9 252191 | 10 747809 | 9 285766 | 9 991749 | 9 294017 | 10 705983 52 |
| 9 9 246069 | 9 999149 | 9 252920 | 10 747080 | 9 286408 | 9 991724 | 9 294684 | 10 705316 51 |
| 10 9 246775 | 9 999127 | 9 253648 | 10 746382 | 9 287048 | 9 991699 | 9 295349 | 10 704651 50 |
| 11 9 247478 | 9 999104 | 9 254374 | 10 745626 | 9 287688 | 9 991674 | 9 296013 | 10 703987 49 |
| 12 9 248181 | 9 999081 | 9 255100 | 10 744900 | 9 288326 | 9 991649 | 9 296677 | 10 703323 48 |
| — | — | — | — | — | — | — | — |
| 13 9 248883 | 9 999059 | 9 255824 | 10 744176 | 9 288064 | 9 991624 | 9 297339 | 10 702661 47 |
| 14 9 249583 | 9 999036 | 9 256547 | 10 743453 | 9 289600 | 9 991599 | 9 298001 | 10 701999 46 |
| 15 9 250282 | 9 999013 | 9 257269 | 10 742781 | 9 290236 | 9 991574 | 9 298662 | 10 701338 45 |
| 16 9 250980 | 9 999090 | 9 257990 | 10 742010 | 9 290870 | 9 991549 | 9 299322 | 10 700678 44 |
| 17 9 251677 | 9 999067 | 9 258710 | 10 741290 | 9 291504 | 9 991524 | 9 299980 | 10 700020 43 |
| 18 9 252373 | 9 999044 | 9 259429 | 10 740571 | 9 292137 | 9 991498 | 9 300638 | 10 699362 42 |
| 19 9 253067 | 9 999021 | 9 260146 | 10 739854 | 9 292768 | 9 991473 | 9 301295 | 10 698705 41 |
| 20 9 253761 | 9 999096 | 9 260863 | 10 739137 | 9 293399 | 9 991448 | 9 301951 | 10 698049 40 |
| 21 9 254453 | 9 9992875 | 9 261578 | 10 738422 | 9 294029 | 9 991422 | 9 302607 | 10 697393 39 |
| 22 9 255144 | 9 9992852 | 9 262292 | 10 737708 | 9 294658 | 9 991397 | 9 303261 | 10 696739 38 |
| 23 9 255831 | 9 9992829 | 9 263005 | 10 736905 | 9 295286 | 9 991372 | 9 303914 | 10 696086 37 |
| 24 9 256523 | 9 9992806 | 9 263717 | 10 736283 | 9 295913 | 9 991346 | 9 304667 | 10 695433 36 |
| — | — | — | — | — | — | — | — |
| 25 9 257211 | 9 9992783 | 9 264428 | 10 735572 | 9 296539 | 9 991321 | 9 305218 | 10 694782 35 |
| 26 9 257898 | 9 9992759 | 9 265138 | 10 734862 | 9 297164 | 9 991295 | 9 305869 | 10 694131 34 |
| 27 9 258583 | 9 9992736 | 9 265847 | 10 734153 | 9 297788 | 9 991270 | 9 306519 | 10 693481 33 |
| 28 9 259268 | 9 9992713 | 9 266555 | 10 733445 | 9 298412 | 9 991244 | 9 307168 | 10 692832 32 |
| 29 9 259951 | 9 9992690 | 9 267261 | 10 732739 | 9 299034 | 9 991218 | 9 307816 | 10 692184 31 |
| 30 9 260633 | 9 9992666 | 9 267967 | 10 732033 | 9 299655 | 9 991193 | 9 308463 | 10 691537 30 |
| 31 9 261314 | 9 9992443 | 9 268671 | 10 731529 | 9 300276 | 9 991167 | 9 309109 | 10 690891 29 |
| 32 9 261991 | 9 9992619 | 9 269375 | 10 730625 | 9 300895 | 9 991141 | 9 309754 | 10 690246 28 |
| 33 9 262673 | 9 9992596 | 9 270077 | 10 729293 | 9 301514 | 9 991115 | 9 300399 | 10 699601 27 |
| 34 9 263351 | 9 9992572 | 9 270779 | 10 729221 | 9 302132 | 9 991090 | 9 311042 | 10 688958 26 |
| 35 9 264027 | 9 9992549 | 9 271479 | 10 728521 | 9 302748 | 9 991064 | 9 311685 | 10 688315 25 |
| 36 9 264703 | 9 9992525 | 9 272178 | 10 727822 | 9 303364 | 9 991038 | 9 312237 | 10 687673 24 |
| — | — | — | — | — | — | — | — |
| 37 9 265377 | 9 9992501 | 9 272976 | 10 727124 | 9 303879 | 9 991012 | 9 312968 | 10 687032 23 |
| 38 9 266051 | 9 9992478 | 9 273573 | 10 726127 | 9 304593 | 9 990986 | 9 313608 | 10 686392 22 |
| 39 9 266723 | 9 9992454 | 9 274269 | 10 725731 | 9 305207 | 9 990960 | 9 314247 | 10 685753 21 |
| 40 9 267395 | 9 9992430 | 9 274964 | 10 725036 | 9 305819 | 9 990934 | 9 314885 | 10 685115 20 |
| 41 9 268065 | 9 9992406 | 9 275658 | 10 724342 | 9 306430 | 9 990908 | 9 315523 | 10 684477 19 |
| 42 9 268734 | 9 9992382 | 9 276351 | 10 723649 | 9 307041 | 9 990882 | 9 316159 | 10 683841 18 |
| 43 9 269402 | 9 9992359 | 9 277043 | 10 722957 | 9 307650 | 9 990855 | 9 316795 | 10 683205 17 |
| 44 9 270069 | 9 9992335 | 9 277734 | 10 722266 | 9 308259 | 9 990829 | 9 317430 | 10 682570 16 |
| 45 9 270735 | 9 9992311 | 9 278424 | 10 721576 | 9 308867 | 9 990803 | 9 318064 | 10 681936 15 |
| 46 9 271400 | 9 9992287 | 9 279113 | 10 720887 | 9 309474 | 9 990777 | 9 318697 | 10 681303 14 |
| 47 9 272064 | 9 9992263 | 9 279801 | 10 720199 | 9 310060 | 9 990750 | 9 319330 | 10 680670 13 |
| 48 9 272726 | 9 9992239 | 9 280488 | 10 719512 | 9 310685 | 9 990724 | 9 319961 | 10 680089 12 |
| — | — | — | — | — | — | — | — |
| 49 9 273388 | 9 9992214 | 9 282174 | 10 718826 | 9 311289 | 9 990697 | 9 320592 | 10 679408 11 |
| 50 9 274049 | 9 9992190 | 9 281858 | 10 718142 | 9 311893 | 9 990671 | 9 321222 | 10 678778 10 |
| 51 9 274708 | 9 9992166 | 9 282542 | 10 717458 | 9 312495 | 9 990645 | 9 321851 | 10 678149 9 |
| 52 9 275367 | 9 9992142 | 9 283225 | 10 716775 | 9 313097 | 9 990618 | 9 322479 | 10 677521 8 |
| 53 9 276025 | 9 9992118 | 9 283907 | 10 716093 | 9 313608 | 9 990591 | 9 323106 | 10 676894 7 |
| 54 9 276681 | 9 9992093 | 9 284586 | 10 715412 | 9 314297 | 9 990565 | 9 323733 | 10 676267 6 |
| 55 9 277337 | 9 9992069 | 9 285268 | 10 714732 | 9 314897 | 9 990538 | 9 324356 | 10 675642 5 |
| 56 9 277991 | 9 9992044 | 9 285947 | 10 714053 | 9 315495 | 9 990511 | 9 324963 | 10 675017 4 |
| 57 9 278645 | 9 9992020 | 9 286624 | 10 713376 | 9 316092 | 9 990485 | 9 325607 | 10 674393 3 |
| 58 9 279297 | 9 9991996 | 9 287301 | 10 712699 | 9 316689 | 9 990458 | 9 326231 | 10 673769 2 |
| 59 9 279948 | 9 9991971 | 9 287977 | 10 712023 | 9 317284 | 9 990431 | 9 326853 | 10 673147 1 |
| 60 9 280599 | 9 9991947 | 9 288652 | 10 711348 | 9 317879 | 9 990404 | 9 327475 | 10 672525 0 |

| | 12° | | | | 13° | | | | |
|----|----------|----------|----------|-----------|----------|----------|----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 9.317879 | 9.990404 | 9.327475 | 10.672525 | 9.352088 | 9.986724 | 9.363364 | 10.630636 | 60 |
| 1 | 9.318473 | 9.990378 | 9.328095 | 10.671905 | 9.352635 | 9.986695 | 9.363040 | 10.630660 | 59 |
| 2 | 9.319066 | 9.990351 | 9.328715 | 10.671285 | 9.353181 | 9.986666 | 9.364515 | 10.635485 | 58 |
| 3 | 9.319658 | 9.990324 | 9.329334 | 10.670666 | 9.353726 | 9.986636 | 9.365000 | 10.634910 | 57 |
| 4 | 9.320249 | 9.990297 | 9.329953 | 10.670047 | 9.354271 | 9.986607 | 9.365064 | 10.634336 | 56 |
| 5 | 9.320840 | 9.990270 | 9.330570 | 10.669430 | 9.354815 | 9.986578 | 9.366237 | 10.633763 | 55 |
| 6 | 9.321430 | 9.990243 | 9.331187 | 10.668813 | 9.355358 | 9.986548 | 9.366810 | 10.633190 | 54 |
| 7 | 9.322019 | 9.990215 | 9.331603 | 10.668197 | 9.355901 | 9.986519 | 9.367382 | 10.632618 | 53 |
| 8 | 9.322607 | 9.990185 | 9.332418 | 10.667582 | 9.356443 | 9.986489 | 9.367068 | 10.632047 | 52 |
| 9 | 9.323194 | 9.990161 | 9.333033 | 10.666967 | 9.356984 | 9.986460 | 9.368524 | 10.631476 | 51 |
| 10 | 9.323780 | 9.990134 | 9.333646 | 10.666354 | 9.357624 | 9.986430 | 9.369094 | 10.630966 | 50 |
| 11 | 9.324366 | 9.990107 | 9.334259 | 10.665741 | 9.358064 | 9.986401 | 9.369663 | 10.630337 | 49 |
| 12 | 9.324950 | 9.990079 | 9.334871 | 10.665129 | 9.358603 | 9.986371 | 9.370232 | 10.629768 | 48 |
| 13 | 9.325534 | 9.990052 | 9.335482 | 10.664518 | 9.359141 | 9.986342 | 9.370799 | 10.629201 | 47 |
| 14 | 9.326117 | 9.990025 | 9.336093 | 10.663907 | 9.359678 | 9.986312 | 9.371367 | 10.628033 | 46 |
| 15 | 9.326700 | 9.989997 | 9.336702 | 10.663298 | 9.360215 | 9.986282 | 9.371933 | 10.628067 | 45 |
| 16 | 9.327281 | 9.989970 | 9.337311 | 10.662689 | 9.360752 | 9.986252 | 9.372499 | 10.627501 | 44 |
| 17 | 9.327862 | 9.989942 | 9.337919 | 10.662081 | 9.361287 | 9.986223 | 9.373064 | 10.626936 | 43 |
| 18 | 9.328442 | 9.989915 | 9.338527 | 10.661473 | 9.361822 | 9.986193 | 9.373629 | 10.626371 | 42 |
| 19 | 9.329021 | 9.989887 | 9.339133 | 10.660867 | 9.362356 | 9.986163 | 9.374193 | 10.625807 | 41 |
| 20 | 9.329599 | 9.989860 | 9.339739 | 10.660261 | 9.362889 | 9.986133 | 9.374756 | 10.625244 | 40 |
| 21 | 9.330176 | 9.989832 | 9.340344 | 10.659566 | 9.363422 | 9.986103 | 9.375319 | 10.624081 | 39 |
| 22 | 9.330753 | 9.989804 | 9.340948 | 10.659052 | 9.363954 | 9.986073 | 9.375881 | 10.624119 | 38 |
| 23 | 9.331329 | 9.989777 | 9.341552 | 10.658448 | 9.364485 | 9.986043 | 9.376442 | 10.623558 | 37 |
| 24 | 9.331903 | 9.989749 | 9.342155 | 10.657845 | 9.365016 | 9.988013 | 9.377003 | 10.622997 | 36 |
| 25 | 9.332478 | 9.989721 | 9.342757 | 10.657243 | 9.365546 | 9.987983 | 9.377563 | 10.622437 | 35 |
| 26 | 9.333051 | 9.989693 | 9.343358 | 10.656042 | 9.366075 | 9.987958 | 9.378122 | 10.621878 | 34 |
| 27 | 9.333624 | 9.989665 | 9.343958 | 10.656042 | 9.366604 | 9.987922 | 9.378681 | 10.621319 | 33 |
| 28 | 9.334195 | 9.989637 | 9.344556 | 10.655442 | 9.367131 | 9.987692 | 9.379239 | 10.620761 | 32 |
| 29 | 9.334767 | 9.989610 | 9.345157 | 10.654843 | 9.367659 | 9.987662 | 9.379797 | 10.620203 | 31 |
| 30 | 9.335337 | 9.989582 | 9.345755 | 10.654245 | 9.368185 | 9.987832 | 9.380354 | 10.619646 | 30 |
| 31 | 9.335906 | 9.989553 | 9.346353 | 10.653647 | 9.368711 | 9.987801 | 9.380910 | 10.619090 | 29 |
| 32 | 9.336475 | 9.989525 | 9.346949 | 10.653051 | 9.369236 | 9.987771 | 9.381466 | 10.618534 | 28 |
| 33 | 9.337043 | 9.989497 | 9.347545 | 10.652455 | 9.369761 | 9.987740 | 9.382020 | 10.617980 | 27 |
| 34 | 9.337610 | 9.989469 | 9.348141 | 10.651859 | 9.370285 | 9.987710 | 9.382575 | 10.617425 | 26 |
| 35 | 9.338176 | 9.989441 | 9.348735 | 10.651265 | 9.370808 | 9.987679 | 9.383129 | 10.616871 | 25 |
| 36 | 9.338742 | 9.989413 | 9.349329 | 10.650671 | 9.371330 | 9.987649 | 9.383682 | 10.616318 | 24 |
| 37 | 9.339307 | 9.989385 | 9.349922 | 10.650078 | 9.371852 | 9.987618 | 9.384234 | 10.615766 | 23 |
| 38 | 9.339871 | 9.989356 | 9.350514 | 10.649486 | 9.372373 | 9.987588 | 9.384786 | 10.615214 | 22 |
| 39 | 9.340434 | 9.989328 | 9.351106 | 10.648894 | 9.372804 | 9.987557 | 9.385337 | 10.614663 | 21 |
| 40 | 9.340996 | 9.989300 | 9.351697 | 10.648303 | 9.373414 | 9.987526 | 9.385886 | 10.614112 | 20 |
| 41 | 9.341558 | 9.989271 | 9.352287 | 10.647713 | 9.373933 | 9.987496 | 9.386438 | 10.613562 | 19 |
| 42 | 9.342119 | 9.989243 | 9.352876 | 10.647124 | 9.374452 | 9.987466 | 9.386987 | 10.613013 | 18 |
| 43 | 9.342679 | 9.989214 | 9.353465 | 10.646535 | 9.374970 | 9.987434 | 9.387536 | 10.612464 | 17 |
| 44 | 9.343239 | 9.989186 | 9.354053 | 10.645047 | 9.375487 | 9.987403 | 9.388084 | 10.611916 | 16 |
| 45 | 9.343797 | 9.989157 | 9.354640 | 10.645360 | 9.376003 | 9.987372 | 9.388631 | 10.611369 | 15 |
| 46 | 9.344355 | 9.989128 | 9.355227 | 10.644773 | 9.376519 | 9.987341 | 9.389178 | 10.610822 | 14 |
| 47 | 9.344912 | 9.989100 | 9.355813 | 10.644187 | 9.377085 | 9.987310 | 9.389724 | 10.610276 | 13 |
| 48 | 9.345469 | 9.989071 | 9.356398 | 10.643602 | 9.377549 | 9.987279 | 9.390270 | 10.609730 | 12 |
| 49 | 9.346024 | 9.989042 | 9.356982 | 10.643018 | 9.378063 | 9.987248 | 9.390815 | 10.609185 | 11 |
| 50 | 9.346579 | 9.989014 | 9.357566 | 10.642434 | 9.378577 | 9.987217 | 9.391360 | 10.608640 | 10 |
| 51 | 9.347134 | 9.988985 | 9.358140 | 10.641851 | 9.379089 | 9.987186 | 9.391903 | 10.608097 | 9 |
| 52 | 9.347687 | 9.988956 | 9.358731 | 10.641269 | 9.379601 | 9.987155 | 9.392447 | 10.607553 | 8 |
| 53 | 9.348240 | 9.988927 | 9.359313 | 10.640687 | 9.380113 | 9.987124 | 9.392989 | 10.607011 | 7 |
| 54 | 9.348792 | 9.988896 | 9.359893 | 10.640107 | 9.380624 | 9.987092 | 9.393531 | 10.606469 | 6 |
| 55 | 9.349343 | 9.988869 | 9.360474 | 10.639526 | 9.381184 | 9.987061 | 9.394073 | 10.605927 | 5 |
| 56 | 9.349893 | 9.988840 | 9.361053 | 10.638947 | 9.381642 | 9.987030 | 9.394614 | 10.605386 | 4 |
| 57 | 9.350443 | 9.988811 | 9.361632 | 10.638368 | 9.382152 | 9.986998 | 9.395154 | 10.604846 | 3 |
| 58 | 9.350992 | 9.988782 | 9.362210 | 10.637790 | 9.382601 | 9.986967 | 9.395694 | 10.604306 | 2 |
| 59 | 9.351540 | 9.988753 | 9.362787 | 10.637213 | 9.383168 | 9.986936 | 9.396233 | 10.603876 | 1 |
| 60 | 9.352088 | 9.988724 | 9.363364 | 10.636366 | 9.383675 | 9.986904 | 9.396771 | 10.603229 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | / |

14°

| | Sine | Cosine | Tan. | Cotan. | | Sine | Cosine | Tan. | Cotan. | |
|----|----------|-----------|----------|-----------|----------|----------|----------|-----------|--------|--|
| 0 | 9·383675 | 9·986934 | 9·396771 | 10·603229 | 9·412936 | 9·984941 | 9·428052 | 10·571948 | 60 | |
| 1 | 9·384182 | 9·986873 | 9·397309 | 10·602631 | 9·413407 | 9·984910 | 9·428553 | 10·571442 | 59 | |
| 2 | 9·384687 | 9·986841 | 9·397846 | 10·602154 | 9·413939 | 9·984876 | 9·429062 | 10·570938 | 58 | |
| 3 | 9·385192 | 9·986909 | 9·398383 | 10·601617 | 9·414409 | 9·984812 | 9·429566 | 10·570434 | 57 | |
| 4 | 9·385697 | 9·986778 | 9·39919 | 10·601081 | 9·414978 | 9·984808 | 9·430070 | 10·569930 | 56 | |
| 5 | 9·386201 | 9·986746 | 9·399155 | 10·600545 | 9·415347 | 9·984774 | 9·430573 | 10·569427 | 55 | |
| 6 | 9·386704 | 9·986714 | 9·399990 | 10·600010 | 9·415815 | 9·984740 | 9·431075 | 10·568925 | 54 | |
| 7 | 9·387207 | 9·986083 | 9·400524 | 10·593476 | 9·416283 | 9·984706 | 9·431577 | 10·568423 | 53 | |
| 8 | 9·387709 | 9·986651 | 9·401058 | 10·593412 | 9·416751 | 9·984672 | 9·432079 | 10·567921 | 52 | |
| 9 | 9·388210 | 9·986619 | 9·401591 | 10·594909 | 9·417217 | 9·984638 | 9·432540 | 10·567420 | 51 | |
| 10 | 9·388711 | 9·986587 | 9·402124 | 10·597876 | 9·417684 | 9·984603 | 9·433080 | 10·566920 | 50 | |
| 11 | 9·389211 | 9·986555 | 9·402656 | 10·573741 | 9·418150 | 9·984569 | 9·433540 | 10·566420 | 49 | |
| 12 | 9·389711 | 9·986523 | 9·403187 | 10·596813 | 9·418615 | 9·984535 | 9·434090 | 10·565920 | 48 | |
| 13 | 9·390210 | 9·986491 | 9·403718 | 10·562682 | 9·419079 | 9·984500 | 9·434579 | 10·565421 | 47 | |
| 14 | 9·390708 | 9·986459 | 9·404249 | 10·597151 | 9·419544 | 9·984466 | 9·435078 | 10·564922 | 46 | |
| 15 | 9·391206 | 9·986427 | 9·404778 | 10·595222 | 9·420007 | 9·984432 | 9·435576 | 10·564424 | 45 | |
| 16 | 9·391703 | 9·986395 | 9·405305 | 10·591602 | 9·420170 | 9·984397 | 9·436073 | 10·563927 | 44 | |
| 17 | 9·392199 | 9·986363 | 9·405836 | 10·591464 | 9·420933 | 9·984363 | 9·436570 | 10·563430 | 43 | |
| 18 | 9·392695 | 9·986331 | 9·406364 | 10·593636 | 9·421395 | 9·984328 | 9·437067 | 10·562933 | 42 | |
| 19 | 9·393191 | 9·986299 | 9·406892 | 10·593108 | 9·421857 | 9·984294 | 9·437563 | 10·562437 | 41 | |
| 20 | 9·393685 | 9·986266 | 9·407419 | 10·592381 | 9·422318 | 9·984259 | 9·438059 | 10·561941 | 40 | |
| 21 | 9·394177 | 9·986234 | 9·407945 | 10·592055 | 9·422778 | 9·984221 | 9·438554 | 10·561446 | 39 | |
| 22 | 9·394673 | 9·986202 | 9·408171 | 10·591529 | 9·423238 | 9·984190 | 9·439048 | 10·560952 | 38 | |
| 23 | 9·395166 | 9·986169 | 9·408696 | 10·591001 | 9·423697 | 9·984155 | 9·439563 | 10·560457 | 37 | |
| 24 | 9·395658 | 9·986137 | 9·409521 | 10·590479 | 9·424156 | 9·984120 | 9·440036 | 10·559964 | 36 | |
| 25 | 9·396150 | 9·986104 | 9·410045 | 10·589455 | 9·424165 | 9·984085 | 9·440529 | 10·559471 | 35 | |
| 26 | 9·396641 | 9·986072 | 9·410569 | 10·589131 | 9·425073 | 9·984050 | 9·441022 | 10·558978 | 34 | |
| 27 | 9·397132 | 9·986039 | 9·411032 | 10·588904 | 9·425530 | 9·984015 | 9·441514 | 10·558486 | 33 | |
| 28 | 9·397621 | 9·986007 | 9·411615 | 10·588385 | 9·425987 | 9·983981 | 9·442006 | 10·557994 | 32 | |
| 29 | 9·398111 | 9·985974 | 9·412137 | 10·587863 | 9·426443 | 9·983916 | 9·442497 | 10·557503 | 31 | |
| 30 | 9·398600 | 9·985942 | 9·412658 | 10·587342 | 9·426899 | 9·983911 | 9·442988 | 10·557012 | 30 | |
| 31 | 9·399088 | 9·985909 | 9·413179 | 10·586821 | 9·427354 | 9·983875 | 9·443179 | 10·556521 | 29 | |
| 32 | 9·399575 | 9·985876 | 9·413699 | 10·586301 | 9·427809 | 9·983840 | 9·443968 | 10·556032 | 28 | |
| 33 | 9·400062 | 9·985813 | 9·414219 | 10·585781 | 9·428263 | 9·983805 | 9·444458 | 10·555542 | 27 | |
| 34 | 9·400549 | 9·985811 | 9·414738 | 10·585262 | 9·428717 | 9·983787 | 9·444947 | 10·555053 | 26 | |
| 35 | 9·401035 | 9·985778 | 9·415257 | 10·584743 | 9·429170 | 9·983735 | 9·445435 | 10·554565 | 25 | |
| 36 | 9·401520 | 9·985743 | 9·415775 | 10·584225 | 9·429623 | 9·983700 | 9·445923 | 10·554077 | 24 | |
| 37 | 9·402065 | 9·985712 | 9·416293 | 10·583707 | 9·430075 | 9·983661 | 9·446111 | 10·553589 | 23 | |
| 38 | 9·402489 | 9·985679 | 9·416810 | 10·583196 | 9·430527 | 9·983629 | 9·446898 | 10·553102 | 22 | |
| 39 | 9·402972 | 9·985616 | 9·417326 | 10·582674 | 9·430978 | 9·983594 | 9·447394 | 10·552616 | 21 | |
| 40 | 9·403415 | 9·9853613 | 9·417842 | 10·582158 | 9·431429 | 9·983558 | 9·447870 | 10·552130 | 20 | |
| 41 | 9·403938 | 9·985580 | 9·418358 | 10·581642 | 9·431879 | 9·983523 | 9·448356 | 10·551644 | 19 | |
| 42 | 9·404420 | 9·985547 | 9·418873 | 10·581127 | 9·432329 | 9·983487 | 9·448841 | 10·551159 | 18 | |
| 43 | 9·404901 | 9·985514 | 9·419387 | 10·580613 | 9·43277- | 9·983452 | 9·449326 | 10·550674 | 17 | |
| 44 | 9·405382 | 9·985480 | 9·419901 | 10·580909 | 9·433220 | 9·983416 | 9·449910 | 10·550190 | 16 | |
| 45 | 9·405862 | 9·985447 | 9·420415 | 10·579585 | 9·433675 | 9·983381 | 9·450294 | 10·549706 | 15 | |
| 46 | 9·406311 | 9·985414 | 9·420927 | 10·579073 | 9·434122 | 9·983315 | 9·450777 | 10·549223 | 14 | |
| 47 | 9·406820 | 9·985381 | 9·421410 | 10·578360 | 9·434569 | 9·983309 | 9·451260 | 10·548740 | 13 | |
| 48 | 9·407299 | 9·985347 | 9·421952 | 10·578019 | 9·435016 | 9·983273 | 9·451743 | 10·548257 | 12 | |
| 49 | 9·407777 | 9·985314 | 9·422463 | 10·577537 | 9·435462 | 9·983238 | 9·452225 | 10·547775 | 11 | |
| 50 | 9·408251 | 9·985280 | 9·422971 | 10·577026 | 9·435908 | 9·983202 | 9·452706 | 10·547294 | 10 | |
| 51 | 9·408731 | 9·985217 | 9·423481 | 10·576516 | 9·436353 | 9·983166 | 9·453187 | 10·546813 | 9 | |
| 52 | 9·409207 | 9·985213 | 9·423993 | 10·576007 | 9·436798 | 9·983130 | 9·453668 | 10·546332 | 8 | |
| 53 | 9·409682 | 9·985180 | 9·424503 | 10·575497 | 9·437242 | 9·983094 | 9·454148 | 10·545852 | 7 | |
| 54 | 9·410157 | 9·985146 | 9·425011 | 10·574989 | 9·437646 | 9·983058 | 9·454628 | 10·545372 | 6 | |
| 55 | 9·410632 | 9·985113 | 9·425519 | 10·574481 | 9·438129 | 9·983028 | 9·455107 | 10·544898 | 5 | |
| 56 | 9·411106 | 9·985079 | 9·426027 | 10·573973 | 9·438572 | 9·982986 | 9·455546 | 10·544414 | 4 | |
| 57 | 9·411579 | 9·985045 | 9·426534 | 10·573409 | 9·439014 | 9·982950 | 9·456061 | 10·543936 | 3 | |
| 58 | 9·412052 | 9·985011 | 9·427011 | 10·572959 | 9·439456 | 9·982914 | 9·456542 | 10·543158 | 2 | |
| 59 | 9·412524 | 9·984978 | 9·427547 | 10·572453 | 9·439947 | 9·982878 | 9·457019 | 10·542981 | 1 | |
| 60 | 9·412996 | 9·984944 | 9·428052 | 10·571948 | 9·440338 | 9·982842 | 9·457496 | 10·542504 | 0 | |
| | Cosine | Sine | Cotan. | Tan. | | Cosine | Sine | Cotan. | Tan. | |

| 16° | | | | 17° | | | | | |
|-----|----------|----------|----------|-----------|----------|----------|----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 9 440338 | 9 982842 | 9 457496 | 10 542504 | 9 465935 | 9 980596 | 9 485339 | 10 514661 | 60 |
| 1 | 9 440776 | 9 982806 | 9 457973 | 10 542027 | 9 466342 | 9 980558 | 9 485791 | 10 514209 | 53 |
| 2 | 9 441213 | 9 982769 | 9 458449 | 10 541551 | 9 466761 | 9 980519 | 9 486242 | 10 513758 | 59 |
| 3 | 9 441658 | 9 982733 | 9 458925 | 10 541075 | 9 467173 | 9 980480 | 9 486693 | 10 513307 | 57 |
| 4 | 9 442096 | 9 982696 | 9 459400 | 10 540600 | 9 467585 | 9 980442 | 9 487143 | 10 512857 | 56 |
| 5 | 9 442535 | 9 982660 | 9 459875 | 10 540125 | 9 467996 | 9 980403 | 9 487593 | 10 512407 | 55 |
| 6 | 9 442973 | 9 982624 | 9 460349 | 10 539651 | 9 468407 | 9 980364 | 9 488043 | 10 511957 | 54 |
| 7 | 9 443410 | 9 982587 | 9 460823 | 10 539177 | 9 468817 | 9 980325 | 9 488492 | 10 511508 | 53 |
| 8 | 9 443847 | 9 982551 | 9 461297 | 10 538703 | 9 469227 | 9 980286 | 9 488941 | 10 511059 | 52 |
| 9 | 9 444284 | 9 982514 | 9 461770 | 10 538330 | 9 469637 | 9 980247 | 9 489390 | 10 510610 | 51 |
| 10 | 9 444720 | 9 983177 | 9 462242 | 10 537758 | 9 470046 | 9 980208 | 9 489838 | 10 510162 | 50 |
| 11 | 9 445155 | 9 982441 | 9 462715 | 10 537285 | 9 470455 | 9 980169 | 9 490266 | 10 509714 | 49 |
| 12 | 9 445590 | 9 982404 | 9 463186 | 10 536814 | 9 470869 | 9 980130 | 9 490733 | 10 509267 | 48 |
| 13 | 9 446025 | 9 982367 | 9 468658 | 10 536342 | 9 471271 | 9 980091 | 9 491180 | 10 508820 | 47 |
| 14 | 9 446459 | 9 982331 | 9 464128 | 10 535872 | 9 471679 | 9 980052 | 9 491627 | 10 508373 | 46 |
| 15 | 9 446893 | 9 982294 | 9 464599 | 10 535401 | 9 472086 | 9 980012 | 9 492073 | 10 507927 | 45 |
| 16 | 9 447226 | 9 982257 | 9 465069 | 10 534931 | 9 472492 | 9 979973 | 9 492519 | 10 507481 | 44 |
| 17 | 9 447759 | 9 982220 | 9 465539 | 10 534461 | 9 472898 | 9 979934 | 9 492965 | 10 507035 | 43 |
| 18 | 9 448191 | 9 982183 | 9 466008 | 10 533992 | 9 473304 | 9 979895 | 9 493410 | 10 506590 | 42 |
| 19 | 9 448623 | 9 982146 | 9 466477 | 10 533523 | 9 473710 | 9 979835 | 9 498851 | 10 506140 | 41 |
| 20 | 9 449054 | 9 982109 | 9 466945 | 10 533055 | 9 474115 | 9 979816 | 9 494299 | 10 505701 | 40 |
| 21 | 9 449485 | 9 982072 | 9 467413 | 10 532557 | 9 474519 | 9 979776 | 9 494748 | 10 505257 | 39 |
| 22 | 9 449915 | 9 982035 | 9 467880 | 10 532120 | 9 474923 | 9 979737 | 9 495186 | 10 504814 | 38 |
| 23 | 9 450345 | 9 981998 | 9 468317 | 10 531653 | 9 475327 | 9 979697 | 9 495630 | 10 504370 | 37 |
| 24 | 9 450775 | 9 981961 | 9 468714 | 10 531186 | 9 475730 | 9 979658 | 9 496078 | 10 503927 | 36 |
| 25 | 9 451204 | 9 981924 | 9 469280 | 10 530720 | 9 476133 | 9 979618 | 9 496515 | 10 503485 | 35 |
| 26 | 9 451632 | 9 981886 | 9 469746 | 10 530254 | 9 476536 | 9 979579 | 9 496957 | 10 503043 | 34 |
| 27 | 9 452060 | 9 981849 | 9 470211 | 10 529789 | 9 476938 | 9 979539 | 9 497399 | 10 502601 | 33 |
| 28 | 9 452488 | 9 981812 | 9 470676 | 10 529324 | 9 477340 | 9 979499 | 9 497841 | 10 502153 | 32 |
| 29 | 9 452915 | 9 981774 | 9 471141 | 10 528859 | 9 477741 | 9 979459 | 9 498282 | 10 501718 | 31 |
| 30 | 9 453342 | 9 981737 | 9 471605 | 10 528395 | 9 478142 | 9 979420 | 9 498722 | 10 501278 | 30 |
| 31 | 9 453768 | 9 981700 | 9 472069 | 10 527931 | 9 471542 | 9 979380 | 9 499163 | 10 500937 | 29 |
| 32 | 9 454114 | 9 981662 | 9 472532 | 10 527468 | 9 478942 | 9 979340 | 9 499603 | 10 500397 | 28 |
| 33 | 9 454161 | 9 981626 | 9 472995 | 10 527005 | 9 479342 | 9 979300 | 9 500042 | 10 499958 | 27 |
| 34 | 9 455044 | 9 981587 | 9 473457 | 10 526543 | 9 479741 | 9 979260 | 9 500481 | 10 499519 | 26 |
| 35 | 9 455469 | 9 981549 | 9 473919 | 10 526081 | 9 480140 | 9 979220 | 9 500920 | 10 499080 | 25 |
| 36 | 9 455893 | 9 981512 | 9 474361 | 10 525619 | 9 480539 | 9 979180 | 9 501359 | 10 498641 | 24 |
| 37 | 9 456316 | 9 981474 | 9 474842 | 10 525155 | 9 480937 | 9 979140 | 9 501797 | 10 498203 | 23 |
| 38 | 9 456739 | 9 981436 | 9 475303 | 10 524697 | 9 481334 | 9 979100 | 9 502235 | 10 497765 | 22 |
| 39 | 9 457162 | 9 981399 | 9 475763 | 10 524237 | 9 481731 | 9 979059 | 9 502672 | 10 497328 | 21 |
| 40 | 9 457581 | 9 981361 | 9 476223 | 10 523777 | 9 482128 | 9 979010 | 9 503109 | 10 496991 | 20 |
| 41 | 9 458006 | 9 981323 | 9 476683 | 10 523317 | 9 482525 | 9 978979 | 9 503546 | 10 496451 | 19 |
| 42 | 9 458427 | 9 981285 | 9 477142 | 10 522858 | 9 482921 | 9 978930 | 9 503992 | 10 496018 | 18 |
| 43 | 9 458848 | 9 981247 | 9 477601 | 10 522399 | 9 483316 | 9 978898 | 9 504418 | 10 495522 | 17 |
| 44 | 9 459268 | 9 981209 | 9 478059 | 10 521941 | 9 483712 | 9 978858 | 9 504854 | 10 495146 | 16 |
| 45 | 9 459688 | 9 981171 | 9 478517 | 10 521483 | 9 484107 | 9 978817 | 9 505289 | 10 494711 | 15 |
| 46 | 9 460108 | 9 981133 | 9 478975 | 10 521025 | 9 484501 | 9 978777 | 9 505724 | 10 494276 | 14 |
| 47 | 9 460527 | 9 981095 | 9 479432 | 10 520568 | 9 484895 | 9 978737 | 9 506159 | 10 493841 | 13 |
| 48 | 9 460946 | 9 981057 | 9 479880 | 10 520111 | 9 485289 | 9 978696 | 9 506593 | 10 493407 | 12 |
| 49 | 9 461364 | 9 981019 | 9 480345 | 10 519655 | 9 485682 | 9 978655 | 9 507097 | 10 492973 | 11 |
| 50 | 9 461782 | 9 980981 | 9 480801 | 10 519199 | 9 486075 | 9 978615 | 9 507460 | 10 492540 | 10 |
| 51 | 9 462199 | 9 980942 | 9 481257 | 10 518743 | 9 486467 | 9 978574 | 9 507893 | 10 492107 | 9 |
| 52 | 9 462616 | 9 980904 | 9 481712 | 10 518258 | 9 486860 | 9 978533 | 9 508326 | 10 491674 | 8 |
| 53 | 9 463032 | 9 980866 | 9 482167 | 10 517833 | 9 487251 | 9 978493 | 9 508759 | 10 491241 | 7 |
| 54 | 9 463448 | 9 980827 | 9 482621 | 10 517379 | 9 487643 | 9 978452 | 9 509191 | 10 490809 | 6 |
| 55 | 9 463864 | 9 980789 | 9 483073 | 10 516023 | 9 488034 | 9 978411 | 9 509622 | 10 490378 | 5 |
| 56 | 9 464279 | 9 980750 | 9 483529 | 10 516471 | 9 488424 | 9 978370 | 9 510054 | 10 489946 | 4 |
| 57 | 9 464694 | 9 980712 | 9 483982 | 10 516018 | 9 488814 | 9 978329 | 9 510485 | 10 489515 | 3 |
| 58 | 9 465108 | 9 980673 | 9 484435 | 10 515505 | 9 489204 | 9 978288 | 9 510916 | 10 489042 | 2 |
| 59 | 9 465522 | 9 980635 | 9 484887 | 10 515113 | 9 489593 | 9 978247 | 9 511346 | 10 488654 | 1 |
| 60 | 9 465935 | 9 980696 | 9 485339 | 10 514661 | 9 489982 | 9 978206 | 9 511776 | 10 488224 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | / |

| 18° | | | | 19° | | | | |
|-----|----------|----------|----------|-----------|----------|----------|----------|-----------|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. |
| 0 | 9.489982 | 9.978206 | 9.511776 | 10.488224 | 9.512642 | 9.975670 | 9.536972 | 10.463028 |
| 1 | 9.490371 | 9.978165 | 9.512206 | 10.487794 | 9.513009 | 9.975627 | 9.537382 | 10.462618 |
| 2 | 9.490759 | 9.978124 | 9.512635 | 10.487365 | 9.513875 | 9.975588 | 9.537792 | 10.462208 |
| 3 | 9.491147 | 9.978083 | 9.513064 | 10.486936 | 9.513741 | 9.975539 | 9.538202 | 10.461798 |
| 4 | 9.491535 | 9.978042 | 9.513493 | 10.486507 | 9.514107 | 9.975476 | 9.538611 | 10.461389 |
| 5 | 9.491922 | 9.978001 | 9.513921 | 10.486079 | 9.514472 | 9.975452 | 9.539020 | 10.460980 |
| 6 | 9.490308 | 9.977959 | 9.514349 | 10.485651 | 9.514837 | 9.975408 | 9.539429 | 10.460571 |
| 7 | 9.492895 | 9.977918 | 9.514777 | 10.485223 | 9.515202 | 9.975365 | 9.539837 | 10.460168 |
| 8 | 9.493081 | 9.977877 | 9.515204 | 10.484796 | 9.515566 | 9.975321 | 9.540245 | 10.459755 |
| 9 | 9.493466 | 9.977835 | 9.515631 | 10.484369 | 9.515930 | 9.975277 | 9.540653 | 10.459347 |
| 10 | 9.493851 | 9.977794 | 9.516057 | 10.483943 | 9.516294 | 9.975233 | 9.541061 | 10.458939 |
| 11 | 9.494236 | 9.977752 | 9.516484 | 10.483516 | 9.516657 | 9.973189 | 9.541468 | 10.458532 |
| 12 | 9.494621 | 9.977711 | 9.516910 | 10.483090 | 9.517020 | 9.975140 | 9.541875 | 10.458125 |
| 13 | 9.495005 | 9.977669 | 9.517335 | 10.482665 | 9.517382 | 9.975101 | 9.542281 | 10.457719 |
| 14 | 9.495388 | 9.977628 | 9.517761 | 10.482239 | 9.517745 | 9.975057 | 9.542688 | 10.457312 |
| 15 | 9.495773 | 9.977586 | 9.518186 | 10.481814 | 9.518107 | 9.975013 | 9.543094 | 10.456906 |
| 16 | 9.496154 | 9.977544 | 9.518610 | 10.481390 | 9.518468 | 9.974969 | 9.543499 | 10.456501 |
| 17 | 9.496537 | 9.977503 | 9.519034 | 10.480966 | 9.518829 | 9.974925 | 9.543905 | 10.456095 |
| 18 | 9.496919 | 9.977461 | 9.519458 | 10.480542 | 9.519190 | 9.974880 | 9.544310 | 10.455690 |
| 19 | 9.497301 | 9.977419 | 9.519882 | 10.480118 | 9.519551 | 9.974836 | 9.544715 | 10.455285 |
| 20 | 9.497682 | 9.977377 | 9.520305 | 10.479695 | 9.519911 | 9.974792 | 9.545119 | 10.454881 |
| 21 | 9.498064 | 9.977335 | 9.520728 | 10.479272 | 9.520271 | 9.974748 | 9.545524 | 10.454476 |
| 22 | 9.498444 | 9.977293 | 9.521151 | 10.478849 | 9.520631 | 9.974703 | 9.545928 | 10.454072 |
| 23 | 9.498825 | 9.977251 | 9.521573 | 10.478427 | 9.520990 | 9.974659 | 9.546331 | 10.453669 |
| 24 | 9.499204 | 9.977209 | 9.521995 | 10.478005 | 9.521349 | 9.974614 | 9.546735 | 10.453265 |
| 25 | 9.499584 | 9.977167 | 9.522417 | 10.477583 | 9.521707 | 9.974570 | 9.547138 | 10.452862 |
| 26 | 9.499963 | 9.977125 | 9.522838 | 10.477162 | 9.522066 | 9.974525 | 9.547540 | 10.452460 |
| 27 | 9.500342 | 9.977083 | 9.523259 | 10.476741 | 9.522424 | 9.974481 | 9.547943 | 10.452057 |
| 28 | 9.500721 | 9.977041 | 9.523680 | 10.476320 | 9.522781 | 9.974436 | 9.548345 | 10.451655 |
| 29 | 9.501099 | 9.976999 | 9.524100 | 10.475900 | 9.523138 | 9.974391 | 9.548747 | 10.451253 |
| 30 | 9.501476 | 9.976857 | 9.524520 | 10.475480 | 9.523495 | 9.974347 | 9.549149 | 10.450851 |
| 31 | 9.501854 | 9.976914 | 9.524940 | 10.475060 | 9.523852 | 9.974302 | 9.549550 | 10.450450 |
| 32 | 9.502231 | 9.976872 | 9.525360 | 10.474641 | 9.524208 | 9.974257 | 9.549951 | 10.450049 |
| 33 | 9.502607 | 9.976830 | 9.525778 | 10.474222 | 9.524564 | 9.974212 | 9.550382 | 10.449648 |
| 34 | 9.502984 | 9.976787 | 9.526197 | 10.473803 | 9.524920 | 9.974167 | 9.550752 | 10.449248 |
| 35 | 9.503360 | 9.976745 | 9.526615 | 10.473385 | 9.525275 | 9.974122 | 9.551153 | 10.448847 |
| 36 | 9.503735 | 9.976702 | 9.527033 | 10.472967 | 9.525630 | 9.974077 | 9.551552 | 10.448448 |
| 37 | 9.504110 | 9.976660 | 9.527451 | 10.472549 | 9.525984 | 9.974032 | 9.551952 | 10.448048 |
| 38 | 9.504485 | 9.976617 | 9.527868 | 10.472132 | 9.526339 | 9.973937 | 9.552351 | 10.447649 |
| 39 | 9.504860 | 9.976574 | 9.528285 | 10.471715 | 9.526693 | 9.973942 | 9.552750 | 10.447250 |
| 40 | 9.505234 | 9.976532 | 9.528702 | 10.471298 | 9.527046 | 9.973897 | 9.553149 | 10.446851 |
| 41 | 9.505608 | 9.976489 | 9.529119 | 10.470881 | 9.527400 | 9.973852 | 9.553548 | 10.446452 |
| 42 | 9.505981 | 9.976446 | 9.529535 | 10.470465 | 9.527753 | 9.973807 | 9.553946 | 10.446054 |
| 43 | 9.506354 | 9.976404 | 9.529951 | 10.470049 | 9.528105 | 9.973761 | 9.554344 | 10.445656 |
| 44 | 9.506727 | 9.976361 | 9.530366 | 10.469634 | 9.528458 | 9.973716 | 9.554741 | 10.445259 |
| 45 | 9.507099 | 9.976318 | 9.530781 | 10.469219 | 9.528810 | 9.973671 | 9.555139 | 10.444861 |
| 46 | 9.507471 | 9.976275 | 9.531196 | 10.468804 | 9.529161 | 9.973624 | 9.555536 | 10.444464 |
| 47 | 9.507843 | 9.976232 | 9.531611 | 10.468389 | 9.529513 | 9.973590 | 9.555933 | 10.444067 |
| 48 | 9.508214 | 9.976189 | 9.532025 | 10.467975 | 9.529864 | 9.973535 | 9.556329 | 10.443671 |
| 49 | 9.508585 | 9.976146 | 9.532439 | 10.467561 | 9.530215 | 9.973489 | 9.556725 | 10.443275 |
| 50 | 9.508856 | 9.976103 | 9.532853 | 10.467147 | 9.530565 | 9.973444 | 9.557121 | 10.442879 |
| 51 | 9.509326 | 9.976060 | 9.533266 | 10.466734 | 9.530915 | 9.973398 | 9.557517 | 10.442483 |
| 52 | 9.509696 | 9.976017 | 9.533679 | 10.466321 | 9.531265 | 9.973352 | 9.557913 | 10.442087 |
| 53 | 9.510065 | 9.975974 | 9.534092 | 10.465905 | 9.531614 | 9.973307 | 9.558308 | 10.441692 |
| 54 | 9.510434 | 9.975930 | 9.534504 | 10.465496 | 9.531963 | 9.973261 | 9.558703 | 10.441297 |
| 55 | 9.510803 | 9.975887 | 9.534916 | 10.465084 | 9.532312 | 9.973215 | 9.559097 | 10.440903 |
| 56 | 9.511172 | 9.975844 | 9.535328 | 10.464672 | 9.532661 | 9.973169 | 9.559491 | 10.440509 |
| 57 | 9.511540 | 9.975800 | 9.535739 | 10.464261 | 9.533009 | 9.973124 | 9.559885 | 10.440115 |
| 58 | 9.511907 | 9.975757 | 9.536150 | 10.463850 | 9.533357 | 9.973078 | 9.560279 | 10.439721 |
| 59 | 9.512275 | 9.975714 | 9.536561 | 10.463439 | 9.533704 | 9.973032 | 9.560673 | 10.439927 |
| 60 | 9.512642 | 9.975670 | 9.536972 | 10.463028 | 9.534052 | 9.972986 | 9.561066 | 10.438934 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. |

| 20° | | | | 21° | | | | |
|-----|----------|----------|----------|-----------|----------|----------|----------|-----------|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. |
| 0 | 9.534062 | 9.972986 | 9.561066 | 10.438934 | 9.554329 | 9.970152 | 9.584177 | 10.415823 |
| 1 | 9.534399 | 9.972940 | 9.561459 | 10.438541 | 9.554658 | 9.970103 | 9.584555 | 10.415445 |
| 2 | 9.534745 | 9.972894 | 9.561851 | 10.438149 | 9.554987 | 9.970055 | 9.584932 | 10.415068 |
| 3 | 9.535092 | 9.972848 | 9.562244 | 10.437756 | 9.555315 | 9.970006 | 9.585309 | 10.414691 |
| 4 | 9.535438 | 9.972802 | 9.562636 | 10.437364 | 9.555643 | 9.969957 | 9.585686 | 10.414314 |
| 5 | 9.535783 | 9.972755 | 9.563028 | 10.436972 | 9.555971 | 9.969909 | 9.586062 | 10.413938 |
| 6 | 9.536129 | 9.972709 | 9.563419 | 10.436581 | 9.556299 | 9.969860 | 9.586439 | 10.413561 |
| 7 | 9.536474 | 9.972663 | 9.563811 | 10.436189 | 9.556626 | 9.969811 | 9.586815 | 10.413185 |
| 8 | 9.536818 | 9.972667 | 9.564202 | 10.435798 | 9.556953 | 9.969762 | 9.587190 | 10.412819 |
| 9 | 9.537163 | 9.972570 | 9.564593 | 10.435407 | 9.557280 | 9.969714 | 9.587566 | 10.412434 |
| 10 | 9.537507 | 9.972524 | 9.564983 | 10.435017 | 9.557606 | 9.969665 | 9.587941 | 10.412059 |
| 11 | 9.537851 | 9.972478 | 9.565373 | 10.434627 | 9.557932 | 9.969616 | 9.588316 | 10.411684 |
| 12 | 9.538194 | 9.972431 | 9.565763 | 10.434237 | 9.558258 | 9.969567 | 9.588691 | 10.411309 |
| 13 | 9.538536 | 9.972385 | 9.566153 | 10.433847 | 9.558583 | 9.969518 | 9.589066 | 10.410934 |
| 14 | 9.538880 | 9.972338 | 9.566542 | 10.433458 | 9.558909 | 9.969469 | 9.589440 | 10.410560 |
| 15 | 9.539223 | 9.972291 | 9.566932 | 10.433063 | 9.559234 | 9.969420 | 9.589814 | 10.410186 |
| 16 | 9.539565 | 9.972245 | 9.567320 | 10.432680 | 9.559558 | 9.969370 | 9.590188 | 10.409812 |
| 17 | 9.539907 | 9.972198 | 9.567709 | 10.432291 | 9.559883 | 9.969321 | 9.590562 | 10.409438 |
| 18 | 9.540249 | 9.972151 | 9.568098 | 10.431902 | 9.560207 | 9.969272 | 9.590935 | 10.409065 |
| 19 | 9.540590 | 9.972105 | 9.568486 | 10.431514 | 9.560531 | 9.969223 | 9.591308 | 10.408692 |
| 20 | 9.540931 | 9.972058 | 9.568873 | 10.431127 | 9.560855 | 9.969173 | 9.591681 | 10.408319 |
| 21 | 9.541272 | 9.972011 | 9.569261 | 10.430739 | 9.561178 | 9.969124 | 9.592054 | 10.407946 |
| 22 | 9.541613 | 9.971964 | 9.569648 | 10.430352 | 9.561501 | 9.969075 | 9.592426 | 10.407574 |
| 23 | 9.541953 | 9.971917 | 9.570005 | 10.429695 | 9.561824 | 9.969025 | 9.592799 | 10.407201 |
| 24 | 9.542293 | 9.971870 | 9.570422 | 10.429578 | 9.562146 | 9.968976 | 9.593171 | 10.406829 |
| 25 | 9.542633 | 9.971823 | 9.570809 | 10.429191 | 9.562468 | 9.968996 | 9.593542 | 10.406458 |
| 26 | 9.542971 | 9.971776 | 9.571195 | 10.428805 | 9.562790 | 9.968877 | 9.593914 | 10.406086 |
| 27 | 9.543310 | 9.971729 | 9.571581 | 10.428419 | 9.563112 | 9.968827 | 9.594285 | 10.405715 |
| 28 | 9.543649 | 9.971682 | 9.571967 | 10.428033 | 9.563433 | 9.968777 | 9.594656 | 10.405344 |
| 29 | 9.543987 | 9.971635 | 9.572352 | 10.427648 | 9.563755 | 9.968728 | 9.595027 | 10.404973 |
| 30 | 9.544325 | 9.971588 | 9.572738 | 10.427262 | 9.564075 | 9.968678 | 9.595398 | 10.404602 |
| 31 | 9.544663 | 9.971540 | 9.573123 | 10.426877 | 9.564498 | 9.968828 | 9.595768 | 10.404232 |
| 32 | 9.545000 | 9.971493 | 9.573507 | 10.426493 | 9.564716 | 9.968578 | 9.596138 | 10.403862 |
| 33 | 9.545338 | 9.971446 | 9.573892 | 10.426108 | 9.565036 | 9.968528 | 9.596508 | 10.403492 |
| 34 | 9.545674 | 9.971398 | 9.574276 | 10.425724 | 9.565356 | 9.968479 | 9.596878 | 10.403122 |
| 35 | 9.546011 | 9.971351 | 9.574660 | 10.425340 | 9.565676 | 9.968429 | 9.597247 | 10.402753 |
| 36 | 9.546347 | 9.971303 | 9.575044 | 10.424956 | 9.565995 | 9.968379 | 9.597616 | 10.402384 |
| 37 | 9.546683 | 9.971256 | 9.575427 | 10.424573 | 9.566314 | 9.968329 | 9.597985 | 10.402015 |
| 38 | 9.547019 | 9.971208 | 9.575810 | 10.424190 | 9.566632 | 9.968278 | 9.598354 | 10.401646 |
| 39 | 9.547354 | 9.971161 | 9.576193 | 10.423807 | 9.566951 | 9.968228 | 9.598722 | 10.401278 |
| 40 | 9.547689 | 9.971113 | 9.576576 | 10.423424 | 9.567269 | 9.968178 | 9.599091 | 10.400909 |
| 41 | 9.548024 | 9.971066 | 9.576959 | 10.423041 | 9.567587 | 9.968128 | 9.599459 | 10.400541 |
| 42 | 9.548359 | 9.971018 | 9.577341 | 10.422659 | 9.567904 | 9.968078 | 9.599827 | 10.400173 |
| 43 | 9.548683 | 9.970970 | 9.577723 | 10.422277 | 9.568222 | 9.968027 | 9.600194 | 10.399806 |
| 44 | 9.549027 | 9.970922 | 9.578104 | 10.421896 | 9.568539 | 9.967977 | 9.600562 | 10.399448 |
| 45 | 9.549360 | 9.970874 | 9.578486 | 10.421514 | 9.568856 | 9.967927 | 9.600929 | 10.399071 |
| 46 | 9.549693 | 9.970827 | 9.578867 | 10.421133 | 9.569172 | 9.967876 | 9.601296 | 10.398704 |
| 47 | 9.550026 | 9.970779 | 9.579248 | 10.420752 | 9.569488 | 9.967826 | 9.601663 | 10.398337 |
| 48 | 9.550359 | 9.970731 | 9.579629 | 10.420371 | 9.569804 | 9.967775 | 9.602029 | 10.397971 |
| 49 | 9.550692 | 9.970683 | 9.580009 | 10.419991 | 9.570120 | 9.967725 | 9.602395 | 10.397605 |
| 50 | 9.551024 | 9.970635 | 9.580389 | 10.419611 | 9.570435 | 9.967674 | 9.602761 | 10.397239 |
| 51 | 9.551356 | 9.970586 | 9.580769 | 10.419231 | 9.570751 | 9.967624 | 9.603127 | 10.396873 |
| 52 | 9.551687 | 9.970538 | 9.581149 | 10.418851 | 9.571068 | 9.967573 | 9.603493 | 10.396507 |
| 53 | 9.552018 | 9.970490 | 9.581528 | 10.418472 | 9.571380 | 9.967522 | 9.603858 | 10.396142 |
| 54 | 9.552349 | 9.970442 | 9.581907 | 10.418093 | 9.571695 | 9.967471 | 9.604223 | 10.395777 |
| 55 | 9.552680 | 9.970394 | 9.582286 | 10.417714 | 9.572009 | 9.967421 | 9.604588 | 10.395412 |
| 56 | 9.553016 | 9.970345 | 9.582665 | 10.417335 | 9.572323 | 9.967370 | 9.604953 | 10.395047 |
| 57 | 9.553341 | 9.970297 | 9.583044 | 10.416956 | 9.572636 | 9.967319 | 9.605317 | 10.394683 |
| 58 | 9.553670 | 9.970249 | 9.583422 | 10.416578 | 9.572950 | 9.967268 | 9.605662 | 10.394318 |
| 59 | 9.554000 | 9.970200 | 9.583800 | 10.416200 | 9.573263 | 9.967217 | 9.606046 | 10.393964 |
| 60 | 9.554329 | 9.970152 | 9.584177 | 10.415823 | 9.573575 | 9.967166 | 9.606410 | 10.393590 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. |

22°

| | Sine | Cosine | Tan. | Cotan. |
|----|----------|----------|----------|-----------|
| 0 | 9.573575 | 9.967166 | 9.606410 | 10.393590 |
| 1 | 9.573888 | 9.967115 | 9.606773 | 10.393227 |
| 2 | 9.574200 | 9.967064 | 9.607137 | 10.392863 |
| 3 | 9.574512 | 9.967013 | 9.607500 | 10.392500 |
| 4 | 9.574824 | 9.966961 | 9.607863 | 10.392137 |
| 5 | 9.575136 | 9.966910 | 9.608225 | 10.391775 |
| 6 | 9.575447 | 9.966859 | 9.608588 | 10.391412 |
| 7 | 9.575758 | 9.966808 | 9.608950 | 10.391050 |
| 8 | 9.576069 | 9.966756 | 9.609312 | 10.390688 |
| 9 | 9.576379 | 9.966708 | 9.609674 | 10.390326 |
| 10 | 9.576689 | 9.966653 | 9.610036 | 10.389964 |
| 11 | 9.576999 | 9.966602 | 9.610397 | 10.389603 |
| 12 | 9.577309 | 9.966550 | 9.610759 | 10.389241 |

23°

| | Sine | Cosine | Tan. | Cotan. |
|----|----------|----------|----------|-----------|
| | 9.591878 | 9.964026 | 9.627857 | 10.372148 |
| 1 | 9.592176 | 9.963972 | 9.628203 | 10.371797 |
| 2 | 9.592473 | 9.963919 | 9.628554 | 10.371446 |
| 3 | 9.592770 | 9.963865 | 9.628905 | 10.371095 |
| 4 | 9.593067 | 9.963811 | 9.629255 | 10.370745 |
| 5 | 9.593363 | 9.963757 | 9.629606 | 10.370394 |
| 6 | 9.593659 | 9.963704 | 9.629956 | 10.370044 |
| 7 | 9.593955 | 9.963650 | 9.630306 | 10.369694 |
| 8 | 9.594251 | 9.963596 | 9.630656 | 10.369344 |
| 9 | 9.594547 | 9.963542 | 9.631005 | 10.368995 |
| 10 | 9.594842 | 9.963488 | 9.631355 | 10.368645 |
| 11 | 9.595137 | 9.963434 | 9.631704 | 10.368296 |
| 12 | 9.595432 | 9.963379 | 9.632053 | 10.367947 |

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|----|----------|----------|----------|-----------|
| 13 | 9.577618 | 9.966499 | 9.611120 | 10.388880 |
| 14 | 9.577927 | 9.966447 | 9.611480 | 10.388520 |
| 15 | 9.578236 | 9.966395 | 9.611841 | 10.388193 |
| 16 | 9.578545 | 9.966344 | 9.612201 | 10.387799 |
| 17 | 9.578853 | 9.966292 | 9.612561 | 10.387439 |
| 18 | 9.579162 | 9.966240 | 9.612921 | 10.387079 |
| 19 | 9.579470 | 9.966188 | 9.613281 | 10.386719 |
| 20 | 9.579777 | 9.966136 | 9.613641 | 10.386359 |
| 21 | 9.580085 | 9.966085 | 9.614000 | 10.386000 |
| 22 | 9.580392 | 9.966033 | 9.614359 | 10.385641 |
| 23 | 9.580699 | 9.965981 | 9.614718 | 10.385282 |
| 24 | 9.581005 | 9.965929 | 9.615077 | 10.384923 |

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|----|----------|----------|----------|-----------|
| | 9.595727 | 9.963325 | 9.632402 | 10.367598 |
| 1 | 9.596021 | 9.963271 | 9.632750 | 10.367250 |
| 2 | 9.596315 | 9.963217 | 9.633099 | 10.366901 |
| 3 | 9.596609 | 9.963163 | 9.633447 | 10.366553 |
| 4 | 9.596903 | 9.963108 | 9.633795 | 10.366205 |
| 5 | 9.597196 | 9.963054 | 9.634143 | 10.365857 |
| 6 | 9.597490 | 9.962999 | 9.634490 | 10.365150 |
| 7 | 9.597783 | 9.962945 | 9.634888 | 10.365162 |
| 8 | 9.598075 | 9.962890 | 9.635185 | 10.364815 |
| 9 | 9.598368 | 9.962836 | 9.635532 | 10.364468 |
| 10 | 9.598660 | 9.962781 | 9.635879 | 10.364121 |
| 11 | 9.598952 | 9.962727 | 9.636226 | 10.363774 |

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|----|----------|----------|----------|-----------|
| 25 | 9.581812 | 9.965876 | 9.615435 | 10.384565 |
| 26 | 9.581618 | 9.965824 | 9.615793 | 10.384207 |
| 27 | 9.581924 | 9.965772 | 9.616151 | 10.383849 |
| 28 | 9.582229 | 9.965720 | 9.616509 | 10.383491 |
| 29 | 9.582535 | 9.965668 | 9.616867 | 10.383133 |
| 30 | 9.582840 | 9.965615 | 9.617224 | 10.382776 |
| 31 | 9.583145 | 9.965563 | 9.617582 | 10.382418 |
| 32 | 9.583449 | 9.965511 | 9.617939 | 10.382061 |
| 33 | 9.583754 | 9.965458 | 9.618295 | 10.381705 |
| 34 | 9.584058 | 9.965406 | 9.618652 | 10.381348 |
| 35 | 9.584361 | 9.965353 | 9.619008 | 10.380992 |
| 36 | 9.584665 | 9.965301 | 9.619364 | 10.380636 |

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|----|----------|----------|----------|-----------|
| | 9.599244 | 9.962672 | 9.636572 | 10.363428 |
| 1 | 9.599536 | 9.962617 | 9.636919 | 10.363081 |
| 2 | 9.599827 | 9.962562 | 9.637265 | 10.362735 |
| 3 | 9.600118 | 9.962508 | 9.637611 | 10.362389 |
| 4 | 9.600409 | 9.962453 | 9.637956 | 10.362044 |
| 5 | 9.600700 | 9.962398 | 9.638302 | 10.361698 |
| 6 | 9.600990 | 9.962343 | 9.638647 | 10.361355 |
| 7 | 9.601280 | 9.962288 | 9.638992 | 10.361008 |
| 8 | 9.601570 | 9.962233 | 9.639337 | 10.360663 |
| 9 | 9.601860 | 9.962178 | 9.639682 | 10.360318 |
| 10 | 9.602150 | 9.962123 | 9.640027 | 10.359973 |
| 11 | 9.602439 | 9.962067 | 9.640371 | 10.359629 |

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|----|----------|----------|----------|-----------|
| 37 | 9.584968 | 9.965248 | 9.619720 | 10.380280 |
| 38 | 9.585272 | 9.965195 | 9.620076 | 10.379924 |
| 39 | 9.585574 | 9.965143 | 9.620423 | 10.379568 |
| 40 | 9.585877 | 9.965090 | 9.620787 | 10.379213 |
| 41 | 9.586179 | 9.965037 | 9.621142 | 10.378858 |
| 42 | 9.586482 | 9.964984 | 9.621497 | 10.378503 |
| 43 | 9.586783 | 9.964931 | 9.621852 | 10.378148 |
| 44 | 9.587085 | 9.964879 | 9.622207 | 10.377793 |
| 45 | 9.587386 | 9.964826 | 9.622561 | 10.377439 |
| 46 | 9.587688 | 9.964773 | 9.622915 | 10.377085 |
| 47 | 9.587989 | 9.964720 | 9.623269 | 10.376731 |
| 48 | 9.588289 | 9.964666 | 9.623623 | 10.376377 |

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|----|----------|----------|----------|-----------|
| | 9.602728 | 9.962012 | 9.640716 | 10.359284 |
| 1 | 9.603017 | 9.961957 | 9.641060 | 10.358940 |
| 2 | 9.603305 | 9.961902 | 9.641404 | 10.358696 |
| 3 | 9.603594 | 9.961846 | 9.641747 | 10.358253 |
| 4 | 9.603882 | 9.961791 | 9.642091 | 10.357909 |
| 5 | 9.604170 | 9.961733 | 9.642434 | 10.357566 |
| 6 | 9.604457 | 9.961680 | 9.642777 | 10.357223 |
| 7 | 9.604745 | 9.961624 | 9.643120 | 10.356880 |
| 8 | 9.605032 | 9.961569 | 9.643463 | 10.356537 |
| 9 | 9.605319 | 9.961513 | 9.643806 | 10.356194 |
| 10 | 9.605606 | 9.961458 | 9.644148 | 10.355852 |
| 11 | 9.605892 | 9.961402 | 9.644490 | 10.355510 |

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|----|----------|----------|----------|-----------|
| 49 | 9.588590 | 9.964613 | 9.623976 | 10.376024 |
| 50 | 9.588890 | 9.964560 | 9.624330 | 10.375670 |
| 51 | 9.589190 | 9.964507 | 9.624683 | 10.375317 |
| 52 | 9.589489 | 9.964454 | 9.625036 | 10.374964 |
| 53 | 9.589789 | 9.964400 | 9.625388 | 10.374612 |
| 54 | 9.590088 | 9.964347 | 9.625741 | 10.374259 |
| 55 | 9.590387 | 9.964294 | 9.626093 | 10.373907 |
| 56 | 9.590686 | 9.964240 | 9.626445 | 10.373553 |
| 57 | 9.590984 | 9.964187 | 9.626797 | 10.373203 |
| 58 | 9.591282 | 9.964133 | 9.627149 | 10.372851 |
| 59 | 9.591580 | 9.964080 | 9.627501 | 10.372499 |
| 60 | 9.591878 | 9.964026 | 9.627852 | 10.372148 |

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|----|----------|----------|----------|-----------|
| | 9.606179 | 9.961346 | 9.644832 | 10.355168 |
| 1 | 9.606165 | 9.961290 | 9.645174 | 10.354826 |
| 2 | 9.606751 | 9.961235 | 9.645516 | 10.354484 |
| 3 | 9.607036 | 9.961179 | 9.645857 | 10.354143 |
| 4 | 9.607322 | 9.961123 | 9.646199 | 10.353801 |
| 5 | 9.607607 | 9.961067 | 9.646540 | 10.353460 |
| 6 | 9.607892 | 9.961011 | 9.646881 | 10.353119 |
| 7 | 9.608177 | 9.960955 | 9.647222 | 10.352778 |
| 8 | 9.608461 | 9.960899 | 9.647562 | 10.352438 |
| 9 | 9.608745 | 9.960843 | 9.647903 | 10.352097 |
| 10 | 9.609029 | 9.960786 | 9.648243 | 10.351787 |
| 11 | 9.609313 | 9.960730 | 9.648583 | 10.351417 |

67°

66°

| 24° | | | | 25° | | | | | |
|-----|----------|----------|----------|-----------|----------|----------|----------|-----------|--------|
| | Sine | Cosine | Tan. | | Sine | Cosine | Tan. | | Cotan. |
| 0 | 9.609313 | 9.960730 | 9.648583 | 10.351417 | 9.625948 | 9.957276 | 9.668673 | 10.331327 | 60 |
| 1 | 9.609597 | 9.960674 | 9.648923 | 10.351077 | 9.626219 | 9.957217 | 9.669002 | 10.330998 | 59 |
| 2 | 9.609880 | 9.960618 | 9.649990 | 10.350737 | 9.626490 | 9.957158 | 9.669332 | 10.330663 | 58 |
| 3 | 9.610164 | 9.960561 | 9.649602 | 10.350398 | 9.626760 | 9.957099 | 9.669661 | 10.330339 | 57 |
| 4 | 9.610447 | 9.960505 | 9.649942 | 10.350058 | 9.627030 | 9.957040 | 9.669991 | 10.330009 | 56 |
| 5 | 9.610729 | 9.960448 | 9.650281 | 10.349719 | 9.627300 | 9.956981 | 9.670320 | 10.329680 | 55 |
| 6 | 9.611012 | 9.960392 | 9.650620 | 10.349380 | 9.627570 | 9.956921 | 9.670649 | 10.329351 | 54 |
| 7 | 9.611294 | 9.960335 | 9.650959 | 10.349041 | 9.627840 | 9.956862 | 9.670977 | 10.328023 | 53 |
| 8 | 9.611576 | 9.960279 | 9.651297 | 10.348703 | 9.628109 | 9.956803 | 9.671306 | 10.328694 | 52 |
| 9 | 9.611858 | 9.960222 | 9.651636 | 10.348364 | 9.628378 | 9.956744 | 9.671635 | 10.328365 | 51 |
| 10 | 9.612140 | 9.960165 | 9.651974 | 10.348026 | 9.628647 | 9.956684 | 9.671963 | 10.328037 | 50 |
| 11 | 9.612421 | 9.960109 | 9.652312 | 10.347688 | 9.628916 | 9.956625 | 9.672291 | 10.327709 | 49 |
| 12 | 9.612702 | 9.960052 | 9.652650 | 10.347350 | 9.629185 | 9.956566 | 9.672619 | 10.327381 | 48 |
| 13 | 9.612983 | 9.959995 | 9.652988 | 10.347012 | 9.629453 | 9.956506 | 9.672947 | 10.327053 | 47 |
| 14 | 9.613264 | 9.959938 | 9.653326 | 10.346674 | 9.629721 | 9.956447 | 9.673274 | 10.326726 | 46 |
| 15 | 9.613545 | 9.959882 | 9.653663 | 10.346337 | 9.629983 | 9.956387 | 9.673602 | 10.326398 | 45 |
| 16 | 9.613825 | 9.959825 | 9.654000 | 10.346000 | 9.630257 | 9.956327 | 9.673929 | 10.326071 | 44 |
| 17 | 9.614105 | 9.959768 | 9.654337 | 10.345663 | 9.630524 | 9.956268 | 9.674257 | 10.325743 | 43 |
| 18 | 9.614385 | 9.959711 | 9.654674 | 10.345326 | 9.630792 | 9.956208 | 9.674584 | 10.325416 | 42 |
| 19 | 9.614665 | 9.959654 | 9.655011 | 10.344989 | 9.631059 | 9.956148 | 9.674911 | 10.325089 | 41 |
| 20 | 9.614944 | 9.959596 | 9.655348 | 10.344652 | 9.631326 | 9.956089 | 9.675237 | 10.324763 | 40 |
| 21 | 9.615223 | 9.959539 | 9.655684 | 10.344316 | 9.631593 | 9.956029 | 9.675564 | 10.324436 | 39 |
| 22 | 9.615502 | 9.959489 | 9.656020 | 10.343980 | 9.631859 | 9.955969 | 9.675890 | 10.324110 | 38 |
| 23 | 9.615781 | 9.959425 | 9.656356 | 10.343644 | 9.632125 | 9.955909 | 9.676217 | 10.323783 | 37 |
| 24 | 9.616060 | 9.959368 | 9.656692 | 10.343308 | 9.632392 | 9.955849 | 9.676543 | 10.323457 | 36 |
| 25 | 9.616338 | 9.959310 | 9.657028 | 10.342972 | 9.632658 | 9.955789 | 9.676869 | 10.323131 | 35 |
| 26 | 9.616616 | 9.959253 | 9.657364 | 10.342636 | 9.632923 | 9.955729 | 9.677194 | 10.322806 | 34 |
| 27 | 9.616894 | 9.959195 | 9.657699 | 10.342301 | 9.633189 | 9.955669 | 9.677520 | 10.322480 | 33 |
| 28 | 9.617172 | 9.959138 | 9.658034 | 10.341966 | 9.633454 | 9.955609 | 9.677846 | 10.322154 | 32 |
| 29 | 9.617450 | 9.959080 | 9.658363 | 10.341631 | 9.633719 | 9.955548 | 9.678171 | 10.321829 | 31 |
| 30 | 9.617727 | 9.959023 | 9.658704 | 10.341296 | 9.633984 | 9.955488 | 9.678496 | 10.321504 | 30 |
| 31 | 9.618004 | 9.958965 | 9.659039 | 10.340961 | 9.634249 | 9.955428 | 9.678821 | 10.321179 | 29 |
| 32 | 9.618281 | 9.958908 | 9.659373 | 10.340327 | 9.634514 | 9.955368 | 9.679146 | 10.320854 | 28 |
| 33 | 9.618558 | 9.958850 | 9.659708 | 10.340292 | 9.634778 | 9.955307 | 9.679471 | 10.320529 | 27 |
| 34 | 9.618834 | 9.958792 | 9.660042 | 10.339958 | 9.635042 | 9.955247 | 9.679795 | 10.320205 | 26 |
| 35 | 9.619110 | 9.958734 | 9.660376 | 10.339624 | 9.635306 | 9.955186 | 9.680120 | 10.319880 | 25 |
| 36 | 9.619386 | 9.958677 | 9.660710 | 10.339296 | 9.635570 | 9.955126 | 9.680444 | 10.319556 | 24 |
| 37 | 9.619662 | 9.958619 | 9.661043 | 10.338957 | 9.635834 | 9.955065 | 9.680768 | 10.319232 | 23 |
| 38 | 9.619938 | 9.958561 | 9.661377 | 10.338623 | 9.636097 | 9.955005 | 9.681092 | 10.318908 | 22 |
| 39 | 9.620213 | 9.958503 | 9.661710 | 10.338290 | 9.636360 | 9.954944 | 9.681416 | 10.318584 | 21 |
| 40 | 9.620488 | 9.958445 | 9.662043 | 10.337957 | 9.636623 | 9.954883 | 9.681740 | 10.318260 | 20 |
| 41 | 9.620763 | 9.958887 | 9.662376 | 10.337624 | 9.636886 | 9.954823 | 9.682063 | 10.317937 | 19 |
| 42 | 9.621038 | 9.958329 | 9.662709 | 10.337291 | 9.637148 | 9.954762 | 9.682387 | 10.317613 | 18 |
| 43 | 9.621313 | 9.958271 | 9.663042 | 10.336958 | 9.637411 | 9.954701 | 9.682710 | 10.317290 | 17 |
| 44 | 9.621597 | 9.958213 | 9.663375 | 10.336623 | 9.637673 | 9.954640 | 9.683033 | 10.316967 | 16 |
| 45 | 9.621861 | 9.958154 | 9.663707 | 10.336293 | 9.637935 | 9.954579 | 9.683356 | 10.316644 | 15 |
| 46 | 9.622138 | 9.958096 | 9.664039 | 10.335961 | 9.638197 | 9.954518 | 9.683679 | 10.316321 | 14 |
| 47 | 9.622409 | 9.958038 | 9.664371 | 10.335629 | 9.638458 | 9.954457 | 9.684001 | 10.315999 | 13 |
| 48 | 9.622682 | 9.957979 | 9.664703 | 10.335297 | 9.638720 | 9.954396 | 9.684324 | 10.315676 | 12 |
| 49 | 9.622956 | 9.957921 | 9.665035 | 10.334965 | 9.638981 | 9.954334 | 9.684646 | 10.315354 | 11 |
| 50 | 9.623229 | 9.957863 | 9.665366 | 10.334634 | 9.639242 | 9.954275 | 9.684968 | 10.315032 | 10 |
| 51 | 9.633502 | 9.957804 | 9.665698 | 10.334302 | 9.639503 | 9.954213 | 9.685290 | 10.314710 | 9 |
| 52 | 9.622774 | 9.957746 | 9.666029 | 10.333971 | 9.639764 | 9.954152 | 9.685612 | 10.314388 | 8 |
| 53 | 9.624047 | 9.957687 | 9.666360 | 10.333640 | 9.640024 | 9.954090 | 9.685934 | 10.314066 | 7 |
| 54 | 9.624319 | 9.957628 | 9.666691 | 10.333309 | 9.640284 | 9.954029 | 9.686255 | 10.313748 | 6 |
| 55 | 9.624591 | 9.957570 | 9.667021 | 10.332979 | 9.640544 | 9.953968 | 9.686577 | 10.313423 | 5 |
| 56 | 9.624863 | 9.957511 | 9.667352 | 10.332648 | 9.640804 | 9.953906 | 9.686898 | 10.313102 | 4 |
| 57 | 9.625135 | 9.957452 | 9.667682 | 10.332318 | 9.641064 | 9.953845 | 9.687219 | 10.312781 | 3 |
| 58 | 9.625406 | 9.957393 | 9.668013 | 10.331987 | 9.641324 | 9.953783 | 9.687540 | 10.312460 | 2 |
| 59 | 9.625677 | 9.957335 | 9.668343 | 10.331657 | 9.641583 | 9.953722 | 9.687861 | 10.312139 | 1 |
| 60 | 9.625948 | 9.957276 | 9.668673 | 10.331327 | 9.641842 | 9.953660 | 9.688182 | 10.311818 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | / |

LOGARITHMIC SINES AND TANGENTS.

37

26°

27°

| | Sine | Cosine | Tan. | Cotan. | | Sine | Cosine | Tan. | Cotan. |
|----|----------|----------|----------|-----------|----------|----------|----------|-----------|--------|
| | Cosine | Sine | Cotan. | Tan. | | Cosine | Sine | Cotan. | Tan. |
| 0 | 9'641842 | 9'953660 | 9'688182 | 10'311818 | 9'657047 | 9'949881 | 9'707166 | 10'292884 | 60 |
| 1 | 9'642101 | 9'953599 | 9'688102 | 10'311498 | 9'657295 | 9'949816 | 9'707478 | 10'292522 | 59 |
| 2 | 9'642360 | 9'953537 | 9'688823 | 10'311177 | 9'657542 | 9'949752 | 9'707790 | 10'292210 | 58 |
| 3 | 9'642618 | 9'953475 | 9'689143 | 10'310857 | 9'657790 | 9'949688 | 9'708102 | 10'291898 | 57 |
| 4 | 9'642877 | 9'953413 | 9'689463 | 10'310537 | 9'658037 | 9'949623 | 9'708414 | 10'291586 | |
| 5 | 9'643135 | 9'953352 | 9'689783 | 10'310217 | 9'658284 | 9'949558 | 9'708720 | 10'291274 | 55 |
| 6 | 9'643393 | 9'953290 | 9'690103 | 10'309897 | 9'658531 | 9'949494 | 9'709037 | 10'290963 | 54 |
| 7 | 9'643650 | 9'953226 | 9'690423 | 10'309577 | 9'658778 | 9'949429 | 9'709349 | 10'290651 | 53 |
| 8 | 9'643908 | 9'953166 | 9'690742 | 10'309258 | 9'659025 | 9'949364 | 9'709660 | 10'290340 | 52 |
| 9 | 9'644165 | 9'953104 | 9'691042 | 10'308938 | 9'659271 | 9'949300 | 9'709971 | 10'290029 | 51 |
| 10 | 9'644423 | 9'953042 | 9'691381 | 10'308619 | 9'659517 | 9'949235 | 9'710282 | 10'289718 | 50 |
| 11 | 9'644680 | 9'952980 | 9'691700 | 10'308300 | 9'659763 | 9'949170 | 9'710593 | 10'289407 | |
| 12 | 9'644936 | 9'952918 | 9'692019 | 10'307981 | 9'660009 | 9'949105 | 9'710904 | 10'289096 | 48 |
| 13 | 9'645193 | 9'952855 | 9'692338 | 10'307662 | 9'660255 | 9'949040 | 9'711215 | 10'288785 | 47 |
| 14 | 9'645450 | 9'952793 | 9'692656 | 10'307344 | 9'660501 | 9'948075 | 9'711525 | 10'288475 | 46 |
| 15 | 9'645706 | 9'952731 | 9'692975 | 10'307025 | 9'660746 | 9'948910 | 9'711886 | 10'288164 | 45 |
| 16 | 9'645962 | 9'952669 | 9'693293 | 10'306707 | 9'660991 | 9'948845 | 9'712146 | 10'287854 | 44 |
| 17 | 9'646218 | 9'952606 | 9'693612 | 10'306388 | 9'661236 | 9'948780 | 9'712456 | 10'287544 | 43 |
| 18 | 9'646474 | 9'952544 | 9'693930 | 10'306070 | 9'661481 | 9'948715 | 9'712766 | 10'287234 | 42 |
| 19 | 9'646729 | 9'952481 | 9'694248 | 10'305752 | 9'661726 | 9'948650 | 9'713076 | 10'286924 | 41 |
| 20 | 9'646984 | 9'952419 | 9'694566 | 10'305434 | 9'661970 | 9'948584 | 9'713386 | 10'286614 | 40 |
| 21 | 9'647240 | 9'952356 | 9'694883 | 10'305117 | 9'662214 | 9'948519 | 9'713696 | 10'286304 | 39 |
| 22 | 9'647494 | 9'952294 | 9'695201 | 10'304799 | 9'662459 | 9'948454 | 9'714005 | 10'285955 | 38 |
| 23 | 9'647749 | 9'952231 | 9'695518 | 10'304482 | 9'662703 | 9'948388 | 9'714314 | 10'285666 | 37 |
| 24 | 9'648004 | 9'952168 | 9'695836 | 10'304164 | 9'662946 | 9'948323 | 9'714624 | 10'285370 | 36 |
| 25 | 9'648258 | 9'952106 | 9'696153 | 10'303847 | 9'663190 | 9'948257 | 9'714933 | 10'285067 | 35 |
| 26 | 9'648512 | 9'952043 | 9'696470 | 10'303530 | 9'663133 | 9'948192 | 9'715242 | 10'284758 | 34 |
| 27 | 9'648766 | 9'951980 | 9'696787 | 10'303213 | 9'663677 | 9'948126 | 9'715551 | 10'284449 | 33 |
| 28 | 9'649020 | 9'951917 | 9'697103 | 10'302897 | 9'663920 | 9'948060 | 9'715860 | 10'284140 | 32 |
| 29 | 9'649274 | 9'951854 | 9'697420 | 10'302580 | 9'664163 | 9'947995 | 9'716168 | 10'283832 | 31 |
| 30 | 9'649527 | 9'951791 | 9'697736 | 10'302264 | 9'664406 | 9'947929 | 9'716477 | 10'283523 | 30 |
| 31 | 9'649781 | 9'951728 | 9'698053 | 10'301947 | 9'664648 | 9'947863 | 9'716785 | 10'283215 | 29 |
| 32 | 9'650034 | 9'951665 | 9'698369 | 10'301631 | 9'664891 | 9'947797 | 9'717093 | 10'282907 | 28 |
| 33 | 9'650287 | 9'951602 | 9'698685 | 10'301315 | 9'665133 | 9'947731 | 9'717401 | 10'282599 | 27 |
| 34 | 9'650539 | 9'951539 | 9'699001 | 10'300999 | 9'665375 | 9'947666 | 9'717709 | 10'282291 | 26 |
| 35 | 9'650792 | 9'951476 | 9'699316 | 10'300684 | 9'665617 | 9'947600 | 9'718017 | 10'281983 | 25 |
| 36 | 9'651044 | 9'951412 | 9'699632 | 10'300368 | 9'665859 | 9'947533 | 9'718325 | 10'281675 | 24 |
| 37 | 9'651297 | 9'951349 | 9'699947 | 10'300053 | 9'666100 | 9'947467 | 9'718633 | 10'281367 | 23 |
| 38 | 9'651549 | 9'951286 | 9'700263 | 10'299737 | 9'666342 | 9'947401 | 9'718940 | 10'281060 | 22 |
| 39 | 9'651800 | 9'951222 | 9'700578 | 10'299422 | 9'666583 | 9'947335 | 9'719248 | 10'280752 | 21 |
| 40 | 9'652052 | 9'951159 | 9'700983 | 10'299107 | 9'666824 | 9'947269 | 9'719555 | 10'280445 | 20 |
| 41 | 9'652304 | 9'951096 | 9'701208 | 10'298792 | 9'667065 | 9'947203 | 9'719862 | 10'280138 | 19 |
| 42 | 9'652555 | 9'951032 | 9'701523 | 10'298477 | 9'667305 | 9'947136 | 9'720169 | 10'279881 | 18 |
| 43 | 9'652806 | 9'950968 | 9'701837 | 10'298163 | 9'667546 | 9'947070 | 9'720476 | 10'279524 | 17 |
| 44 | 9'653057 | 9'950905 | 9'702152 | 10'297848 | 9'667786 | 9'947004 | 9'720783 | 10'279217 | 16 |
| 45 | 9'653308 | 9'950841 | 9'702466 | 10'297534 | 9'668027 | 9'946937 | 9'721089 | 10'278911 | 15 |
| 46 | 9'653558 | 9'950778 | 9'702781 | 10'297219 | 9'668267 | 9'948171 | 9'721396 | 10'278604 | 14 |
| 47 | 9'653808 | 9'950714 | 9'703095 | 10'296905 | 9'668506 | 9'948004 | 9'721702 | 10'278298 | 13 |
| 48 | 9'654059 | 9'950650 | 9'703409 | 10'296591 | 9'668746 | 9'946738 | 9'722009 | 10'277991 | 12 |
| 49 | 9'654309 | 9'950586 | 9'703722 | 10'296278 | 9'668986 | 9'946671 | 9'722315 | 10'277685 | 11 |
| 50 | 9'654558 | 9'950522 | 9'704036 | 10'295964 | 9'669225 | 9'946604 | 9'722621 | 10'277379 | 10 |
| 51 | 9'654808 | 9'950458 | 9'704350 | 10'295650 | 9'669464 | 9'946538 | 9'722927 | 10'277073 | 9 |
| 52 | 9'655058 | 9'950394 | 9'704663 | 10'295337 | 9'669703 | 9'946471 | 9'723232 | 10'276768 | 8 |
| 53 | 9'655307 | 9'950330 | 9'704976 | 10'295024 | 9'669942 | 9'946404 | 9'723538 | 10'276462 | 7 |
| 54 | 9'655556 | 9'950266 | 9'705290 | 10'294710 | 9'670181 | 9'946337 | 9'723844 | 10'276156 | 6 |
| 55 | 9'655805 | 9'950202 | 9'705603 | 10'294397 | 9'670419 | 9'946270 | 9'724149 | 10'275851 | 5 |
| 56 | 9'656054 | 9'950138 | 9'705916 | 10'294084 | 9'670658 | 9'946203 | 9'724454 | 10'275546 | 4 |
| 57 | 9'656302 | 9'950074 | 9'706228 | 10'293772 | 9'670896 | 9'946136 | 9'724760 | 10'275240 | 3 |
| 58 | 9'656561 | 9'950010 | 9'706341 | 10'293459 | 9'671134 | 9'946069 | 9'725065 | 10'274985 | 2 |
| 59 | 9'656679 | 9'949945 | 9'706854 | 10'293146 | 9'671372 | 9'946002 | 9'725370 | 10'274630 | 1 |
| 60 | 9'657047 | 9'949881 | 9'707166 | 10'292834 | 9'671609 | 9'945935 | 9'725674 | 10'274326 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | |

63° 1' V ..

62°

| 28° | | | | | 29° | | | | |
|-----|----------|----------|----------|-----------|----------|-----------|----------|-----------|--------|
| | Sine | Cosine | Tan. | Cotan. | | Sine | Cosine | Tan. | Cotan. |
| 0 | 9.671609 | 9.945935 | 9.725074 | 10.274326 | 9.685571 | 9.941819 | 9.743752 | 10.256248 | 60 |
| 1 | 9.671847 | 9.945668 | 9.725979 | 10.274021 | 9.685799 | 9.941749 | 9.744050 | 10.255950 | 59 |
| 2 | 9.672084 | 9.945800 | 9.726284 | 10.273716 | 9.686027 | 9.9411679 | 9.744348 | 10.255652 | 58 |
| 3 | 9.672321 | 9.945733 | 9.726588 | 10.273412 | 9.686254 | 9.941609 | 9.744646 | 10.255355 | 57 |
| 4 | 9.672558 | 9.945666 | 9.726892 | 10.273108 | 9.686482 | 9.941589 | 9.744943 | 10.255057 | 56 |
| 5 | 9.672795 | 9.945598 | 9.727197 | 10.272803 | 9.686709 | 9.941409 | 9.745240 | 10.254760 | 55 |
| 6 | 9.673032 | 9.945531 | 9.727501 | 10.272499 | 9.686936 | 9.941398 | 9.745538 | 10.254462 | 54 |
| 7 | 9.673265 | 9.945104 | 9.727805 | 10.272195 | 9.687163 | 9.941328 | 9.745835 | 10.254165 | 53 |
| 8 | 9.673505 | 9.945396 | 9.728109 | 10.271891 | 9.687389 | 9.941258 | 9.746132 | 10.253863 | 52 |
| 9 | 9.673741 | 9.945288 | 9.728412 | 10.271588 | 9.687616 | 9.941187 | 9.746429 | 10.253571 | 51 |
| 10 | 9.673977 | 9.945201 | 9.728716 | 10.271284 | 9.687843 | 9.941117 | 9.746726 | 10.253274 | 50 |
| 11 | 9.674213 | 9.945193 | 9.729200 | 10.270980 | 9.688069 | 9.941046 | 9.747023 | 10.252977 | 49 |
| 12 | 9.674448 | 9.945125 | 9.729323 | 10.270677 | 9.688295 | 9.940975 | 9.747319 | 10.252681 | 48 |
| 13 | 9.674684 | 9.945058 | 9.729626 | 10.270374 | 9.688521 | 9.940905 | 9.747616 | 10.252384 | 47 |
| 14 | 9.674919 | 9.944990 | 9.729929 | 10.270071 | 9.688747 | 9.940834 | 9.747913 | 10.252087 | 46 |
| 15 | 9.675155 | 9.944922 | 9.730233 | 10.269767 | 9.689072 | 9.940763 | 9.748209 | 10.251791 | 45 |
| 16 | 9.675390 | 9.944854 | 9.730535 | 10.269465 | 9.689198 | 9.940693 | 9.748505 | 10.251495 | 44 |
| 17 | 9.675624 | 9.944786 | 9.730838 | 10.269162 | 9.689428 | 9.940622 | 9.748801 | 10.251199 | 43 |
| 18 | 9.675859 | 9.944718 | 9.731141 | 10.268859 | 9.689648 | 9.940551 | 9.749097 | 10.250903 | 42 |
| 19 | 9.676094 | 9.944650 | 9.731444 | 10.268556 | 9.689873 | 9.940480 | 9.749393 | 10.250607 | 41 |
| 20 | 9.676328 | 9.944682 | 9.731746 | 10.268254 | 9.690008 | 9.940409 | 9.749689 | 10.250311 | 40 |
| 21 | 9.676562 | 9.944514 | 9.732048 | 10.267952 | 9.690323 | 9.940338 | 9.749985 | 10.250015 | 39 |
| 22 | 9.676796 | 9.944446 | 9.732351 | 10.267649 | 9.690548 | 9.940207 | 9.750281 | 10.249719 | 38 |
| 23 | 9.677030 | 9.944377 | 9.732653 | 10.267347 | 9.690772 | 9.940196 | 9.750576 | 10.249424 | 37 |
| 24 | 9.677264 | 9.944309 | 9.732955 | 10.267045 | 9.690996 | 9.940125 | 9.750872 | 10.249128 | 36 |
| 25 | 9.677498 | 9.944241 | 9.733257 | 10.266743 | 9.691220 | 9.940054 | 9.751167 | 10.248533 | 35 |
| 26 | 9.677731 | 9.944172 | 9.733558 | 10.266442 | 9.691444 | 9.939982 | 9.751462 | 10.248538 | 34 |
| 27 | 9.677964 | 9.944104 | 9.733860 | 10.266140 | 9.691668 | 9.939910 | 9.751757 | 10.248243 | 33 |
| 28 | 9.678197 | 9.944036 | 9.734162 | 10.265858 | 9.691892 | 9.939840 | 9.752052 | 10.247948 | 32 |
| 29 | 9.678430 | 9.943967 | 9.734463 | 10.265557 | 9.692115 | 9.939768 | 9.752347 | 10.247653 | 31 |
| 30 | 9.678663 | 9.943899 | 9.734761 | 10.265236 | 9.692330 | 9.939697 | 9.752642 | 10.247358 | 30 |
| 31 | 9.678895 | 9.943830 | 9.735066 | 10.264934 | 9.692560 | 9.939625 | 9.752937 | 10.247063 | 29 |
| 32 | 9.679128 | 9.943761 | 9.735367 | 10.264633 | 9.692783 | 9.939554 | 9.753231 | 10.246769 | 28 |
| 33 | 9.679360 | 9.943693 | 9.735668 | 10.264332 | 9.693008 | 9.939482 | 9.753526 | 10.246474 | 27 |
| 34 | 9.679592 | 9.943624 | 9.735969 | 10.264031 | 9.693231 | 9.939410 | 9.753820 | 10.246180 | 26 |
| 35 | 9.679824 | 9.943555 | 9.736269 | 10.263731 | 9.693453 | 9.939339 | 9.754115 | 10.245585 | 25 |
| 36 | 9.680056 | 9.943486 | 9.736570 | 10.263430 | 9.693676 | 9.939267 | 9.754409 | 10.245591 | 24 |
| 37 | 9.680288 | 9.943417 | 9.736870 | 10.263130 | 9.693898 | 9.939195 | 9.754703 | 10.245297 | 23 |
| 38 | 9.680519 | 9.943348 | 9.737171 | 10.262829 | 9.694120 | 9.939123 | 9.754997 | 10.245003 | 22 |
| 39 | 9.680750 | 9.943279 | 9.737471 | 10.262529 | 9.694342 | 9.939052 | 9.755291 | 10.244709 | 21 |
| 40 | 9.680982 | 9.943210 | 9.737771 | 10.262229 | 9.694564 | 9.939080 | 9.755585 | 10.244415 | 20 |
| 41 | 9.681213 | 9.943141 | 9.738071 | 10.261929 | 9.694786 | 9.939008 | 9.755878 | 10.244122 | 19 |
| 42 | 9.681443 | 9.943072 | 9.738371 | 10.261629 | 9.695007 | 9.938836 | 9.756172 | 10.243828 | 18 |
| 43 | 9.681674 | 9.943003 | 9.738671 | 10.261329 | 9.695229 | 9.938763 | 9.756465 | 10.243535 | 17 |
| 44 | 9.681905 | 9.942934 | 9.738971 | 10.261029 | 9.695450 | 9.938691 | 9.756759 | 10.243241 | 16 |
| 45 | 9.682135 | 9.942864 | 9.739271 | 10.260729 | 9.695671 | 9.938619 | 9.757052 | 10.242948 | 15 |
| 46 | 9.682365 | 9.942795 | 9.739570 | 10.260430 | 9.695892 | 9.938547 | 9.757345 | 10.242655 | 14 |
| 47 | 9.682695 | 9.942726 | 9.739870 | 10.260130 | 9.696113 | 9.938475 | 9.757638 | 10.242362 | 13 |
| 48 | 9.682825 | 9.942656 | 9.740169 | 10.259831 | 9.696334 | 9.938402 | 9.757931 | 10.242069 | 12 |
| 49 | 9.683055 | 9.942587 | 9.740468 | 10.259532 | 9.696554 | 9.938330 | 9.758224 | 10.241776 | 11 |
| 50 | 9.683284 | 9.942517 | 9.740767 | 10.259233 | 9.696775 | 9.938258 | 9.758517 | 10.241483 | 10 |
| 51 | 9.683514 | 9.942448 | 9.741066 | 10.258934 | 9.696995 | 9.938185 | 9.758810 | 10.241190 | 9 |
| 52 | 9.683743 | 9.942378 | 9.741365 | 10.258635 | 9.697215 | 9.938113 | 9.759102 | 10.240898 | 8 |
| 53 | 9.683972 | 9.942308 | 9.741664 | 10.258336 | 9.697435 | 9.938040 | 9.759395 | 10.240605 | 7 |
| 54 | 9.684201 | 9.942239 | 9.741962 | 10.258038 | 9.697654 | 9.937967 | 9.759687 | 10.240313 | 6 |
| 55 | 9.684430 | 9.942169 | 9.742261 | 10.257739 | 9.697874 | 9.937895 | 9.759979 | 10.240021 | 5 |
| 56 | 9.684655 | 9.942099 | 9.742559 | 10.257441 | 9.698094 | 9.937822 | 9.760272 | 10.239728 | 4 |
| 57 | 9.684887 | 9.942029 | 9.742858 | 10.257142 | 9.698313 | 9.937749 | 9.760564 | 10.239486 | 3 |
| 58 | 9.685115 | 9.941959 | 9.743156 | 10.256844 | 9.698532 | 9.937676 | 9.760856 | 10.239144 | 2 |
| 59 | 9.685343 | 9.941889 | 9.743454 | 10.256546 | 9.698751 | 9.937604 | 9.761148 | 10.238852 | 1 |
| 60 | 9.685571 | 9.941819 | 9.743752 | 10.256248 | 9.698970 | 9.937531 | 9.761439 | 10.238561 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | / |

| 30° | | | | 31° | | | | | |
|-----|-----------|----------|----------|-----------|----------|----------|----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 9.698970 | 9.937531 | 9.761489 | 10.238561 | 9.711839 | 9.933066 | 9.778774 | 10.221226 | 60 |
| 1 | 9.699189 | 9.937458 | 9.761731 | 10.238269 | 9.712050 | 9.932990 | 9.779060 | 10.220940 | 59 |
| 2 | 9.699407 | 9.937385 | 9.762023 | 10.237977 | 9.712260 | 9.932914 | 9.779346 | 10.220654 | 58 |
| 3 | 9.699626 | 9.937312 | 9.762314 | 10.237686 | 9.712469 | 9.932938 | 9.779632 | 10.220368 | 57 |
| 4 | 9.699844 | 9.937238 | 9.762606 | 10.237394 | 9.712679 | 9.932762 | 9.779918 | 10.220082 | 56 |
| 5 | 9.700062 | 9.937165 | 9.762897 | 10.237103 | 9.712889 | 9.932685 | 9.780203 | 10.219797 | 55 |
| 6 | 9.700280 | 9.937092 | 9.763188 | 10.236812 | 9.713098 | 9.932609 | 9.780489 | 10.219511 | 54 |
| 7 | 9.700498 | 9.937019 | 9.763479 | 10.236521 | 9.713308 | 9.932533 | 9.780775 | 10.219225 | 53 |
| 8 | 9.700716 | 9.936946 | 9.763770 | 10.236230 | 9.713517 | 9.932457 | 9.781060 | 10.218940 | 52 |
| 9 | 9.700933 | 9.936872 | 9.764061 | 10.235939 | 9.713726 | 9.932380 | 9.781346 | 10.218654 | 51 |
| 10 | 9.701151 | 9.936799 | 9.764352 | 10.235648 | 9.713953 | 9.932304 | 9.781631 | 10.218369 | 50 |
| 11 | 9.701368 | 9.936725 | 9.764643 | 10.235357 | 9.714144 | 9.932228 | 9.781916 | 10.218084 | 49 |
| 12 | 9.701585 | 9.936652 | 9.764933 | 10.235067 | 9.714352 | 9.932151 | 9.782201 | 10.217799 | 48 |
| 13 | 9.701802 | 9.936578 | 9.765224 | 10.234776 | 9.714561 | 9.932075 | 9.782486 | 10.217514 | 47 |
| 14 | 9.702019 | 9.936505 | 9.765514 | 10.234486 | 9.714769 | 9.931998 | 9.782771 | 10.217229 | 46 |
| 15 | 9.702236 | 9.936431 | 9.765805 | 10.234195 | 9.714978 | 9.931921 | 9.783056 | 10.216914 | 45 |
| 16 | 9.702452 | 9.936357 | 9.766095 | 10.233905 | 9.715186 | 9.931845 | 9.783341 | 10.216659 | 44 |
| 17 | 9.702669 | 9.936284 | 9.766386 | 10.233615 | 9.715394 | 9.931768 | 9.783620 | 10.216374 | 43 |
| 18 | 9.702885 | 9.936210 | 9.766675 | 10.233325 | 9.715602 | 9.931691 | 9.783810 | 10.216090 | 42 |
| 19 | 9.703101 | 9.936136 | 9.766965 | 10.233035 | 9.715809 | 9.931614 | 9.784195 | 10.215805 | 41 |
| 20 | 9.703317 | 9.936062 | 9.767255 | 10.232745 | 9.716017 | 9.931537 | 9.784479 | 10.215521 | 40 |
| 21 | 9.703533 | 9.935988 | 9.767545 | 10.232155 | 9.716224 | 9.931460 | 9.784764 | 10.215236 | 39 |
| 22 | 9.703749 | 9.935914 | 9.767834 | 10.232166 | 9.716432 | 9.931383 | 9.785048 | 10.214952 | 38 |
| 23 | 9.703964 | 9.935840 | 9.768124 | 10.231876 | 9.716639 | 9.931306 | 9.785332 | 10.214668 | 37 |
| 24 | 9.704179 | 9.935766 | 9.768414 | 10.231586 | 9.716816 | 9.931229 | 9.785616 | 10.214384 | 36 |
| 25 | 9.704395 | 9.935692 | 9.768703 | 10.231297 | 9.717053 | 9.931152 | 9.785900 | 10.214100 | 35 |
| 26 | 9.704610 | 9.935618 | 9.768992 | 10.231008 | 9.717259 | 9.931075 | 9.786184 | 10.213816 | 34 |
| 27 | 9.704825 | 9.935543 | 9.769281 | 10.230719 | 9.717466 | 9.930998 | 9.786468 | 10.213532 | 33 |
| 28 | 9.705040 | 9.935469 | 9.769571 | 10.230429 | 9.717673 | 9.930921 | 9.786752 | 10.213248 | 32 |
| 29 | 9.705254 | 9.935395 | 9.769860 | 10.230140 | 9.717879 | 9.930843 | 9.787036 | 10.212964 | 31 |
| 30 | 9.705469 | 9.935320 | 9.770148 | 10.229882 | 9.718083 | 9.930766 | 9.787319 | 10.212681 | 30 |
| 31 | 9.705683 | 9.935246 | 9.770437 | 10.229563 | 9.718291 | 9.930688 | 9.787603 | 10.212397 | 29 |
| 32 | 9.705998 | 9.935171 | 9.770726 | 10.229274 | 9.718497 | 9.930611 | 9.787866 | 10.212114 | 28 |
| 33 | 9.706112 | 9.935097 | 9.771015 | 10.228985 | 9.718703 | 9.930533 | 9.788170 | 10.211830 | 27 |
| 34 | 9.706326 | 9.935022 | 9.771303 | 10.228697 | 9.718909 | 9.930456 | 9.788453 | 10.211547 | 26 |
| 35 | 9.706539 | 9.934948 | 9.771592 | 10.228408 | 9.719114 | 9.930378 | 9.788736 | 10.211264 | 25 |
| 36 | 9.706753 | 9.934873 | 9.771880 | 10.228120 | 9.719320 | 9.930300 | 9.789019 | 10.210981 | 24 |
| 37 | 9.706967 | 9.934798 | 9.772168 | 10.227832 | 9.719525 | 9.930223 | 9.789302 | 10.210698 | 23 |
| 38 | 9.707180 | 9.934723 | 9.772457 | 10.227543 | 9.719730 | 9.930145 | 9.789585 | 10.210415 | 22 |
| 39 | 9.707393 | 9.934649 | 9.772745 | 10.227255 | 9.719935 | 9.930067 | 9.789868 | 10.210132 | 21 |
| 40 | 9.7 70606 | 9.934574 | 9.773033 | 10.226967 | 9.720140 | 9.929989 | 9.790151 | 10.209849 | 20 |
| 41 | 9.707819 | 9.934499 | 9.773321 | 10.226679 | 9.720345 | 9.929911 | 9.790434 | 10.209506 | 19 |
| 42 | 9.708032 | 9.934424 | 9.773608 | 10.226392 | 9.720545 | 9.929833 | 9.790716 | 10.209284 | 18 |
| 43 | 9.708245 | 9.934349 | 9.773896 | 10.226104 | 9.720754 | 9.929755 | 9.790999 | 10.209001 | 17 |
| 44 | 9.708458 | 9.934274 | 9.774184 | 10.225816 | 9.720958 | 9.929677 | 9.791281 | 10.208719 | 16 |
| 45 | 9.708670 | 9.934199 | 9.774471 | 10.225525 | 9.721162 | 9.929599 | 9.791563 | 10.208437 | 15 |
| 46 | 9.708882 | 9.934123 | 9.774759 | 10.225241 | 9.721366 | 9.929521 | 9.791846 | 10.208154 | 14 |
| 47 | 9.709093 | 9.934048 | 9.775046 | 10.224954 | 9.721570 | 9.929442 | 9.792128 | 10.207872 | 13 |
| 48 | 9.709306 | 9.933973 | 9.775333 | 10.224667 | 9.721774 | 9.929364 | 9.794410 | 10.207590 | 12 |
| 49 | 9.709518 | 9.933898 | 9.775621 | 10.224379 | 9.721978 | 9.929286 | 9.792692 | 10.207308 | 11 |
| 50 | 9.709730 | 9.933822 | 9.775908 | 10.224092 | 9.722181 | 9.929207 | 9.792974 | 10.207026 | 10 |
| 51 | 9.709941 | 9.933747 | 9.776195 | 10.223805 | 9.722385 | 9.929129 | 9.793256 | 10.206744 | 9 |
| 52 | 9.710153 | 9.933671 | 9.776482 | 10.223518 | 9.722588 | 9.929050 | 9.793538 | 10.206462 | 8 |
| 53 | 9.710364 | 9.933596 | 9.776769 | 10.223232 | 9.722791 | 9.928972 | 9.793819 | 10.206181 | 7 |
| 54 | 9.710575 | 9.933520 | 9.777055 | 10.222945 | 9.722994 | 9.928853 | 9.794101 | 10.205899 | 6 |
| 55 | 9.710786 | 9.933445 | 9.777342 | 10.222658 | 9.723197 | 9.928815 | 9.794383 | 10.205617 | 5 |
| 56 | 9.710997 | 9.933369 | 9.777628 | 10.222372 | 9.723400 | 9.928796 | 9.794664 | 10.205336 | 4 |
| 57 | 9.711208 | 9.933293 | 9.777915 | 10.222084 | 9.723603 | 9.928657 | 9.794946 | 10.205054 | 3 |
| 58 | 9.711419 | 9.933217 | 9.778201 | 10.221799 | 9.723805 | 9.928578 | 9.795227 | 10.204773 | 2 |
| 59 | 9.711629 | 9.933141 | 9.778488 | 10.221512 | 9.724007 | 9.928499 | 9.795508 | 10.204492 | 1 |
| 60 | 9.711839 | 9.933066 | 9.778774 | 10.221226 | 9.724210 | 9.928420 | 9.795789 | 10.204211 | 0 |

| | 32° | | | | 33° | | | | |
|----|----------|----------|----------|-----------|----------|----------|-----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 9'724210 | 9'928420 | 9'795789 | 10'204211 | 9'736100 | 9'923591 | 9'812517 | 10'187483 | 60 |
| 1 | 9'724412 | 9'928312 | 9'796070 | 10'203930 | 9'736303 | 9'923509 | 9'812794 | 10'187206 | 59 |
| 2 | 9'724614 | 9'928263 | 9'796351 | 10'203649 | 9'736498 | 9'923427 | 9'813070 | 10'186930 | 58 |
| 3 | 9'724816 | 9'928183 | 9'796632 | 10'203368 | 9'736692 | 9'923345 | 9'813347 | 10'186653 | 57 |
| 4 | 9'725017 | 9'928104 | 9'796913 | 10'203087 | 9'736886 | 9'923263 | 9'813623 | 10'186377 | 56 |
| 5 | 9'725219 | 9'928025 | 9'797194 | 10'202806 | 9'737080 | 9'923181 | 9'813899 | 10'186101 | 55 |
| 6 | 9'725420 | 9'927946 | 9'797474 | 10'202526 | 9'737274 | 9'923098 | 9'814176 | 10'185824 | 54 |
| 7 | 9'725622 | 9'927807 | 9'797755 | 10'202245 | 9'737467 | 9'923016 | 9'814452 | 10'185548 | 53 |
| 8 | 9'725823 | 9'927787 | 9'798036 | 10'201964 | 9'737661 | 9'922933 | 9'814728 | 10'185272 | 52 |
| 9 | 9'726024 | 9'927708 | 9'798316 | 10'201684 | 9'737855 | 9'922851 | 9'815004 | 10'184996 | 51 |
| 10 | 9'726225 | 9'927629 | 9'798596 | 10'201404 | 9'738048 | 9'922768 | 9'815280 | 10'184720 | 50 |
| 11 | 9'726426 | 9'927549 | 9'798877 | 10'201123 | 9'738241 | 9'922686 | 9'815555 | 10'184446 | 49 |
| 12 | 9'726626 | 9'927470 | 9'799157 | 10'200843 | 9'738434 | 9'922603 | 9'815831 | 10'184169 | 48 |
| 13 | 9'726827 | 9'927390 | 9'799437 | 10'200563 | 9'738627 | 9'922520 | 9'816107 | 10'183893 | 47 |
| 14 | 9'727027 | 9'927310 | 9'799717 | 10'200288 | 9'738820 | 9'922438 | 9'816382 | 10'183618 | 46 |
| 15 | 9'727228 | 9'927231 | 9'799997 | 10'200008 | 9'739013 | 9'922355 | 9'816658 | 10'183342 | 45 |
| 16 | 9'727428 | 9'927151 | 9'800277 | 10'199723 | 9'739206 | 9'922272 | 9'816933 | 10'183007 | 44 |
| 17 | 9'727628 | 9'927071 | 9'800557 | 10'199443 | 9'739398 | 9'922189 | 9'817209 | 10'182791 | 43 |
| 18 | 9'727828 | 9'926991 | 9'800836 | 10'199164 | 9'739590 | 9'922106 | 9'817484 | 10'182516 | 42 |
| 19 | 9'728027 | 9'926911 | 9'801116 | 10'198884 | 9'739785 | 9'922023 | 9'817759 | 10'182241 | 41 |
| 20 | 9'728227 | 9'926831 | 9'801396 | 10'198604 | 9'739975 | 9'921910 | 9'818035 | 10'181965 | 40 |
| 21 | 9'728427 | 9'926751 | 9'801675 | 10'198325 | 9'740167 | 9'921867 | 9'818310 | 10'181690 | 39 |
| 22 | 9'728626 | 9'926671 | 9'801955 | 10'198045 | 9'740359 | 9'921774 | 9'818585 | 10'181415 | 38 |
| 23 | 9'728825 | 9'926591 | 9'802234 | 10'197766 | 9'740550 | 9'921601 | 9'818860 | 10'181140 | 37 |
| 24 | 9'729024 | 9'926511 | 9'802513 | 10'197487 | 9'740742 | 9'921607 | 9'819135 | 10'180865 | 36 |
| 25 | 9'729223 | 9'926431 | 9'802792 | 10'197208 | 9'740934 | 9'921524 | 9'819410 | 10'180590 | 35 |
| 26 | 9'729422 | 9'926351 | 9'803073 | 10'196928 | 9'741125 | 9'921441 | 9'819684 | 10'180316 | 34 |
| 27 | 9'729621 | 9'926270 | 9'803351 | 10'196649 | 9'741316 | 9'921367 | 9'819959 | 10'180041 | 33 |
| 28 | 9'729820 | 9'926190 | 9'803630 | 10'196370 | 9'741506 | 9'921274 | 9'820234 | 10'179766 | 32 |
| 29 | 9'730018 | 9'926110 | 9'803909 | 10'196091 | 9'741699 | 9'921190 | 9'820508 | 10'179492 | 31 |
| 30 | 9'730217 | 9'926029 | 9'804187 | 10'195813 | 9'741889 | 9'921107 | 9'820783 | 10'179217 | 30 |
| 31 | 9'730415 | 9'925949 | 9'804466 | 10'195531 | 9'742080 | 9'921023 | 9'821037 | 10'178943 | 29 |
| 32 | 9'730613 | 9'925868 | 9'804745 | 10'195255 | 9'742274 | 9'920939 | 9'821332 | 10'178668 | 28 |
| 33 | 9'730811 | 9'925788 | 9'805023 | 10'194977 | 9'742462 | 9'920656 | 9'821606 | 10'178394 | 27 |
| 34 | 9'731009 | 9'925707 | 9'805302 | 10'194698 | 9'742652 | 9'920772 | 9'821880 | 10'178120 | 26 |
| 35 | 9'731206 | 9'925626 | 9'805580 | 10'194420 | 9'742842 | 9'920688 | 9'822154 | 10'177846 | 25 |
| 36 | 9'731404 | 9'925545 | 9'805859 | 10'194141 | 9'743033 | 9'920604 | 9'822429 | 10'177571 | 24 |
| 37 | 9'731602 | 9'925465 | 9'806137 | 10'193863 | 9'743223 | 9'920520 | 9'822703 | 10'177297 | 23 |
| 38 | 9'731799 | 9'925384 | 9'806415 | 10'193585 | 9'743413 | 9'920436 | 9'822977 | 10'177023 | 22 |
| 39 | 9'731996 | 9'925303 | 9'806693 | 10'193307 | 9'743602 | 9'920352 | 9'823251 | 10'176749 | 21 |
| 40 | 9'732493 | 9'925222 | 9'806971 | 10'193029 | 9'743792 | 9'920268 | 9'823524 | 10'176476 | 20 |
| 41 | 9'732830 | 9'925141 | 9'807249 | 10'192751 | 9'743982 | 9'920184 | 9'823798 | 10'176202 | 19 |
| 42 | 9'732587 | 9'925060 | 9'807527 | 10'192473 | 9'744171 | 9'920099 | 9'8241072 | 10'1759.8 | 18 |
| 43 | 9'732784 | 9'924979 | 9'807805 | 10'192195 | 9'744361 | 9'920015 | 9'824345 | 10'1756.5 | 17 |
| 44 | 9'732980 | 9'924897 | 9'808083 | 10'191917 | 9'744550 | 9'919931 | 9'824619 | 10'175381 | 16 |
| 45 | 9'733177 | 9'924816 | 9'808361 | 10'191639 | 9'744739 | 9'919846 | 9'824893 | 10'175107 | 15 |
| 46 | 9'733373 | 9'924735 | 9'808638 | 10'191362 | 9'744929 | 9'919762 | 9'825166 | 10'174834 | 14 |
| 47 | 9'733569 | 9'924654 | 9'808916 | 10'191084 | 9'745117 | 9'919677 | 9'825439 | 10'174561 | 13 |
| 48 | 9'733765 | 9'924572 | 9'809093 | 10'190607 | 9'745306 | 9'919593 | 9'825713 | 10'174287 | 12 |
| 49 | 9'733961 | 9'924491 | 9'809471 | 10'190529 | 9'745494 | 9'919508 | 9'825986 | 10'174014 | 11 |
| 50 | 9'734157 | 9'924409 | 9'809748 | 10'190262 | 9'745683 | 9'919424 | 9'826259 | 10'173741 | 10 |
| 51 | 9'734353 | 9'924328 | 9'810025 | 10'189975 | 9'745871 | 9'919339 | 9'826532 | 10'173468 | 9 |
| 52 | 9'734549 | 9'924246 | 9'810302 | 10'189698 | 9'746060 | 9'919254 | 9'826805 | 10'173195 | 8 |
| 53 | 9'734744 | 9'924164 | 9'810580 | 10'189420 | 9'746218 | 9'919169 | 9'827078 | 10'172922 | 7 |
| 54 | 9'734939 | 9'924088 | 9'810857 | 10'189143 | 9'746436 | 9'919085 | 9'827351 | 10'172649 | 6 |
| 55 | 9'735185 | 9'924001 | 9'811134 | 10'188866 | 9'746624 | 9'919000 | 9'827624 | 10'172376 | 5 |
| 56 | 9'735380 | 9'923919 | 9'811410 | 10'188590 | 9'746812 | 9'919195 | 9'827897 | 10'172103 | 4 |
| 57 | 9'735525 | 9'923837 | 9'811687 | 10'188313 | 9'746998 | 9'919890 | 9'828170 | 10'171830 | 3 |
| 58 | 9'735719 | 9'923755 | 9'811964 | 10'188036 | 9'747187 | 9'919745 | 9'828442 | 10'171558 | 2 |
| 59 | 9'735914 | 9'923673 | 9'812241 | 10'187759 | 9'747374 | 9'919659 | 9'828715 | 10'171285 | 1 |
| 60 | 9'736109 | 9'923591 | 9'812517 | 10'187483 | 9'747562 | 9'919574 | 9'828987 | 10'171013 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | |

LOGARITHMIC SINES AND TANGENTS.

| 34° | | | | | 35° | | | | |
|-----|---------|---------|----------|-----------|---------|---------|----------|-----------|--------|
| | Sine | Cosine | Tan. | Cotan. | | Sine | Cosine | Tan. | Cotan. |
| 0 | 9747562 | 9918574 | 97828987 | 10·171013 | 9758501 | 9913365 | 97845227 | 10·151773 | 60 |
| 1 | 9747719 | 9918489 | 97829260 | 10·170740 | 9753772 | 9913276 | 97845406 | 10·154504 | 59 |
| 2 | 9747936 | 9918104 | 97829532 | 10·170468 | 9758552 | 9913187 | 97845761 | 10·154236 | 58 |
| 3 | 9748123 | 9918318 | 97829805 | 10·170195 | 9759132 | 9913099 | 97846033 | 10·153967 | 57 |
| 4 | 9748310 | 9918233 | 97830077 | 10·169923 | 9759312 | 9913010 | 97846302 | 10·153698 | 56 |
| 5 | 9748497 | 9918147 | 97830349 | 10·169651 | 9759492 | 9912922 | 97846570 | 10·153130 | 55 |
| 6 | 9748643 | 9918062 | 97830621 | 10·169379 | 9759672 | 9912833 | 97846839 | 10·153161 | 54 |
| 7 | 9748870 | 9917976 | 97830893 | 10·169107 | 9759852 | 9912714 | 97847108 | 10·152832 | 53 |
| 8 | 9749056 | 9917891 | 97831165 | 10·168835 | 9760031 | 9912655 | 97847376 | 10·152624 | 52 |
| 9 | 9749243 | 9917805 | 97831437 | 10·168563 | 9760211 | 9912566 | 97847641 | 10·152356 | 51 |
| 10 | 9749429 | 9917719 | 97831709 | 10·168291 | 9760390 | 9912477 | 97847913 | 10·152087 | 50 |
| 11 | 9749615 | 9917634 | 97831981 | 10·168019 | 9760569 | 9912388 | 97848181 | 10·151819 | 49 |
| 12 | 9749801 | 9917548 | 97832253 | 10·167747 | 9760749 | 9912299 | 97848410 | 10·151551 | 48 |
| 13 | 9749987 | 9917462 | 9783252 | 10·167475 | 9760927 | 9912210 | 97848717 | 10·151283 | 47 |
| 14 | 9750172 | 9917376 | 97832796 | 10·167204 | 9761106 | 9912121 | 97849086 | 10·151014 | 46 |
| 15 | 9750358 | 9917290 | 97833068 | 10·166932 | 9761285 | 9912031 | 97849254 | 10·150746 | 45 |
| 16 | 9750543 | 9917204 | 97833329 | 10·166661 | 9761464 | 9911942 | 97849522 | 10·150478 | 44 |
| 17 | 9750729 | 9917118 | 97833611 | 10·166389 | 9761642 | 9911853 | 97849730 | 10·150210 | 43 |
| 18 | 9750914 | 9917032 | 97833882 | 10·166118 | 9761821 | 9911763 | 97850057 | 10·149943 | 42 |
| 19 | 9751099 | 9916946 | 97834154 | 10·165846 | 9761939 | 9911673 | 97850325 | 10·149675 | 41 |
| 20 | 9751284 | 9916859 | 97834425 | 10·165575 | 9762177 | 9911584 | 97850593 | 10·149407 | 40 |
| 21 | 9751460 | 9916773 | 97834696 | 10·165304 | 9762356 | 9911495 | 97850861 | 10·149139 | 39 |
| 22 | 9751654 | 9916687 | 97834967 | 10·165033 | 9762534 | 9911405 | 97851129 | 10·148871 | 38 |
| 23 | 9751839 | 9916600 | 97835238 | 10·164762 | 9762712 | 9911315 | 97851306 | 10·148604 | 37 |
| 24 | 9752023 | 9916514 | 97835503 | 10·164491 | 9762889 | 9911226 | 97851664 | 10·148336 | 36 |
| 25 | 9752208 | 9916427 | 97835780 | 10·164220 | 9763067 | 9911136 | 97851931 | 10·148069 | 35 |
| 26 | 9752392 | 9916341 | 97836051 | 10·163949 | 9763245 | 9911046 | 97852199 | 10·147801 | 34 |
| 27 | 9752576 | 9916254 | 97836322 | 10·163678 | 9763422 | 9910956 | 97852466 | 10·147534 | 33 |
| 28 | 9752760 | 9916167 | 97836593 | 10·163407 | 9763609 | 9910866 | 97852733 | 10·147267 | 32 |
| 29 | 9752944 | 9916081 | 97836864 | 10·163136 | 9763777 | 9910776 | 97853091 | 10·146999 | 31 |
| 30 | 9753128 | 9915994 | 97837134 | 10·162866 | 9763951 | 9910686 | 97853268 | 10·146732 | 30 |
| 31 | 9753312 | 9915907 | 97837405 | 10·162595 | 9764131 | 9910596 | 97853535 | 10·146465 | 29 |
| 32 | 9753495 | 9915820 | 97837675 | 10·162325 | 9764308 | 9910506 | 97853802 | 10·146198 | 28 |
| 33 | 9753679 | 9915733 | 97837946 | 10·162051 | 9764483 | 9910415 | 97854069 | 10·145931 | 27 |
| 34 | 9753852 | 9915646 | 97838210 | 10·161784 | 9764662 | 9910323 | 97854336 | 10·145664 | 26 |
| 35 | 9754046 | 9915559 | 97838487 | 10·161513 | 9764838 | 9910231 | 97854603 | 10·145397 | 25 |
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| 39 | 9754778 | 9915210 | 97839568 | 10·160432 | 9765544 | 9909873 | 97855671 | 10·144329 | 21 |
| 40 | 9754960 | 9915123 | 97839838 | 10·160162 | 9765720 | 9909782 | 97855938 | 10·144062 | 20 |
| 41 | 9755143 | 9915035 | 97840108 | 10·159892 | 9765896 | 9909691 | 97856201 | 10·143796 | 19 |
| 42 | 9755326 | 9914948 | 97840378 | 10·159622 | 9766072 | 9909601 | 97856471 | 10·143529 | 18 |
| 43 | 9755508 | 9914860 | 97840648 | 10·159352 | 9766247 | 9909510 | 97856737 | 10·143263 | 17 |
| 44 | 9755690 | 9914773 | 97840917 | 10·159083 | 9766423 | 9909419 | 97857004 | 10·142996 | 16 |
| 45 | 9755872 | 9914685 | 97841187 | 10·158813 | 9766598 | 9909329 | 97857270 | 10·142730 | 15 |
| 46 | 9756054 | 9914598 | 97841457 | 10·158543 | 9766774 | 9909287 | 97857537 | 10·142463 | 14 |
| 47 | 9756236 | 9914510 | 97841727 | 10·15273 | 9766949 | 9909146 | 97857803 | 10·142197 | 13 |
| 48 | 9756418 | 9914422 | 97841996 | 10·156004 | 9767124 | 9909055 | 97858069 | 10·141931 | 12 |
| 49 | 9756600 | 9914334 | 97842266 | 10·157731 | 9767300 | 9908961 | 97858336 | 10·141664 | 11 |
| 50 | 9756782 | 9914246 | 97842535 | 10·157465 | 9767475 | 9908873 | 97858602 | 10·141398 | 10 |
| 51 | 9756963 | 9914158 | 97842805 | 10·157195 | 9767649 | 9908781 | 97858868 | 10·141132 | 9 |
| 52 | 9757141 | 9914070 | 97843074 | 10·156926 | 9767824 | 9908690 | 97859131 | 10·140866 | 8 |
| 53 | 9757326 | 9913982 | 97843343 | 10·156657 | 9767999 | 9908599 | 97859400 | 10·140600 | 7 |
| 54 | 9757507 | 9913894 | 97843612 | 10·156388 | 9768173 | 9908507 | 97859666 | 10·140334 | 6 |
| 55 | 9757688 | 9913806 | 97843882 | 10·156118 | 9768348 | 9908416 | 97859932 | 10·140068 | 5 |
| 56 | 9757869 | 9913718 | 97844151 | 10·155849 | 9768522 | 9908324 | 97860198 | 10·139802 | 4 |
| 57 | 9758050 | 9913630 | 97844420 | 10·155580 | 9768697 | 9908233 | 97860464 | 10·139536 | 3 |
| 58 | 9758230 | 9913541 | 97844689 | 10·155311 | 9768871 | 9908141 | 97860730 | 10·139270 | 2 |
| 59 | 9758411 | 9913453 | 97844958 | 10·155042 | 9769045 | 9908049 | 97860995 | 10·138005 | 1 |
| 60 | 9758591 | 9913365 | 97845227 | 10·154773 | 9769219 | 9907958 | 97861261 | 10·138739 | 0 |

55°

54°

| | 36° | | | | 37° | | | | |
|----|----------|----------|----------|-----------|----------|-----------|-----------|-----------|----|
| / | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 9'760219 | 9'907938 | 9'861261 | 10'138749 | 9'779463 | 9'902359 | 9'874114 | 10'122886 | 60 |
| 1 | 9'763393 | 9'907866 | 9'861527 | 10'138473 | 9'779631 | 9'902253 | 9'877377 | 10'122023 | 59 |
| 2 | 9'769566 | 9'907774 | 9'861792 | 10'138208 | 9'779798 | 9'902158 | 9'877640 | 10'122360 | 58 |
| 3 | 9'769740 | 9'907682 | 9'862058 | 10'137942 | 9'779966 | 9'902063 | 9'877903 | 10'122097 | 57 |
| 4 | 9'769913 | 9'907590 | 9'862323 | 10'137677 | 9'780133 | 9'901967 | 9'878165 | 10'121835 | 56 |
| 5 | 9'770087 | 9'907498 | 9'862589 | 10'137411 | 9'780300 | 9'901872 | 9'878428 | 10'121572 | 55 |
| 6 | 9'770260 | 9'907406 | 9'862854 | 10'137146 | 9'780467 | 9'901776 | 9'878691 | 10'121309 | 54 |
| 7 | 9'770433 | 9'907314 | 9'863119 | 10'136881 | 9'780634 | 9'901681 | 9'878953 | 10'121047 | 53 |
| 8 | 9'770606 | 9'907222 | 9'863385 | 10'136615 | 9'780801 | 9'901585 | 9'879216 | 10'120764 | 52 |
| 9 | 9'770779 | 9'907129 | 9'863650 | 10'136350 | 9'780968 | 9'901490 | 9'879478 | 10'120592 | 51 |
| 10 | 9'770952 | 9'907037 | 9'863915 | 10'136085 | 9'781134 | 9'901394 | 9'879741 | 10'120259 | 50 |
| 11 | 9'771425 | 9'906945 | 9'864180 | 10'135820 | 9'781301 | 9'901298 | 9'880003 | 10'119997 | 49 |
| 12 | 9'771298 | 9'906852 | 9'864445 | 10'135555 | 9'781468 | 9'901202 | 9'880265 | 10'119735 | 48 |
| 13 | 9'771470 | 9'906760 | 9'864710 | 10'135290 | 9'781634 | 9'901106 | 9'880528 | 10'119472 | 47 |
| 14 | 9'771643 | 9'906667 | 9'864975 | 10'135025 | 9'781800 | 9'901010 | 9'880790 | 10'119210 | 46 |
| 15 | 9'771815 | 9'906575 | 9'865240 | 10'134760 | 9'782166 | 9'900914 | 9'881052 | 10'118918 | 45 |
| 16 | 9'771987 | 9'906482 | 9'865505 | 10'134495 | 9'782132 | 9'900818 | 9'881314 | 10'118686 | 44 |
| 17 | 9'772159 | 9'906389 | 9'865770 | 10'134230 | 9'782298 | 9'900722 | 9'881577 | 10'118423 | 43 |
| 18 | 9'772331 | 9'906296 | 9'866035 | 10'133965 | 9'782464 | 9'900626 | 9'881839 | 10'118161 | 42 |
| 19 | 9'772503 | 9'906204 | 9'866300 | 10'133700 | 9'782630 | 9'900529 | 9'882101 | 10'117899 | 41 |
| 20 | 9'772675 | 9'906111 | 9'866564 | 10'133436 | 9'782796 | 9'900433 | 9'882363 | 10'117637 | 40 |
| 21 | 9'772847 | 9'906018 | 9'866820 | 10'133171 | 9'782961 | 9'900337 | 9'882625 | 10'117375 | 39 |
| 22 | 9'773018 | 9'905925 | 9'867094 | 10'132906 | 9'783127 | 9'900240 | 9'882887 | 10'117113 | 38 |
| 23 | 9'773190 | 9'905832 | 9'867358 | 10'132642 | 9'783292 | 9'900144 | 9'883148 | 10'116852 | 37 |
| 24 | 9'773361 | 9'905739 | 9'867623 | 10'132377 | 9'783458 | 9'900047 | 9'883410 | 10'116590 | 36 |
| 25 | 9'773533 | 9'905645 | 9'867887 | 10'132113 | 9'783623 | 9'889951 | 9'883672 | 10'116328 | 35 |
| 26 | 9'773704 | 9'905552 | 9'868152 | 10'131848 | 9'783788 | 9'889954 | 9'883934 | 10'116066 | 34 |
| 27 | 9'773875 | 9'905459 | 9'868416 | 10'131584 | 9'783953 | 9'889976 | 9'884196 | 10'115804 | 33 |
| 28 | 9'774046 | 9'905366 | 9'868680 | 10'131320 | 9'784118 | 9'889660 | 9'884457 | 10'115543 | 32 |
| 29 | 9'774217 | 9'905272 | 9'868945 | 10'131055 | 9'784282 | 9'889564 | 9'884719 | 10'115281 | 31 |
| 30 | 9'774388 | 9'905179 | 9'869209 | 10'130791 | 9'784447 | 9'889467 | 9'884980 | 10'115020 | 30 |
| 31 | 9'774558 | 9'905085 | 9'869473 | 10'130527 | 9'784612 | 9'889370 | 9'885242 | 10'114758 | 29 |
| 32 | 9'774729 | 9'904992 | 9'869737 | 10'130263 | 9'784776 | 9'889273 | 9'885504 | 10'114496 | 28 |
| 33 | 9'774899 | 9'904898 | 9'870001 | 10'129999 | 9'784941 | 9'889176 | 9'885765 | 10'114235 | 27 |
| 34 | 9'775070 | 9'904804 | 9'870265 | 10'129735 | 9'785103 | 9'889078 | 9'886026 | 10'113974 | 26 |
| 35 | 9'775240 | 9'904711 | 9'870529 | 10'129471 | 9'785260 | 9'889881 | 9'886288 | 10'113712 | 25 |
| 36 | 9'775410 | 9'904617 | 9'870793 | 10'129207 | 9'785433 | 9'889884 | 9'886549 | 10'113451 | 24 |
| 37 | 9'775580 | 9'904523 | 9'871057 | 10'128943 | 9'785597 | 9'889878 | 9'886811 | 10'113180 | 23 |
| 38 | 9'775759 | 9'904429 | 9'871321 | 10'128679 | 9'785761 | 9'889689 | 9'887072 | 10'112928 | 22 |
| 39 | 9'775920 | 9'904335 | 9'871585 | 10'128415 | 9'785925 | 9'889552 | 9'887333 | 10'112667 | 21 |
| 40 | 9'776090 | 9'904241 | 9'871849 | 10'128151 | 9'786089 | 9'889494 | 9'887594 | 10'112406 | 20 |
| 41 | 9'776259 | 9'904147 | 9'872112 | 10'127888 | 9'786252 | 9'889397 | 9'887855 | 10'112145 | 19 |
| 42 | 9'776429 | 9'904053 | 9'872376 | 10'127624 | 9'786416 | 9'889299 | 9'888116 | 10'111884 | 18 |
| 43 | 9'776598 | 9'903950 | 9'872640 | 10'127360 | 9'786579 | 9'889120 | 9'888378 | 10'111622 | 17 |
| 44 | 9'776768 | 9'903864 | 9'872903 | 10'127097 | 9'786742 | 9'889104 | 9'888639 | 10'111361 | 16 |
| 45 | 9'776937 | 9'903770 | 9'873167 | 10'126833 | 9'786806 | 9'889006 | 9'888900 | 10'111100 | 15 |
| 46 | 9'777106 | 9'903676 | 9'873430 | 10'126570 | 9'786969 | 9'8897908 | 9'889161 | 10'108039 | 14 |
| 47 | 9'777275 | 9'903581 | 9'873694 | 10'126306 | 9'787232 | 9'8897810 | 9'889421 | 10'105079 | 13 |
| 48 | 9'777444 | 9'903487 | 9'873957 | 10'126043 | 9'787395 | 9'8897712 | 9'889682 | 10'10318 | 12 |
| 49 | 9'777613 | 9'903392 | 9'874220 | 10'125780 | 9'787557 | 9'8897614 | 9'889943 | 10'100057 | 11 |
| 50 | 9'777781 | 9'903298 | 9'874184 | 10'125516 | 9'787720 | 9'8897516 | 9'8890204 | 10'09796 | 10 |
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| 52 | 9'778119 | 9'903108 | 9'875010 | 10'124990 | 9'788045 | 9'8897320 | 9'890723 | 10'09275 | 8 |
| 53 | 9'778287 | 9'903014 | 9'875273 | 10'124727 | 9'788208 | 9'8897222 | 9'890986 | 10'09014 | 7 |
| 54 | 9'778455 | 9'902919 | 9'875537 | 10'124463 | 9'788370 | 9'8897123 | 9'891247 | 10'08753 | 6 |
| 55 | 9'778624 | 9'902824 | 9'875800 | 10'124200 | 9'788532 | 9'8897025 | 9'891507 | 10'08493 | 5 |
| 56 | 9'778792 | 9'902729 | 9'876063 | 10'123937 | 9'788694 | 9'8896926 | 9'891768 | 10'08232 | 4 |
| 57 | 9'778900 | 9'902634 | 9'876326 | 10'123674 | 9'788856 | 9'8896828 | 9'892028 | 10'07972 | 3 |
| 58 | 9'779128 | 9'902539 | 9'876589 | 10'123411 | 9'789018 | 9'8896729 | 9'892219 | 10'07711 | 2 |
| 59 | 9'779295 | 9'902444 | 9'876852 | 10'123148 | 9'789180 | 9'8896631 | 9'892549 | 10'07451 | 1 |
| 60 | 9'779463 | 9'902349 | 9'877114 | 10'122886 | 9'789532 | 9'8896592 | 9'892810 | 10'07190 | 0 |
| | Covine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | / |

38°

39°

| | Sine | Cosine | Tan. | Cotan. | | Sine | Cosine | Tan. | Cotan. | |
|----|---------|---------|----------|-----------|----------|----------|----------|----------|--------|--|
| 0 | 9788342 | 9896532 | 9792810 | 10107190 | 9798872 | 9890503 | 9790369 | 10091631 | 60 | |
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| 2 | 9786665 | 9896335 | 9793331 | 10106669 | 979184 | 9890206 | 9790686 | 10091114 | 58 | |
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| 4 | 9786998 | 9896137 | 9793851 | 10106149 | 9794945 | 9890093 | 97909402 | 10090598 | 56 | |
| 5 | 9790149 | 9896038 | 9794111 | 10105889 | 9795651 | 9889990 | 97909660 | 10090340 | 55 | |
| 6 | 9790310 | 9895930 | 97941872 | 10105368 | 9799804 | 9889888 | 97909918 | 10090082 | 54 | |
| 7 | 9790471 | 9895840 | 9794632 | 10105028 | 9799962 | 9889785 | 97910177 | 10089823 | 53 | |
| 8 | 9790632 | 9895741 | 9794892 | 10105108 | 9800117 | 9889682 | 97910485 | 10089565 | 52 | |
| 9 | 9790703 | 9895041 | 9795152 | 10104848 | 9800272 | 9889579 | 97910093 | 10089307 | 51 | |
| 10 | 9790951 | 9895542 | 9795412 | 10104588 | 9800427 | 9889477 | 97910951 | 10089049 | 50 | |
| 11 | 9791115 | 9895443 | 9795672 | 10104328 | 9800582 | 9889374 | 97911209 | 10088791 | 49 | |
| 12 | 9791275 | 9895343 | 9795932 | 10104068 | 9800737 | 9889271 | 97911467 | 10088533 | 48 | |
| 13 | 9791436 | 9895214 | 9796192 | 10103808 | 9800892 | 9889168 | 97911726 | 10088275 | 47 | |
| 14 | 9791596 | 9895145 | 9796462 | 10103548 | 9801047 | 9889064 | 97911982 | 10088018 | 46 | |
| 15 | 9791757 | 9895045 | 9796712 | 10103288 | 9801201 | 9888961 | 97912210 | 10087760 | 45 | |
| 16 | 9791917 | 9894945 | 9796971 | 10103029 | 9801356 | 9888858 | 97912498 | 10087502 | 44 | |
| 17 | 9792077 | 9894846 | 9797231 | 10102709 | 9801511 | 9888755 | 97912756 | 10087244 | 43 | |
| 18 | 9792237 | 9894746 | 9797491 | 10102509 | 9801665 | 9888651 | 97913014 | 10086986 | 42 | |
| 19 | 9792397 | 9894646 | 9797751 | 10102249 | 9801819 | 9888548 | 97913271 | 10086729 | 41 | |
| 20 | 9792557 | 9894546 | 9798010 | 10101990 | 9801973 | 9888444 | 97913529 | 10086471 | 40 | |
| 21 | 9792716 | 9894446 | 9798270 | 10101730 | 9802128 | 9888341 | 97913787 | 10086213 | 39 | |
| 22 | 9792876 | 9894346 | 9798530 | 10101470 | 9802289 | 9888237 | 97914044 | 10085956 | 38 | |
| 23 | 9793035 | 9894246 | 9798878 | 10101211 | 9802430 | 9888134 | 97914302 | 10085819 | 37 | |
| 24 | 9793195 | 9894146 | 9799049 | 10100951 | 9802589 | 9888030 | 97914560 | 10085440 | 36 | |
| 25 | 9793354 | 9894046 | 9799308 | 10100692 | 9802743 | 9887926 | 97914817 | 10085183 | 35 | |
| 26 | 9793514 | 9893946 | 9799568 | 10100432 | 9802897 | 9887822 | 97915075 | 10084325 | 34 | |
| 27 | 9793673 | 9893846 | 9799827 | 10100173 | 9803050 | 9887718 | 97915332 | 10084068 | 33 | |
| 28 | 9793832 | 9893748 | 97990087 | 10100913 | 9803204 | 9887614 | 97915590 | 10084410 | 32 | |
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| 31 | 9794308 | 9893444 | 97990864 | 101009136 | 9803664 | 9887302 | 97916362 | 10083438 | 29 | |
| 32 | 9794467 | 9893343 | 97991124 | 101008876 | 9803817 | 9887198 | 97916619 | 10083381 | 28 | |
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| 35 | 9791942 | 9893041 | 97991901 | 101008099 | 9804276 | 9886885 | 97917391 | 10082609 | 25 | |
| 36 | 9795101 | 9892940 | 97992160 | 101007840 | 9804428 | 9886780 | 97917648 | 10082352 | 24 | |
| 37 | 9795259 | 9892839 | 97902420 | 10097580 | 97904581 | 9886676 | 97917906 | 10082094 | 23 | |
| 38 | 9795417 | 9892739 | 97902679 | 10097321 | 97904734 | 9886571 | 97918163 | 10081837 | 22 | |
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| 46 | 9796679 | 9791929 | 97904750 | 10095250 | 97905951 | 9885732 | 97920219 | 10079781 | 14 | |
| 47 | 9796836 | 9791827 | 97905008 | 10094992 | 97906103 | 9885627 | 97920476 | 10079524 | 13 | |
| 48 | 9796993 | 9791726 | 97905267 | 10094733 | 97906253 | 9885522 | 97920733 | 10079267 | 12 | |
| 49 | 9797150 | 9791624 | 97905526 | 10094474 | 9790406 | 9885416 | 97920990 | 10079710 | 11 | |
| 50 | 9797307 | 9791523 | 97905785 | 10094215 | 9790557 | 9885311 | 97921247 | 10078753 | 10 | |
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| 54 | 9797934 | 9791115 | 97906819 | 10093181 | 97907163 | 97904889 | 97922274 | 10077726 | 6 | |
| 55 | 9798091 | 9790103 | 97907077 | 10092923 | 97907314 | 97904783 | 97922530 | 10077470 | 5 | |
| 56 | 9798247 | 9790911 | 97907336 | 10092664 | 97907465 | 97904677 | 97922784 | 10077213 | 4 | |
| 57 | 9798403 | 9790809 | 97907594 | 10092406 | 97907615 | 97904572 | 97923044 | 10076956 | 3 | |
| 58 | 9798560 | 9790707 | 97907853 | 10092147 | 97907766 | 97904466 | 97923300 | 10076700 | 2 | |
| 59 | 9798616 | 9790605 | 97908111 | 10091889 | 97907917 | 97904300 | 97923557 | 10076443 | 1 | |
| 60 | 9798672 | 9790503 | 97908369 | 10091631 | 97908067 | 97904254 | 97923814 | 10076186 | 0 | |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | | |

51°

50°

| 40° | | | | 41° | | | | | |
|-----|----------|----------|----------|------------|----------|----------|----------|-----------|----|
| | Sine | Cosine | Tan. | Cotan. | Sine | Cosine | Tan. | Cotan. | |
| 0 | 9.808067 | 9.884254 | 9.923814 | 10.076180 | 9.816943 | 9.877780 | 9.939163 | 10.060837 | 60 |
| 1 | 9.808218 | 9.884148 | 9.924070 | 10.075930 | 9.817089 | 9.877670 | 9.939418 | 10.060582 | 59 |
| 2 | 9.808368 | 9.884042 | 9.924327 | 10.075673 | 9.817233 | 9.877560 | 9.939763 | 10.060327 | 58 |
| 3 | 9.808519 | 9.883936 | 9.921582 | 10.075417 | 9.817379 | 9.877450 | 9.939929 | 10.060072 | 57 |
| 4 | 9.808669 | 9.883829 | 9.921440 | 10.075160 | 9.817524 | 9.877340 | 9.940183 | 10.059817 | 56 |
| 5 | 9.808819 | 9.883723 | 9.925096 | 10.074904 | 9.817668 | 9.877230 | 9.940459 | 10.059561 | 55 |
| 6 | 9.808969 | 9.883617 | 9.925352 | 10.074648 | 9.817813 | 9.877120 | 9.940694 | 10.059306 | 54 |
| 7 | 9.809119 | 9.883510 | 9.925609 | 10.074319 | 9.817958 | 9.877010 | 9.940949 | 10.059051 | 53 |
| 8 | 9.809269 | 9.883404 | 9.925865 | 10.074135 | 9.818013 | 9.876899 | 9.941120 | 10.058796 | 52 |
| 9 | 9.809419 | 9.883297 | 9.926122 | 10.073783 | 9.818247 | 9.876789 | 9.941459 | 10.058511 | 51 |
| 10 | 9.809569 | 9.883191 | 9.926178 | 10.073622 | 9.818392 | 9.876678 | 9.941713 | 10.058287 | 50 |
| 11 | 9.809718 | 9.883094 | 9.926034 | 10.073366 | 9.818536 | 9.876568 | 9.941968 | 10.058082 | 49 |
| 12 | 9.808768 | 9.882977 | 9.926891 | 10.073110 | 9.818681 | 9.876457 | 9.942223 | 10.057777 | 48 |
| 13 | 9.810017 | 9.882871 | 9.927147 | 10.072853 | 9.818825 | 9.876347 | 9.942478 | 10.057522 | 47 |
| 14 | 9.810167 | 9.882764 | 9.927403 | 10.072597 | 9.818969 | 9.876236 | 9.942733 | 10.057267 | 46 |
| 15 | 9.810316 | 9.882657 | 9.927659 | 10.072341 | 9.819113 | 9.876125 | 9.942988 | 10.057012 | 45 |
| 16 | 9.810465 | 9.882550 | 9.927915 | 10.072085 | 9.819257 | 9.876014 | 9.943243 | 10.056757 | 44 |
| 17 | 9.810614 | 9.882443 | 9.928171 | 10.071829 | 9.819401 | 9.875904 | 9.943498 | 10.056502 | 43 |
| 18 | 9.810763 | 9.882336 | 9.928427 | 10.071573 | 9.819545 | 9.875793 | 9.943752 | 10.056248 | 42 |
| 19 | 9.810912 | 9.882229 | 9.928684 | 10.071316 | 9.819689 | 9.875682 | 9.944007 | 10.055993 | 41 |
| 20 | 9.811061 | 9.882121 | 9.928940 | 10.071060 | 9.819832 | 9.875571 | 9.944262 | 10.055738 | 40 |
| 21 | 9.811210 | 9.882014 | 9.929196 | 10.070804 | 9.819976 | 9.875459 | 9.944517 | 10.055483 | 39 |
| 22 | 9.811358 | 9.881907 | 9.929152 | 10.070548 | 9.820120 | 9.875348 | 9.944771 | 10.055229 | 38 |
| 23 | 9.811507 | 9.881799 | 9.929708 | 10.070292 | 9.820203 | 9.875237 | 9.945026 | 10.054974 | 37 |
| 24 | 9.811655 | 9.881692 | 9.929904 | 10.070036 | 9.820406 | 9.875126 | 9.945281 | 10.054719 | 36 |
| 25 | 9.811804 | 9.881554 | 9.930220 | 10.069780 | 9.820550 | 9.875014 | 9.945535 | 10.054465 | 35 |
| 26 | 9.811952 | 9.881477 | 9.930475 | 10.069525 | 9.820693 | 9.874903 | 9.945790 | 10.054210 | 34 |
| 27 | 9.812100 | 9.881369 | 9.930731 | 10.069269 | 9.820836 | 9.874791 | 9.946045 | 10.053955 | 33 |
| 28 | 9.812248 | 9.881261 | 9.930987 | 10.069013 | 9.820979 | 9.874680 | 9.946299 | 10.053701 | 32 |
| 29 | 9.812396 | 9.881153 | 9.931243 | 10.0665757 | 9.821122 | 9.874568 | 9.946554 | 10.053446 | 31 |
| 30 | 9.812544 | 9.881046 | 9.931499 | 10.068501 | 9.821265 | 9.874456 | 9.946808 | 10.053192 | 30 |
| 31 | 9.812692 | 9.880938 | 9.931755 | 10.068245 | 9.821407 | 9.874344 | 9.947063 | 10.052937 | 29 |
| 32 | 9.812840 | 9.880836 | 9.932010 | 10.067990 | 9.821550 | 9.874232 | 9.947318 | 10.052682 | 28 |
| 33 | 9.812988 | 9.880722 | 9.932266 | 10.067734 | 9.821693 | 9.874121 | 9.947575 | 10.052428 | 27 |
| 34 | 9.813135 | 9.880613 | 9.932522 | 10.067478 | 9.821835 | 9.874009 | 9.947827 | 10.052173 | 26 |
| 35 | 9.813283 | 9.880505 | 9.932778 | 10.067222 | 9.821977 | 9.873896 | 9.948081 | 10.051919 | 25 |
| 36 | 9.813430 | 9.880397 | 9.933033 | 10.066967 | 9.822120 | 9.873784 | 9.948335 | 10.051663 | 24 |
| 37 | 9.813578 | 9.880289 | 9.933289 | 10.066711 | 9.822262 | 9.873672 | 9.948590 | 10.051410 | 23 |
| 38 | 9.813725 | 9.880180 | 9.933545 | 10.066455 | 9.822404 | 9.873560 | 9.948844 | 10.051156 | 22 |
| 39 | 9.813872 | 9.880072 | 9.933800 | 10.066200 | 9.822546 | 9.873448 | 9.949099 | 10.050901 | 21 |
| 40 | 9.814019 | 9.879963 | 9.934056 | 10.065944 | 9.822688 | 9.873335 | 9.949353 | 10.050647 | 20 |
| 41 | 9.814166 | 9.879955 | 9.934311 | 10.065689 | 9.822830 | 9.873223 | 9.949608 | 10.050392 | 19 |
| 42 | 9.814313 | 9.879716 | 9.934507 | 10.065433 | 9.822972 | 9.873110 | 9.949862 | 10.050138 | 18 |
| 43 | 9.814460 | 9.879367 | 9.934822 | 10.065178 | 9.823114 | 9.872998 | 9.950116 | 10.049584 | 17 |
| 44 | 9.814607 | 9.879329 | 9.935074 | 10.064922 | 9.823255 | 9.872885 | 9.950371 | 10.049329 | 16 |
| 45 | 9.814753 | 9.879240 | 9.935333 | 10.064667 | 9.823397 | 9.872772 | 9.950625 | 10.049375 | 15 |
| 46 | 9.814900 | 9.879111 | 9.935589 | 10.064411 | 9.823539 | 9.872659 | 9.950879 | 10.049121 | 14 |
| 47 | 9.815046 | 9.879202 | 9.935844 | 10.064156 | 9.823680 | 9.872547 | 9.951133 | 10.048867 | 13 |
| 48 | 9.815193 | 9.879093 | 9.936100 | 10.063900 | 9.823821 | 9.872434 | 9.951388 | 10.048412 | 12 |
| 49 | 9.815339 | 9.878984 | 9.936355 | 10.063645 | 9.823963 | 9.872321 | 9.951642 | 10.048358 | 11 |
| 50 | 9.815485 | 9.878875 | 9.936611 | 10.063389 | 9.824104 | 9.872208 | 9.951896 | 10.048104 | 10 |
| 51 | 9.815632 | 9.878766 | 9.936860 | 10.063134 | 9.824245 | 9.872095 | 9.952150 | 10.047850 | 9 |
| 52 | 9.815778 | 9.878566 | 9.937121 | 10.062879 | 9.824386 | 9.871981 | 9.952405 | 10.047595 | 8 |
| 53 | 9.815924 | 9.878547 | 9.937377 | 10.062023 | 9.824527 | 9.871868 | 9.952659 | 10.047341 | 7 |
| 54 | 9.816069 | 9.878438 | 9.937632 | 10.062368 | 9.824668 | 9.871755 | 9.952913 | 10.047087 | 6 |
| 55 | 9.816215 | 9.878328 | 9.937875 | 10.062113 | 9.824808 | 9.871641 | 9.953167 | 10.046832 | 5 |
| 56 | 9.816361 | 9.878219 | 9.938142 | 10.061858 | 9.824949 | 9.871528 | 9.953421 | 10.046579 | 4 |
| 57 | 9.816507 | 9.878109 | 9.938398 | 10.061602 | 9.825090 | 9.871414 | 9.953675 | 10.046325 | 3 |
| 58 | 9.816652 | 9.877999 | 9.938653 | 10.061337 | 9.825230 | 9.871301 | 9.953929 | 10.046071 | 2 |
| 59 | 9.816798 | 9.877890 | 9.938908 | 10.061092 | 9.825371 | 9.871187 | 9.954183 | 10.045817 | 1 |
| 60 | 9.816943 | 9.877780 | 9.939163 | 10.060837 | 9.825511 | 9.871073 | 9.954437 | 10.045513 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | / |

42°

| | Sine | Cosine | Tan. | Cotan. | | Sine | Cosine | Tan. | Cotan. |
|----|----------|----------|-----------|-----------|----------|----------|----------|-----------|--------|
| 0 | 9.825511 | 9.871073 | 9.954437 | 10.04-563 | 9.833783 | 9.864127 | 9.969656 | 10.030344 | 60 |
| 1 | 9.25651 | 9.870960 | 9.954-.91 | 10.045309 | 9.833919 | 9.864010 | 9.969909 | 10.030091 | 59 |
| 2 | 9.825791 | 9.870846 | 9.954946 | 10.045054 | 9.834054 | 9.863892 | 9.970162 | 10.029-38 | 58 |
| 3 | 9.825391 | 9.870732 | 9.955200 | 10.044800 | 9.834189 | 9.863774 | 9.970416 | 10.0295-4 | 57 |
| 4 | 9.826071 | 9.870618 | 9.955454 | 10.044540 | 9.834325 | 9.863656 | 9.970669 | 10.029331 | 56 |
| 5 | 9.826211 | 9.870504 | 9.955708 | 10.044292 | 9.834460 | 9.863538 | 9.970922 | 10.029078 | 55 |
| 6 | 9.826351 | 9.870390 | 9.955961 | 10.044039 | 9.834595 | 9.863419 | 9.971175 | 10.028825 | 54 |
| 7 | 9.826491 | 9.870276 | 9.956215 | 10.043785 | 9.834730 | 9.863301 | 9.971429 | 10.028571 | 53 |
| 8 | 9.826631 | 9.870161 | 9.956469 | 10.043521 | 9.834865 | 9.863183 | 9.971682 | 10.028318 | 52 |
| 9 | 9.826770 | 9.870047 | 9.956723 | 10.043277 | 9.834999 | 9.863064 | 9.971935 | 10.028065 | 51 |
| 10 | 9.826910 | 9.869933 | 9.956977 | 10.043023 | 9.835184 | 9.862946 | 9.972188 | 10.027812 | 50 |
| 11 | 9.827049 | 9.869818 | 9.957231 | 10.042769 | 9.835269 | 9.862827 | 9.972441 | 10.027559 | 49 |
| 12 | 9.827189 | 9.869704 | 9.957485 | 10.042515 | 9.835403 | 9.862709 | 9.972695 | 10.027305 | 48 |
| 13 | 9.827328 | 9.869589 | 9.957739 | 10.042261 | 9.835538 | 9.862590 | 9.972948 | 10.027052 | 47 |
| 14 | 9.827467 | 9.869474 | 9.957993 | 10.042007 | 9.835672 | 9.862471 | 9.973201 | 10.026799 | 46 |
| 15 | 9.827606 | 9.869360 | 9.958247 | 10.041753 | 9.835807 | 9.862353 | 9.973454 | 10.026546 | 45 |
| 16 | 9.827745 | 9.869245 | 9.958500 | 10.041500 | 9.835941 | 9.862234 | 9.973707 | 10.026293 | 44 |
| 17 | 9.827884 | 9.869130 | 9.958754 | 10.041240 | 9.836075 | 9.862115 | 9.973960 | 10.026040 | 43 |
| 18 | 9.828023 | 9.869015 | 9.959008 | 10.040992 | 9.836209 | 9.861996 | 9.974213 | 10.025787 | 42 |
| 19 | 9.828162 | 9.868900 | 9.959262 | 10.040738 | 9.836343 | 9.861877 | 9.974466 | 10.025534 | 41 |
| 20 | 9.828301 | 9.868785 | 9.959516 | 10.040481 | 9.836477 | 9.861758 | 9.974720 | 10.025280 | 40 |
| 21 | 9.828439 | 9.868670 | 9.959769 | 10.040231 | 9.836611 | 9.861638 | 9.974973 | 10.025027 | 39 |
| 22 | 9.828578 | 9.868555 | 9.960023 | 10.039977 | 9.836745 | 9.861519 | 9.975226 | 10.024774 | 38 |
| 23 | 9.828716 | 9.868440 | 9.960277 | 10.039723 | 9.836878 | 9.861400 | 9.975479 | 10.024521 | 37 |
| 24 | 9.828855 | 9.868324 | 9.960530 | 10.039470 | 9.837012 | 9.861280 | 9.975732 | 10.024268 | 36 |
| 25 | 9.828993 | 9.868209 | 9.960784 | 10.039216 | 9.837146 | 9.861161 | 9.975995 | 10.024015 | 35 |
| 26 | 9.829131 | 9.868093 | 9.961038 | 10.038962 | 9.837279 | 9.861041 | 9.976238 | 10.023762 | 34 |
| 27 | 9.829269 | 9.867978 | 9.961292 | 10.038708 | 9.837412 | 9.860922 | 9.976191 | 10.023509 | 33 |
| 28 | 9.829407 | 9.867862 | 9.961545 | 10.038455 | 9.837546 | 9.860802 | 9.976744 | 10.023256 | 32 |
| 29 | 9.829545 | 9.867747 | 9.961799 | 10.038201 | 9.837679 | 9.860682 | 9.976997 | 10.023003 | 31 |
| 30 | 9.829683 | 9.867631 | 9.962052 | 10.037941 | 9.837812 | 9.860562 | 9.977250 | 10.022750 | 30 |
| 31 | 9.829821 | 9.867515 | 9.962306 | 10.037694 | 9.837945 | 9.860442 | 9.977503 | 10.022497 | 29 |
| 32 | 9.829959 | 9.867399 | 9.962560 | 10.037440 | 9.838078 | 9.860322 | 9.977766 | 10.022244 | 28 |
| 33 | 9.830097 | 9.867283 | 9.962813 | 10.037187 | 9.838211 | 9.860202 | 9.978009 | 10.021991 | 27 |
| 34 | 9.830234 | 9.867167 | 9.963067 | 10.036933 | 9.838344 | 9.860082 | 9.978262 | 10.021736 | 26 |
| 35 | 9.830372 | 9.867051 | 9.963320 | 10.036680 | 9.838477 | 9.859902 | 9.978515 | 10.021465 | 25 |
| 36 | 9.830509 | 9.866935 | 9.963574 | 10.036426 | 9.838610 | 9.859812 | 9.978768 | 10.021232 | 24 |
| 37 | 9.830646 | 9.866819 | 9.963828 | 10.036172 | 9.838742 | 9.859721 | 9.979021 | 10.020979 | 23 |
| 38 | 9.830784 | 9.866703 | 9.964061 | 10.035919 | 9.838875 | 9.859601 | 9.979274 | 10.020726 | 22 |
| 39 | 9.830921 | 9.866556 | 9.964305 | 10.035665 | 9.839007 | 9.859480 | 9.979527 | 10.020473 | 21 |
| 40 | 9.831058 | 9.866470 | 9.964588 | 10.035412 | 9.839140 | 9.859360 | 9.979760 | 10.020230 | 20 |
| 41 | 9.831195 | 9.866353 | 9.964842 | 10.035158 | 9.839272 | 9.859239 | 9.980033 | 10.019907 | 19 |
| 42 | 9.831332 | 9.866237 | 9.965095 | 10.034905 | 9.839404 | 9.859119 | 9.980286 | 10.019714 | 18 |
| 43 | 9.831469 | 9.866120 | 9.965349 | 10.034651 | 9.839536 | 9.858996 | 9.980538 | 10.019462 | 17 |
| 44 | 9.831606 | 9.866004 | 9.965602 | 10.034398 | 9.839668 | 9.858877 | 9.980791 | 10.019209 | 16 |
| 45 | 9.831742 | 9.865887 | 9.965855 | 10.034145 | 9.839800 | 9.858756 | 9.981044 | 10.018956 | 15 |
| 46 | 9.831879 | 9.865770 | 9.966109 | 10.033891 | 9.839932 | 9.858633 | 9.981297 | 10.018708 | 14 |
| 47 | 9.832015 | 9.865563 | 9.966362 | 10.033638 | 9.840164 | 9.858514 | 9.981550 | 10.018450 | 13 |
| 48 | 9.832152 | 9.865536 | 9.966616 | 10.033384 | 9.840196 | 9.858393 | 9.981803 | 10.018197 | 12 |
| 49 | 9.832288 | 9.865419 | 9.966869 | 10.033131 | 9.840328 | 9.858272 | 9.982056 | 10.017944 | 11 |
| 50 | 9.832425 | 9.865302 | 9.967123 | 10.032877 | 9.840459 | 9.858151 | 9.982309 | 10.017691 | 10 |
| 51 | 9.832561 | 9.865185 | 9.967376 | 10.032624 | 9.840591 | 9.858029 | 9.982552 | 10.017438 | 9 |
| 52 | 9.832697 | 9.865068 | 9.967629 | 10.032371 | 9.840722 | 9.857908 | 9.982814 | 10.017186 | 8 |
| 53 | 9.832833 | 9.864950 | 9.967883 | 10.032117 | 9.840854 | 9.857766 | 9.983067 | 10.016933 | 7 |
| 54 | 9.832969 | 9.864833 | 9.968136 | 10.031964 | 9.840985 | 9.857665 | 9.983320 | 10.016680 | 6 |
| 55 | 9.833105 | 9.864716 | 9.968389 | 10.031811 | 9.841116 | 9.857543 | 9.983573 | 10.016427 | 5 |
| 56 | 9.833241 | 9.864698 | 9.968643 | 10.031357 | 9.841247 | 9.857422 | 9.983826 | 10.016174 | 4 |
| 57 | 9.833377 | 9.864481 | 9.968896 | 10.031104 | 9.841378 | 9.857300 | 9.984079 | 10.015921 | 3 |
| 58 | 9.833512 | 9.864363 | 9.969149 | 10.030851 | 9.841509 | 9.857178 | 9.984332 | 10.015668 | 2 |
| 59 | 9.833648 | 9.864245 | 9.969403 | 10.030597 | 9.841640 | 9.857056 | 9.984584 | 10.015416 | 1 |
| 60 | 9.833783 | 9.864127 | 9.969656 | 10.030344 | 9.841771 | 9.856934 | 9.984837 | 10.015163 | 0 |
| | Cosine | Sine | Cotan. | Tan. | Cosine | Sine | Cotan. | Tan. | |

47°

46°

44°

| | Sine | Cosine | Tan. | Cotan. |
|----|----------|----------|-----------|-----------|
| 0 | 9'841771 | 9'856934 | 9'984887 | 10'015163 |
| 1 | 9'841902 | 9'856812 | 9'985090 | 10'014910 |
| 2 | 9'842033 | 9'856690 | 9'985343 | 10'014657 |
| 3 | 9'842163 | 9'856568 | 9'985596 | 10'014404 |
| 4 | 9'842294 | 9'856446 | 9'985848 | 10'014152 |
| 5 | 9'842424 | 9'856323 | 9'986101 | 10'013899 |
| 6 | 9'842555 | 9'856201 | 9'986354 | 10'013646 |
| 7 | 9'842685 | 9'856076 | 9'986607 | 10'013393 |
| 8 | 9'842815 | 9'855956 | 9'986860 | 10'013140 |
| 9 | 9'842946 | 9'855833 | 9'987112 | 10'012888 |
| 10 | 9'843076 | 9'855711 | 9'987365 | 10'012635 |
| 11 | 9'843206 | 9'855588 | 9'987618 | 10'012382 |
| 12 | 9'843336 | 9'855465 | 9'987871 | 10'012129 |
| — | | | | |
| 13 | 9'843466 | 9'855342 | 9'988123 | 10'011877 |
| 14 | 9'843595 | 9'855219 | 9'988376 | 10'011624 |
| 15 | 9'843725 | 9'855096 | 9'988629 | 10'011371 |
| 16 | 9'843855 | 9'854973 | 9'988882 | 10'011118 |
| 17 | 9'843984 | 9'854850 | 9'989134 | 10'010866 |
| 18 | 9'844114 | 9'854727 | 9'989387 | 10'010613 |
| 19 | 9'844243 | 9'854603 | 9'989640 | 10'010360 |
| 20 | 9'844372 | 9'854480 | 9'989893 | 10'010107 |
| 21 | 9'844502 | 9'854356 | 9'990145 | 10'009555 |
| 22 | 9'844631 | 9'854233 | 9'990398 | 10'009602 |
| 23 | 9'844760 | 9'854109 | 9'990651 | 10'009349 |
| 24 | 9'844889 | 9'853986 | 9'990903 | 10'009097 |
| — | | | | |
| 25 | 9'845018 | 9'853862 | 9'991156 | 10'008844 |
| 26 | 9'845147 | 9'853738 | 9'991409 | 10'008591 |
| 27 | 9'845276 | 9'853614 | 9'991662 | 10'008338 |
| 28 | 9'845405 | 9'853490 | 9'991914 | 10'008086 |
| 29 | 9'845533 | 9'853366 | 9'992167 | 10'007833 |
| 30 | 9'845662 | 9'853242 | 9'992420 | 10'007580 |
| 31 | 9'845790 | 9'853118 | 9'992672 | 10'007328 |
| 32 | 9'845919 | 9'852994 | 9'992925 | 10'007075 |
| 33 | 9'846047 | 9'852869 | 9'993178 | 10'006822 |
| 34 | 9'846175 | 9'852745 | 9'993431 | 10'006569 |
| 35 | 9'846304 | 9'852620 | 9'993683 | 10'006317 |
| 36 | 9'846132 | 9'852496 | 9'993936 | 10'006064 |
| — | | | | |
| 37 | 9'846560 | 9'852371 | 9'994189 | 10'005811 |
| 38 | 9'846688 | 9'852247 | 9'994441 | 10'005559 |
| 39 | 9'846816 | 9'852122 | 9'994694 | 10'005306 |
| 40 | 9'846944 | 9'851997 | 9'994947 | 10'005053 |
| 41 | 9'847071 | 9'851872 | 9'995199 | 10'004801 |
| 42 | 9'847199 | 9'851747 | 9'995452 | 10'004548 |
| 43 | 9'847327 | 9'851622 | 9'995705 | 10'004295 |
| 44 | 9'847454 | 9'851497 | 9'995957 | 10'004043 |
| 45 | 9'847582 | 9'851372 | 9'996210 | 10'003790 |
| 46 | 9'847709 | 9'851246 | 9'996463 | 10'003537 |
| 47 | 9'847836 | 9'851121 | 9'996715 | 10'003285 |
| 48 | 9'847964 | 9'850996 | 9'996968 | 10'003032 |
| — | | | | |
| 49 | 9'848091 | 9'850870 | 9'997221 | 10.002779 |
| 50 | 9'848218 | 9'850745 | 9'997473 | 10.002527 |
| 51 | 9'848345 | 9'850619 | 9'997726 | 10.002274 |
| 52 | 9'848472 | 9'850493 | 9'997979 | 10.002021 |
| 53 | 9'848599 | 9'850368 | 9'998231 | 10'001769 |
| 54 | 9'848726 | 9'850242 | 9'998484 | 10'001516 |
| 55 | 9'848852 | 9'850116 | 9'998737 | 10'001263 |
| 56 | 9'848979 | 9'849990 | 9'998989 | 10'001011 |
| 57 | 9'849106 | 9'849864 | 9'999242 | 10'000758 |
| 58 | 9'849232 | 9'849738 | 9'999496 | 10'000505 |
| 59 | 9'849358 | 9'849611 | 9'999747 | 10'000253 |
| 60 | 9'849495 | 9'849485 | 10.000000 | 10'000000 |
| | Cosine | Sine | Cotan. | Tan. |

45°

TRAVERSE TABLES,

CALCULATED TO ANY NUMBER

OR

CHAINS, OR LINKS OF DISTANCE,

AND TO

THREE MINUTES OF THE ANGLE OF BEARING.*

* The principle and method of using these tables will be found in the Third Part, page 193.

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| / | Lat. | Dep. | Lat. | Dtp. | Lat. | Dep. | Lat. | Dep. | Lat. | Dep. | / |
| 3 | 99° 0' 2 | 1° 99° 105' | 2° 99° 157' | 3° 99° 209' | 4° 99° 261' | 5° 99° 314' | 6° 99° 366' | 7° 99° 418' | 8° 99° 471' | 9° 99° 523' | 87 |
| 3 | 99° 0' 53 | 1° 99° 106' | 2° 99° 160' | 3° 99° 213' | 4° 99° 266' | 5° 99° 319' | 6° 99° 372' | 7° 99° 426' | 8° 99° 479' | 9° 99° 532' | 57 |
| 6 | 99° 0' 54 | 1° 99° 108' | 2° 99° 162' | 3° 99° 216' | 4° 99° 271' | 5° 99° 325' | 6° 99° 379' | 7° 99° 433' | 8° 99° 487' | 9° 99° 541' | 54 |
| 9 | 99° 0' 55 | 1° 99° 110' | 2° 99° 165' | 3° 99° 220' | 4° 99° 275' | 5° 99° 330' | 6° 99° 385' | 7° 99° 440' | 8° 99° 495' | 9° 99° 550' | 48 |
| 12 | 99° 0' 56 | 1° 99° 112' | 2° 99° 167' | 3° 99° 223' | 4° 99° 279' | 5° 99° 335' | 6° 99° 391' | 7° 99° 446' | 8° 99° 502' | 9° 99° 558' | 48 |
| 9 | 99° 0' 57 | 1° 99° 113' | 2° 99° 170' | 3° 99° 227' | 4° 99° 283' | 5° 99° 340' | 6° 99° 397' | 7° 99° 464' | 8° 99° 510' | 9° 99° 567' | 45 |
| 11 | 99° 0' 58 | 1° 99° 115' | 2° 99° 173' | 3° 99° 230' | 4° 99° 288' | 5° 99° 345' | 6° 99° 403' | 7° 99° 461' | 8° 99° 518' | 9° 99° 575' | 42 |
| 21 | 99° 0' 58 | 1° 99° 117' | 2° 99° 175' | 3° 99° 234' | 4° 99° 292' | 5° 99° 350' | 6° 99° 409' | 7° 99° 467' | 8° 99° 526' | 9° 99° 584' | 39 |
| 24 | 99° 0' 59 | 1° 99° 119' | 2° 99° 178' | 3° 99° 237' | 4° 99° 297' | 5° 99° 356' | 6° 99° 415' | 7° 99° 475' | 8° 99° 534' | 9° 99° 593' | 36 |
| 27 | 99° 0' 60 | 1° 99° 120' | 2° 99° 180' | 3° 99° 240' | 4° 99° 300' | 5° 99° 361' | 6° 99° 421' | 7° 99° 481' | 8° 99° 541' | 9° 99° 601' | 33 |
| 30 | 99° 0' 61 | 1° 99° 122' | 2° 99° 183' | 3° 99° 244' | 4° 99° 305' | 5° 99° 366' | 6° 99° 427' | 7° 99° 489' | 8° 99° 549' | 9° 99° 610' | 30 |
| 3 | 99° 0' 62 | 1° 99° 124' | 2° 99° 186' | 3° 99° 245' | 4° 99° 310' | 5° 99° 371' | 6° 99° 433' | 7° 99° 495' | 8° 99° 557' | 9° 99° 619' | 27 |
| 3 | 99° 0' 63 | 1° 99° 126' | 2° 99° 189' | 3° 99° 251' | 4° 99° 314' | 5° 99° 377' | 6° 99° 440' | 7° 99° 502' | 8° 99° 565' | 9° 99° 626' | 24 |
| 89 | 99° 0' 64 | 1° 99° 127' | 2° 99° 191' | 3° 99° 254' | 4° 99° 318' | 5° 99° 382' | 6° 99° 445' | 7° 99° 509' | 8° 99° 572' | 9° 99° 636' | 21 |
| 42 | 99° 0' 64 | 1° 99° 129' | 2° 99° 193' | 3° 99° 258' | 4° 99° 323' | 5° 99° 387' | 6° 99° 452' | 7° 99° 516' | 8° 99° 581' | 9° 99° 645' | 18 |
| 45 | 99° 0' 65 | 1° 99° 131' | 2° 99° 196' | 3° 99° 262' | 4° 99° 327' | 5° 99° 392' | 6° 99° 458' | 7° 99° 523' | 8° 99° 589' | 9° 99° 654' | 15 |
| 4 | 99° 0' 66 | 1° 99° 132' | 2° 99° 199' | 3° 99° 265' | 4° 99° 331' | 5° 99° 397' | 6° 99° 463' | 7° 99° 530' | 8° 99° 596' | 9° 99° 662' | 12 |
| 51 | 99° 0' 67 | 1° 99° 134' | 2° 99° 201' | 3° 99° 268' | 4° 99° 336' | 5° 99° 403' | 6° 99° 470' | 7° 99° 537' | 8° 99° 604' | 9° 99° 671' | 9 |
| 54 | 99° 0' 68 | 1° 99° 136' | 2° 99° 204' | 3° 99° 272' | 4° 99° 340' | 5° 99° 408' | 6° 99° 476' | 7° 99° 544' | 8° 99° 612' | 9° 99° 680' | 6 |
| 57 | 99° 0' 69 | 1° 99° 138' | 2° 99° 207' | 3° 99° 276' | 4° 99° 345' | 5° 99° 413' | 6° 99° 482' | 7° 99° 551' | 8° 99° 620' | 9° 99° 689' | 3 |
| 4 | 99° 0' 70 | 1° 99° 139' | 2° 99° 209' | 3° 99° 279' | 4° 99° 349' | 5° 99° 418' | 6° 99° 488' | 7° 99° 558' | 8° 99° 627' | 9° 99° 697' | 86 |
| 3 | 99° 0' 71 | 1° 99° 141' | 2° 99° 212' | 3° 99° 282' | 4° 99° 353' | 5° 99° 424' | 6° 99° 494' | 7° 99° 565' | 8° 99° 636' | 9° 99° 706' | 57 |
| 6 | 99° 0' 71 | 1° 99° 143' | 2° 99° 214' | 3° 99° 286' | 4° 99° 357' | 5° 99° 429' | 6° 99° 500' | 7° 99° 572' | 8° 99° 643' | 9° 99° 717' | 51 |
| 9 | 99° 0' 72 | 1° 99° 145' | 2° 99° 217' | 3° 99° 289' | 4° 99° 362' | 5° 99° 434' | 6° 99° 507' | 7° 99° 579' | 8° 99° 651' | 9° 99° 724' | 51 |
| 12 | 99° 0' 73 | 1° 99° 146' | 2° 99° 220' | 3° 99° 293' | 4° 99° 366' | 5° 99° 439' | 6° 99° 513' | 7° 99° 586' | 8° 99° 659' | 9° 99° 732' | 48 |
| 15 | 99° 0' 74 | 1° 99° 148' | 2° 99° 222' | 3° 99° 296' | 4° 99° 371' | 5° 99° 445' | 6° 99° 519' | 7° 99° 593' | 8° 99° 667' | 9° 99° 747' | 45 |
| 18 | 99° 0' 75 | 1° 99° 150' | 2° 99° 225' | 3° 99° 300' | 4° 99° 375' | 5° 99° 450' | 6° 99° 528' | 7° 99° 600' | 8° 99° 675' | 9° 99° 750' | 42 |
| 21 | 99° 0' 76 | 1° 99° 152' | 2° 99° 227' | 3° 99° 303' | 4° 99° 379' | 5° 99° 455' | 6° 99° 531' | 7° 99° 607' | 8° 99° 683' | 9° 99° 758' | 39 |
| 24 | 99° 0' 77 | 1° 99° 153' | 2° 99° 230' | 3° 99° 307' | 4° 99° 384' | 5° 99° 460' | 6° 99° 537' | 7° 99° 614' | 8° 99° 690' | 9° 99° 767' | 36 |
| 27 | 99° 0' 78 | 1° 99° 155' | 2° 99° 233' | 3° 99° 310' | 4° 99° 388' | 5° 99° 466' | 6° 99° 543' | 7° 99° 621' | 8° 99° 698' | 9° 99° 776' | 33 |
| 30 | 99° 0' 78 | 1° 99° 157' | 2° 99° 235' | 3° 99° 314' | 4° 99° 392' | 5° 99° 470' | 6° 99° 549' | 7° 99° 627' | 8° 99° 707' | 9° 99° 784' | 30 |
| 33 | 99° 0' 79 | 1° 99° 159' | 2° 99° 238' | 3° 99° 317' | 4° 99° 397' | 5° 99° 476' | 6° 99° 558' | 7° 99° 634' | 8° 99° 714' | 9° 99° 793' | 27 |
| 36 | 99° 0' 80 | 1° 99° 160' | 2° 99° 241' | 3° 99° 321' | 4° 99° 401' | 5° 99° 481' | 6° 99° 561' | 7° 99° 642' | 8° 99° 722' | 9° 99° 802' | 24 |
| 39 | 99° 0' 81 | 1° 99° 162' | 2° 99° 243' | 3° 99° 324' | 4° 99° 406' | 5° 99° 486' | 6° 99° 568' | 7° 99° 649' | 8° 99° 730' | 9° 99° 811' | 21 |
| 42 | 99° 0' 82 | 1° 99° 164' | 2° 99° 246' | 3° 99° 328' | 4° 99° 410' | 5° 99° 491' | 6° 99° 573' | 7° 99° 655' | 8° 99° 737' | 9° 99° 819' | 18 |
| 45 | 99° 0' 83 | 1° 99° 166' | 2° 99° 248' | 3° 99° 331' | 4° 99° 414' | 5° 99° 498' | 6° 99° 580' | 7° 99° 662' | 8° 99° 745' | 9° 99° 828' | 15 |
| 48 | 99° 0' 84 | 1° 99° 167' | 2° 99° 251' | 3° 99° 335' | 4° 99° 419' | 5° 99° 498' | 6° 99° 586' | 7° 99° 670' | 8° 99° 753' | 9° 99° 837' | 12 |
| 51 | 99° 0' 85 | 1° 99° 169' | 2° 99° 253' | 3° 99° 338' | 4° 99° 423' | 5° 99° 497' | 6° 99° 592' | 7° 99° 676' | 8° 99° 767' | 9° 99° 846' | 9 |
| 54 | 99° 0' 85 | 1° 99° 171' | 2° 99° 256' | 3° 99° 342' | 4° 99° 427' | 5° 99° 412' | 6° 99° 598' | 7° 99° 683' | 8° 99° 766' | 9° 99° 854' | 6 |
| 57 | 99° 0' 86 | 1° 99° 172' | 2° 99° 259' | 3° 99° 345' | 4° 99° 431' | 5° 99° 417' | 6° 99° 603' | 7° 99° 690' | 8° 99° 776' | 9° 99° 862' | 3 |
| 3 | 99° 0' 87 | 1° 99° 174' | 2° 99° 261' | 3° 99° 349' | 4° 99° 436' | 5° 99° 523' | 6° 99° 610' | 7° 99° 697' | 8° 99° 784' | 9° 99° 872' | 85 |
| 3 | 99° 0' 88 | 1° 99° 176' | 2° 99° 264' | 3° 99° 352' | 4° 99° 440' | 5° 99° 528' | 6° 99° 616' | 7° 99° 704' | 8° 99° 792' | 9° 99° 880' | 57 |
| 6 | 99° 0' 89 | 1° 99° 178' | 2° 99° 267' | 3° 99° 356' | 4° 99° 445' | 5° 99° 533' | 6° 99° 622' | 7° 99° 711' | 8° 99° 790' | 9° 99° 889' | 54 |
| 9 | 99° 0' 90 | 1° 99° 179' | 2° 99° 269' | 3° 99° 359' | 4° 99° 448' | 5° 99° 538' | 6° 99° 628' | 7° 99° 718' | 8° 99° 796' | 9° 99° 897' | 51 |
| 12 | 99° 0' 91 | 1° 99° 181' | 2° 99° 272' | 3° 99° 362' | 4° 99° 453' | 5° 99° 543' | 6° 99° 634' | 7° 99° 725' | 8° 99° 815' | 9° 99° 906' | 48 |
| 15 | 99° 0' 91 | 1° 99° 183' | 2° 99° 274' | 3° 99° 366' | 4° 99° 457' | 5° 99° 547' | 6° 99° 640' | 7° 99° 732' | 8° 99° 828' | 9° 99° 915' | 45 |
| 18 | 99° 0' 92 | 1° 99° 185' | 2° 99° 277' | 3° 99° 369' | 4° 99° 462' | 5° 99° 554' | 6° 99° 647' | 7° 99° 739' | 8° 99° 831' | 9° 99° 924' | 42 |
| 21 | 99° 0' 93 | 1° 99° 186' | 2° 99° 280' | 3° 99° 373' | 4° 99° 466' | 5° 99° 559' | 6° 99° 652' | 7° 99° 745' | 8° 99° 836' | 9° 99° 932' | 39 |
| 24 | 99° 0' 94 | 1° 99° 188' | 2° 99° 282' | 3° 99° 376' | 4° 99° 470' | 5° 99° 563' | 6° 99° 659' | 7° 99° 753' | 8° 99° 847' | 9° 99° 941' | 36 |
| 27 | 99° 0' 95 | 1° 99° 190' | 2° 99° 285' | 3° 99° 380' | 4° 99° 475' | 5° 99° 567' | 6° 99° 665' | 7° 99° 760' | 8° 99° 855' | 9° 99° 950' | 33 |
| 30 | 99° 0' 95 | 1° 99° 191' | 2° 99° 287' | 3° 99° 383' | 4° 99° 478' | 5° 99° 575' | 6° 99° 671' | 7° 99° 766' | 8° 99° 862' | 9° 99° 958' | 30 |
| 33 | 99° 0' 95 | 1° 99° 193' | 2° 99° 290' | 3° 99° 387' | 4° 99° 483' | 5° 99° 580' | 6° 99° 677' | 7° 99° 774' | 8° 99° 870' | 9° 99° 967' | 27 |
| 36 | 99° 0' 95 | 1° 99° 195' | 2° 99° 293' | 3° 99° 390' | 4° 99° 488' | 5° 99° 587' | 6° 99° 683' | 7° 99° 781' | 8° 99° 878' | 9° 99° 975' | 24 |
| 39 | 99° 0' 95 | 1° 99° 197' | 2° 99° 295' | 3° 99° 393' | 4° 99° 492' | 5° 99° 591' | 6° 99° 689' | 7° 99° 787' | 8° 99° 886' | 9° 99° 984' | 21 |
| 42 | 99° 0' 95 | 1° 99° 199' | 2° 99° 298' | 3° 99° 397' | 4° 99° 497' | 5° 99° 597' | 6° 99° 695' | 7° 99° 794' | 8° 99° 894' | 9° 99° 993' | 18 |
| 45 | 99° 0' 95 | 1° 99° 200' | 2° 99° 300' | 3° 99° 400' | 4° 99° 501' | 5° 99° 601' | 6° 99° 701' | 7° 99° 801' | 8° 99° 902' | 9° 99° 100' | 15 |
| 48 | 99° 0' 95 | 1° 99° 202' | 2° 99° 303' | 3° 99° 404' | 4° 99° 505' | 5° 99° 607' | 6° 99° 707' | 7° 99° 808' | 8° 99° 905' | 9° 99° 101' | 12 |
| 51 | 99° 0' 95 | 1° 99° 204' | 2° 99° 306' | 3° 99° 408' | 4° 99° 510' | 5° 99° 612' | 6° 99° 714' | 7° 99° 816' | 8° 99° 915' | 9° 99° 102' | 9 |
| 54 | 99° 0' 95 | 1° 99° 205' | 2° 99° 308' | 3° 99° 411' | 4° 99° 513' | 5° 99° 616' | 6° 99° 719' | 7° 99° 822' | 8° 99° 924' | 9° 99° 103' | 6 |
| 57 | 99° 0' 95 | 1° 99° 207' | 2° 99° 311' | 3° 99° 415' | 4° 99° 518' | 5° 99° 622' | 6° 99° 726' | 7° 99° 830' | 8° 99° 933' | 9° 99° 104' | 3 |
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| 15 | 981-195 | 196-390 | 294-585 | 3-92-780 | 490-975 | 588-117 | 687-136 | 784-156 | 883-175 | 981-195 | 45 |
| 18 | 981-196 | 196-392 | 294-588 | 3-92-784 | 490-980 | 588-117 | 686-137 | 784-157 | 883-176 | 981-196 | 42 |
| 21 | 980-197 | 196-394 | 294-590 | 3-92-787 | 490-984 | 588-118 | 686-138 | 784-157 | 882-177 | 980-197 | 39 |
| 24 | 980-198 | 196-395 | 294-593 | 3-92-790 | 490-988 | 588-118 | 686-138 | 784-158 | 882-178 | 980-198 | 36 |
| 27 | 980-198 | 196-397 | 294-595 | 3-92-794 | 490-992 | 588-119 | 686-138 | 784-159 | 882-179 | 980-198 | 33 |
| 30 | 980-199 | 196-398 | 294-598 | 3-92-797 | 490-997 | 588-120 | 686-139 | 784-159 | 882-179 | 980-199 | 30 |
| 33 | 980-200 | 196-400 | 294-600 | 3-92-801 | 490-1000 | 588-120 | 686-140 | 784-160 | 882-180 | 980-200 | 27 |
| 36 | 980-201 | 196-402 | 294-603 | 3-92-804 | 490-100 | 588-120 | 686-141 | 784-160 | 882-181 | 980-201 | 24 |
| 39 | 979-202 | 196-404 | 294-606 | 3-92-808 | 490-101 | 588-121 | 686-141 | 784-161 | 881-182 | 979-202 | 21 |
| 42 | 979-203 | 196-406 | 294-608 | 3-92-811 | 490-101 | 588-122 | 685-142 | 783-162 | 881-182 | 979-203 | 18 |
| 45 | 979-204 | 196-407 | 294-611 | 3-92-814 | 490-102 | 587-122 | 685-143 | 783-163 | 881-183 | 979-204 | 15 |
| 48 | 979-204 | 196-409 | 294-613 | 3-91-818 | 489-102 | 587-123 | 685-143 | 783-164 | 881-184 | 979-204 | 12 |
| 51 | 979-205 | 196-410 | 294-616 | 3-91-821 | 489-102 | 587-123 | 685-144 | 783-164 | 881-185 | 979-205 | 9 |
| 54 | 979-206 | 196-412 | 293-618 | 3-91-825 | 489-103 | 587-124 | 685-144 | 783-165 | 881-185 | 979-206 | 6 |
| 57 | 978-207 | 196-414 | 293-621 | 3-91-828 | 489-103 | 587-124 | 685-145 | 783-165 | 880-186 | 978-207 | 3 |

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| | Lat. | Dep | Lat. | Dep | Lat. | Dep | Lat. | Dep | Lat. | Dep | |
| 12° | 978-208 | 196-416 | 293-624 | 3-91-832 | 4-89-104 | 5-87-125 | 6-85-146 | 7-82-166 | 8-80-187 | 9-78-208 | 78 |
| 3 | 978-209 | 196-418 | 293-626 | 3-91-835 | 4-89-104 | 5-87-125 | 6-85-146 | 7-82-167 | 8-80-188 | 9-78-209 | 57 |
| 6 | 978-210 | 196-419 | 293-629 | 3-91-838 | 4-89-105 | 5-87-126 | 6-84-147 | 7-82-168 | 8-80-189 | 9-78-210 | 54 |
| 9 | 978-211 | 196-421 | 293-633 | 3-91-841 | 4-89-105 | 5-87-126 | 6-84-147 | 7-82-168 | 8-80-190 | 9-78-210 | 51 |
| 12 | 977-211 | 195-421 | 293-634 | 3-91-845 | 4-89-106 | 5-86-127 | 6-84-148 | 7-82-169 | 8-80-191 | 9-77-211 | 48 |
| 15 | 977-212 | 195-424 | 293-637 | 3-91-849 | 4-89-106 | 5-86-127 | 6-84-149 | 7-82-170 | 8-80-191 | 9-77-212 | 45 |
| 18 | 977-213 | 195-426 | 293-639 | 3-91-852 | 4-89-107 | 5-86-128 | 6-84-149 | 7-82-170 | 8-79-192 | 9-77-213 | 42 |
| 21 | 977-214 | 195-428 | 293-642 | 3-91-855 | 4-88-107 | 5-86-128 | 6-84-150 | 7-81-171 | 8-79-192 | 9-77-214 | 39 |
| 24 | 977-215 | 195-429 | 293-644 | 3-91-859 | 4-88-107 | 5-86-129 | 6-84-150 | 7-81-172 | 8-79-193 | 9-77-215 | 36 |
| 27 | 977-216 | 195-431 | 293-647 | 3-91-862 | 4-88-108 | 5-86-129 | 6-84-151 | 7-81-172 | 8-79-194 | 9-76-216 | 33 |
| 30 | 976-216 | 195-433 | 293-649 | 3-91-866 | 4-88-108 | 5-86-130 | 6-83-152 | 7-81-173 | 8-79-195 | 9-76-216 | 30 |
| 33 | 976-217 | 195-435 | 293-651 | 3-90-869 | 4-88-109 | 5-86-130 | 6-83-152 | 7-81-174 | 8-78-196 | 9-76-217 | 27 |
| 36 | 976-218 | 195-436 | 293-654 | 3-90-873 | 4-88-109 | 5-86-131 | 6-83-153 | 7-81-175 | 8-78-196 | 9-76-218 | 24 |
| 39 | 976-219 | 195-438 | 293-657 | 3-90-876 | 4-88-109 | 5-86-131 | 6-83-153 | 7-81-175 | 8-78-197 | 9-76-219 | 21 |
| 42 | 976-220 | 195-440 | 293-660 | 3-90-879 | 4-88-110 | 5-86-132 | 6-83-154 | 7-80-176 | 8-78-198 | 9-76-220 | 18 |
| 45 | 975-221 | 195-441 | 293-662 | 3-90-883 | 4-88-110 | 5-86-132 | 6-83-154 | 7-80-177 | 8-78-199 | 9-75-221 | 15 |
| 48 | 975-222 | 195-443 | 293-665 | 3-90-886 | 4-88-111 | 5-86-133 | 6-83-155 | 7-80-177 | 8-78-199 | 9-75-222 | 12 |
| 51 | 975-222 | 195-446 | 292-667 | 3-90-890 | 4-87-111 | 5-86-133 | 6-82-156 | 7-80-178 | 8-77-200 | 9-75-222 | 9 |
| 54 | 975-223 | 195-447 | 292-670 | 3-90-893 | 4-87-112 | 5-86-134 | 6-82-156 | 7-80-179 | 8-77-201 | 9-75-223 | 6 |
| 57 | 975-224 | 195-448 | 292-672 | 3-90-896 | 4-87-112 | 5-86-134 | 6-82-157 | 7-80-179 | 8-77-202 | 9-75-224 | 3 |
| 13° | 974-226 | 195-450 | 292-675 | 3-90-900 | 4-87-112 | 5-85-135 | 6-82-157 | 7-80-180 | 8-77-202 | 9-74-225 | 77 |
| 3 | 974-226 | 195-452 | 292-677 | 3-90-903 | 4-87-113 | 5-84-135 | 6-82-158 | 7-79-181 | 8-77-203 | 9-74-226 | 57 |
| 6 | 974-227 | 195-453 | 292-680 | 3-90-907 | 4-87-113 | 5-84-136 | 6-82-159 | 7-79-181 | 8-76-204 | 9-74-227 | 54 |
| 9 | 974-227 | 195-455 | 292-682 | 3-90-910 | 4-87-114 | 5-84-136 | 6-82-159 | 7-79-181 | 8-76-205 | 9-74-227 | 51 |
| 12 | 974-228 | 195-457 | 292-685 | 3-89-913 | 4-87-114 | 5-84-137 | 6-81-160 | 7-79-183 | 8-76-206 | 9-74-228 | 48 |
| 15 | 974-229 | 195-458 | 292-688 | 3-89-917 | 4-87-115 | 5-84-138 | 6-81-160 | 7-79-183 | 8-76-206 | 9-73-229 | 45 |
| 18 | 973-230 | 195-460 | 292-690 | 3-89-920 | 4-87-115 | 5-84-138 | 6-81-161 | 7-78-183 | 8-76-207 | 9-73-230 | 42 |
| 21 | 973-231 | 195-462 | 292-693 | 3-89-924 | 4-86-115 | 5-84-138 | 6-81-162 | 7-78-183 | 8-76-208 | 9-73-231 | 39 |
| 24 | 973-232 | 195-463 | 292-695 | 3-89-927 | 4-86-116 | 5-84-138 | 6-81-162 | 7-78-183 | 8-76-208 | 9-73-232 | 36 |
| 27 | 973-233 | 194-466 | 292-699 | 3-89-931 | 4-86-116 | 5-83-140 | 6-81-163 | 7-78-186 | 8-75-210 | 9-73-233 | 33 |
| 30 | 972-233 | 194-467 | 292-700 | 3-89-934 | 4-86-117 | 5-83-140 | 6-81-163 | 7-78-186 | 8-75-210 | 9-72-233 | 30 |
| 33 | 972-234 | 194-468 | 292-702 | 3-89-937 | 4-86-117 | 5-83-141 | 6-81-164 | 7-78-187 | 8-75-211 | 9-72-234 | 27 |
| 36 | 972-235 | 194-470 | 292-705 | 3-89-940 | 4-86-118 | 5-83-141 | 6-80-165 | 7-78-188 | 8-75-212 | 9-72-235 | 24 |
| 39 | 972-236 | 194-472 | 291-708 | 3-89-944 | 4-86-118 | 5-83-142 | 6-80-165 | 7-77-189 | 8-75-212 | 9-72-236 | 21 |
| 42 | 972-237 | 194-474 | 291-710 | 3-89-947 | 4-86-118 | 5-83-142 | 6-80-166 | 7-77-189 | 8-74-213 | 9-72-237 | 18 |
| 45 | 971-238 | 194-475 | 291-713 | 3-89-951 | 4-86-119 | 5-83-143 | 6-80-166 | 7-77-190 | 8-74-214 | 9-71-238 | 15 |
| 48 | 971-239 | 194-478 | 291-716 | 3-89-955 | 4-86-119 | 5-83-143 | 6-80-167 | 7-77-191 | 8-74-215 | 9-71-239 | 12 |
| 51 | 971-239 | 194-479 | 291-718 | 3-88-957 | 4-85-120 | 5-82-144 | 6-80-167 | 7-77-191 | 8-74-215 | 9-71-239 | 9 |
| 54 | 971-240 | 194-480 | 291-721 | 3-88-960 | 4-85-120 | 5-82-144 | 6-79-168 | 7-77-192 | 8-74-216 | 9-71-240 | 6 |
| 57 | 971-241 | 194-482 | 291-723 | 3-88-964 | 4-85-121 | 5-82-145 | 6-79-169 | 7-76-193 | 8-73-217 | 9-70-241 | 3 |
| 14° | 970-242 | 194-484 | 291-726 | 3-88-968 | 4-85-121 | 5-82-145 | 6-79-169 | 7-76-193 | 8-73-218 | 9-70-242 | 76 |
| 3 | 970-243 | 194-485 | 291-725 | 3-88-971 | 4-85-121 | 5-82-146 | 6-79-170 | 7-76-191 | 8-73-218 | 9-70-243 | 57 |
| 6 | 970-243 | 194-487 | 291-730 | 3-88-974 | 4-85-122 | 5-82-146 | 6-79-170 | 7-76-193 | 8-73-219 | 9-70-244 | 54 |
| 9 | 970-244 | 194-489 | 291-733 | 3-87-978 | 4-85-122 | 5-82-147 | 6-79-171 | 7-76-196 | 8-73-220 | 9-70-244 | 51 |
| 12 | 970-245 | 194-491 | 291-746 | 3-88-981 | 4-85-123 | 5-82-147 | 6-79-172 | 7-76-196 | 8-73-221 | 9-69-245 | 48 |
| 15 | 969-246 | 194-492 | 291-748 | 3-88-985 | 4-85-123 | 5-82-148 | 6-78-172 | 7-75-197 | 8-73-222 | 9-69-246 | 45 |
| 18 | 969-247 | 194-494 | 291-741 | 3-88-988 | 4-85-123 | 5-81-148 | 6-78-173 | 7-75-197 | 8-72-222 | 9-69-247 | 42 |
| 21 | 969-248 | 194-496 | 291-743 | 3-87-991 | 4-84-124 | 5-81-149 | 6-78-173 | 7-75-198 | 8-72-223 | 9-69-248 | 39 |
| 24 | 969-249 | 194-497 | 291-746 | 3-87-995 | 4-84-124 | 5-81-149 | 6-78-174 | 7-75-199 | 8-72-224 | 9-69-249 | 36 |
| 27 | 968-250 | 194-499 | 290-749 | 3-87-998 | 4-84-125 | 5-81-150 | 6-78-175 | 7-75-200 | 8-72-225 | 9-68-250 | 33 |
| 30 | 968-250 | 194-500 | 290-751 | 3-87-100 | 4-84-125 | 5-81-150 | 6-78-175 | 7-75-200 | 8-71-225 | 9-68-250 | 30 |
| 33 | 968-251 | 194-502 | 290-751 | 3-87-100 | 4-84-126 | 5-81-151 | 6-78-176 | 7-74-201 | 8-71-226 | 9-68-251 | 27 |
| 36 | 968-252 | 194-504 | 290-756 | 3-87-101 | 4-84-126 | 5-81-151 | 6-77-176 | 7-74-202 | 8-71-227 | 9-68-252 | 24 |
| 39 | 967-253 | 194-506 | 290-759 | 3-87-101 | 4-84-126 | 5-80-152 | 6-77-177 | 7-74-202 | 8-71-228 | 9-67-253 | 21 |
| 42 | 967-254 | 193-507 | 290-761 | 3-87-101 | 4-84-127 | 5-80-152 | 6-77-177 | 7-74-203 | 8-71-228 | 9-67-254 | 18 |
| 45 | 967-255 | 193-509 | 290-761 | 3-87-102 | 4-84-127 | 5-80-153 | 6-77-178 | 7-74-204 | 8-70-229 | 9-67-255 | 15 |
| 48 | 967-255 | 193-511 | 290-766 | 3-87-102 | 4-84-128 | 5-80-153 | 6-77-179 | 7-73-204 | 8-70-230 | 9-67-255 | 12 |
| 51 | 967-256 | 193-513 | 290-766 | 3-87-103 | 4-84-128 | 5-80-154 | 6-77-179 | 7-73-205 | 8-70-231 | 9-67-256 | 9 |
| 54 | 966-257 | 193-514 | 290-771 | 3-87-103 | 4-83-129 | 5-80-154 | 6-76-180 | 7-73-206 | 8-70-231 | 9-67-257 | 6 |
| 57 | 966-258 | 193-516 | 290-774 | 3-86-103 | 4-83-129 | 5-80-155 | 6-76-181 | 7-73-206 | 8-70-232 | 9-66-258 | 3 |

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| | Lat. | Dep. | Lat. | Dep. | Lat. | Dep. | Lat. | Dep. | Lat. | Dep. | |
| 15° | 966-259 | 1°33'51" | 29°0'76" | 386°1'04" | 483°1'29" | 580°1'55" | 670°1'81" | 773°2'07" | 860°2'33" | 96°2'59" | 75 |
| 3 | 966-260 | 1°33'51" | 29°0'79" | 386°1'04" | 483°1'30" | 579°1'56" | 676°1'82" | 773°2'08" | 869°2'34" | 96°2'60" | 57 |
| 6 | 965-260 | 1°33'52" | 29°0'78" | 386°1'04" | 483°1'30" | 579°1'56" | 676°1'82" | 772°2'08" | 869°2'34" | 96°2'60" | 54 |
| 9 | 965-261 | 1°33'52" | 29°0'78" | 386°1'05" | 483°1'31" | 579°1'57" | 676°1'83" | 772°2'09" | 869°2'35" | 96°2'61" | 51 |
| 12 | 965-262 | 1°33'52" | 29°0'79" | 386°1'05" | 482°1'31" | 579°1'57" | 675°1'83" | 772°2'10" | 868°2'36" | 96°2'62" | 48 |
| 15 | 965-263 | 1°33'52" | 29°0'79" | 386°1'05" | 482°1'32" | 579°1'58" | 675°1'81" | 772°2'10" | 868°2'37" | 96°2'63" | 45 |
| 18 | 965-264 | 1°33'52" | 29°0'79" | 386°1'06" | 482°1'32" | 579°1'58" | 675°1'83" | 772°2'11" | 868°2'37" | 96°2'64" | 42 |
| 21 | 964-265 | 1°33'52" | 29°0'79" | 386°1'06" | 482°1'32" | 578°1'59" | 675°1'85" | 771°2'12" | 868°2'38" | 96°2'65" | 39 |
| 24 | 964-266 | 1°33'53" | 29°0'79" | 386°1'06" | 482°1'33" | 578°1'59" | 675°1'85" | 771°2'12" | 868°2'39" | 96°2'66" | 36 |
| 27 | 964-266 | 1°33'53" | 29°0'79" | 386°1'07" | 482°1'33" | 578°1'60" | 675°1'86" | 771°2'13" | 868°2'40" | 96°2'66" | 33 |
| 30 | 964-267 | 1°33'53" | 29°0'80" | 386°1'07" | 482°1'34" | 578°1'60" | 675°1'87" | 771°2'14" | 867°2'41" | 96°2'67" | 30 |
| 33 | 963-268 | 1°33'53" | 29°0'80" | 386°1'07" | 482°1'34" | 578°1'61" | 671°1'88" | 771°2'14" | 867°2'41" | 96°2'68" | 27 |
| 36 | 963-279 | 1°33'53" | 29°0'80" | 386°1'08" | 482°1'34" | 578°1'61" | 671°1'88" | 771°2'15" | 867°2'42" | 96°2'69" | 24 |
| 39 | 963-270 | 1°33'54" | 29°0'80" | 386°1'08" | 481°1'35" | 578°1'62" | 674°1'89" | 770°2'16" | 867°2'43" | 96°2'70" | 21 |
| 42 | 963-271 | 1°33'54" | 29°0'81" | 386°1'08" | 481°1'35" | 578°1'62" | 674°1'89" | 770°2'16" | 866°2'44" | 96°2'71" | 18 |
| 45 | 962-271 | 1°32'543 | 29°0'81" | 385°1'09" | 481°1'36" | 577°1'63" | 674°1'90" | 770°2'17" | 866°2'44" | 96°2'71" | 15 |
| 48 | 962-272 | 1°32'545 | 29°0'81" | 385°1'09" | 481°1'36" | 577°1'63" | 674°1'91" | 770°2'18" | 866°2'45" | 96°2'72" | 12 |
| 51 | 962-273 | 1°32'546 | 29°0'81" | 385°1'09" | 481°1'37" | 577°1'64" | 673°1'93" | 770°2'18" | 866°2'46" | 96°2'73" | 9 |
| 54 | 962-274 | 1°32'548 | 29°0'81" | 385°1'10" | 481°1'37" | 577°1'64" | 673°1'92" | 769°2'19" | 866°2'47" | 96°2'74" | 6 |
| 57 | 961-275 | 1°32'550 | 29°0'88" | 385°1'10" | 481°1'37" | 577°1'65" | 673°1'92" | 769°2'20" | 865°2'47" | 96°2'75" | 3 |
| 16° | 961-276 | 1°32'551 | 29°0'88" | 385°1'10" | 481°1'38" | 577°1'65" | 673°1'93" | 769°2'20" | 865°2'48" | 96°2'76" | 74 |
| 3 | 961-276 | 1°32'552 | 29°0'88" | 385°1'10" | 480°1'38" | 577°1'66" | 673°1'94" | 769°2'21" | 865°2'49" | 96°2'77" | 57 |
| 6 | 961-277 | 1°32'554 | 29°0'88" | 385°1'11" | 480°1'39" | 576°1'69" | 673°1'94" | 769°2'22" | 865°2'50" | 96°2'77" | 54 |
| 9 | 961-278 | 1°32'556 | 29°0'88" | 385°1'11" | 480°1'39" | 576°1'69" | 672°1'95" | 768°2'22" | 864°2'50" | 96°2'78" | 51 |
| 12 | 960-279 | 1°32'558 | 29°0'88" | 385°1'12" | 480°1'39" | 576°1'69" | 672°1'95" | 768°2'23" | 864°2'51" | 96°2'79" | 48 |
| 15 | 960-280 | 1°32'560 | 29°0'88" | 385°1'12" | 480°1'40" | 576°1'68" | 672°1'96" | 768°2'24" | 864°2'52" | 96°2'80" | 45 |
| 18 | 960-281 | 1°32'561 | 29°0'88" | 385°1'12" | 480°1'40" | 576°1'68" | 672°1'96" | 768°2'24" | 864°2'53" | 96°2'81" | 42 |
| 21 | 960-281 | 1°32'563 | 29°0'88" | 385°1'13" | 480°1'41" | 576°1'69" | 672°1'97" | 768°2'25" | 864°2'54" | 96°2'81" | 39 |
| 24 | 959-282 | 1°32'565 | 29°0'88" | 385°1'13" | 480°1'41" | 576°1'69" | 672°1'98" | 767°2'26" | 863°2'54" | 95°2'82" | 36 |
| 27 | 959-283 | 1°32'566 | 29°0'88" | 385°1'13" | 480°1'42" | 575°1'69" | 671°1'98" | 767°2'26" | 863°2'55" | 95°2'83" | 33 |
| 30 | 959-284 | 1°32'568 | 29°0'88" | 385°1'14" | 479°1'42" | 575°1'70" | 671°1'99" | 767°2'27" | 863°2'56" | 95°2'84" | 30 |
| 33 | 959-285 | 1°32'570 | 29°0'88" | 385°1'14" | 479°1'42" | 575°1'71" | 671°1'99" | 767°2'28" | 863°2'56" | 95°2'85" | 27 |
| 36 | 958-286 | 1°32'571 | 29°0'87" | 385°1'14" | 479°1'43" | 575°1'71" | 671°2'00" | 767°2'22" | 862°2'57" | 95°2'86" | 24 |
| 39 | 958-287 | 1°32'573 | 29°0'87" | 385°1'15" | 479°1'43" | 575°1'72" | 671°2'01" | 766°2'30" | 862°2'58" | 95°2'87" | 21 |
| 42 | 958-287 | 1°32'575 | 29°0'87" | 385°1'15" | 479°1'44" | 575°1'72" | 670°2'01" | 766°2'30" | 862°2'59" | 95°2'87" | 18 |
| 45 | 958-287 | 1°32'576 | 29°0'87" | 385°1'15" | 479°1'44" | 575°1'73" | 670°2'02" | 765°2'31" | 862°2'59" | 95°2'88" | 15 |
| 48 | 957-289 | 1°31'578 | 29°0'87" | 385°1'16" | 479°1'44" | 574°1'73" | 670°2'02" | 766°2'31" | 862°2'60" | 95°2'89" | 12 |
| 51 | 957-290 | 1°31'580 | 29°0'87" | 385°1'16" | 479°1'45" | 574°1'74" | 670°2'03" | 766°2'32" | 861°2'61" | 95°2'90" | 9 |
| 54 | 957-291 | 1°31'581 | 29°0'87" | 387°1'16" | 478°1'45" | 574°1'74" | 670°2'03" | 765°2'32" | 861°2'62" | 95°2'91" | 6 |
| 57 | 957-291 | 1°31'583 | 29°0'87" | 387°1'17" | 478°1'46" | 574°1'75" | 670°2'04" | 765°2'33" | 861°2'62" | 95°2'91" | 3 |
| 17° | 956-292 | 1°31'585 | 29°0'87" | 387°1'17" | 478°1'46" | 574°1'75" | 669°2'05" | 765°2'34" | 861°2'63" | 95°2'92" | 73 |
| 3 | 956-293 | 1°31'586 | 29°0'87" | 387°1'17" | 478°1'47" | 573°1'76" | 670°2'05" | 765°2'35" | 866°2'64" | 95°2'93" | 57 |
| 6 | 956-291 | 1°31'588 | 29°0'87" | 387°1'18" | 478°1'47" | 573°1'76" | 669°2'06" | 765°2'35" | 865°2'64" | 95°2'94" | 54 |
| 9 | 956-295 | 1°31'590 | 29°0'87" | 387°1'18" | 478°1'47" | 573°1'77" | 669°2'06" | 764°2'36" | 865°2'65" | 95°2'95" | 51 |
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| 54 | 928°373' | 1°56'746" | 2°78'112" | 3°71'149" | 1°64'186" | 5°57'224" | 0°49'261" | 7°42'298" | 8°35'336" | 9°28'373" | 928°374' | 3 | | | | | | | | | |
| 57 | 928°374' | 1°56'748" | 2°78'112" | 3°71'150" | 1°64'187" | 5°57'224" | 0°49'262" | 7°42'299" | 8°35'336" | 9°28'374" | 928°374' | 3 | | | | | | | | | |
| 22° | 927°375' | 1°55'749" | 2°78'112" | 3°71'150" | 1°64'187" | 5°56'225" | 0°49'262" | 7°42'300" | 8°34'337" | 9°27'375" | 927°375' | 57 | | | | | | | | | 68' |
| 8 | 927°375' | 1°55'751" | 2°78'113" | 3°71'150" | 1°63'188" | 5°56'225" | 0°49'263" | 7°41'300" | 8°34'338" | 9°27'375" | 927°376' | 54 | | | | | | | | | |
| 6 | 927°376' | 1°55'752" | 2°78'113" | 3°71'150" | 1°63'188" | 5°56'226" | 0°49'263" | 7°41'301" | 8°34'339" | 9°27'376" | 927°377' | 51 | | | | | | | | | |
| 9 | 927°377' | 1°55'754" | 2°78'113" | 3°70'151" | 1°63'189" | 5°56'226" | 0°48'264" | 7°41'302" | 8°34'339" | 9°26'377" | 927°378' | 48 | | | | | | | | | |
| 12 | 927°378' | 1°55'756" | 2°78'113" | 3°70'151" | 1°63'189" | 5°56'227" | 0°48'264" | 7°41'302" | 8°33'340" | 9°26'378" | 927°379' | 45 | | | | | | | | | |
| 15 | 927°379' | 1°55'757" | 2°78'111" | 3°70'151" | 1°63'190" | 5°55'227" | 0°48'265" | 7°40'303" | 8°33'341" | 9°26'379" | 927°379' | 42 | | | | | | | | | |
| 18 | 927°379' | 1°55'758" | 2°78'111" | 3°70'152" | 1°63'190" | 5°55'228" | 0°48'266" | 7°40'304" | 8°33'342" | 9°25'379" | 927°380' | 39 | | | | | | | | | |
| 21 | 927°380' | 1°55'761" | 2°77'111" | 3°70'152" | 1°62'190" | 5°55'228" | 0°47'266" | 7°40'304" | 8°32'342" | 9°25'380" | 927°381' | 36 | | | | | | | | | |
| 24 | 927°381' | 1°55'762" | 2°77'111" | 3°70'152" | 1°62'191" | 5°55'229" | 0°47'267" | 7°40'305" | 8°32'343" | 9°25'381" | 927°382' | 33 | | | | | | | | | |
| 27 | 927°382' | 1°55'764" | 2°77'111" | 3°70'153" | 1°62'191" | 5°55'229" | 0°47'267" | 7°40'306" | 8°32'344" | 9°24'382" | 927°383' | 30 | | | | | | | | | |
| 30 | 927°383' | 1°55'765" | 2°77'111" | 3°70'153" | 1°62'191" | 5°54'230" | 0°47'268" | 7°39'306" | 8°32'344" | 9°24'383" | 927°384' | 27 | | | | | | | | | |
| 33 | 927°384' | 1°55'767" | 2°77'111" | 3°69'153" | 1°62'192" | 5°54'230" | 0°46'268" | 7°39'307" | 8°31'345" | 9°24'383" | 927°385' | 24 | | | | | | | | | |
| 36 | 927°385' | 1°55'768" | 2°77'111" | 3°69'154" | 1°62'192" | 5°54'231" | 0°46'269" | 7°39'307" | 8°31'346" | 9°23'384" | 927°386' | 21 | | | | | | | | | |
| 39 | 927°386' | 1°55'770" | 2°77'111" | 3°69'154" | 1°61'193" | 5°54'231" | 0°46'270" | 7°38'308" | 8°31'347" | 9°23'385" | 927°387' | 18 | | | | | | | | | |
| 42 | 927°387' | 1°55'772" | 2°77'111" | 3°69'154" | 1°61'193" | 5°54'232" | 0°46'270" | 7°38'309" | 8°30'347" | 9°23'386" | 927°388' | 15 | | | | | | | | | |
| 45 | 927°388' | 1°55'773" | 2°77'116" | 3°69'155" | 1°61'193" | 5°53'232" | 0°46'271" | 7°38'309" | 8°30'348" | 9°22'387" | 927°389' | 12 | | | | | | | | | |
| 48 | 927°389' | 1°55'775" | 2°77'116" | 3°69'155" | 1°61'194" | 5°53'233" | 0°45'271" | 7°37'310" | 8°30'349" | 9°22'388" | 927°390' | 9 | | | | | | | | | |
| 51 | 927°390' | 1°55'777" | 2°76'117" | 3°69'156" | 1°61'194" | 5°53'233" | 0°45'272" | 7°37'311" | 8°29'340" | 9°22'388" | 927°391' | 6 | | | | | | | | | |
| 54 | 927°391' | 1°55'778" | 2°76'117" | 3°68'156" | 1°61'195" | 5°53'233" | 0°45'272" | 7°37'311" | 8'29'350" | 9'21'389" | 927°392' | 3 | | | | | | | | | |
| 57 | 927°392' | 1°55'780" | 2°76'117" | 3°68'156" | 1°60'195" | 5°52'234" | 0'45'273" | 7'37'312" | 8'29'351" | 9'21'390" | 927°393' | 3 | | | | | | | | | |
| 23° | 921°391' | 1°54'781" | 2°76'117" | 3°68'156" | 1°60'195" | 5°52'234" | 0'44'274" | 7'36'313" | 8'28'352" | 9'21'391" | 921°392' | 57 | | | | | | | | | 67' |
| 8 | 921°392' | 1°54'783" | 2°76'117" | 3°68'157" | 1°60'196" | 5°52'235" | 0'44'274" | 7'36'313" | 8'28'352" | 9'20'392" | 921°393' | 54 | | | | | | | | | |
| 6 | 921°393' | 1°54'785" | 2°76'118" | 3°68'157" | 1°60'196" | 5°52'235" | 0'44'275" | 7'36'314" | 8'28'353" | 9'20'392" | 921°394' | 51 | | | | | | | | | |
| 9 | 921°394' | 1°54'786" | 2°76'118" | 3°68'157" | 1°60'197" | 5°52'236" | 0'44'275" | 7'36'315" | 8'28'354" | 9'19'394" | 921°395' | 48 | | | | | | | | | |
| 12 | 921°395' | 1°54'788" | 2°76'118" | 3°68'158" | 1°59'197" | 5'51'237" | 0'43'276" | 7'35'316" | 8'27'355" | 9'19'395" | 921°396' | 45 | | | | | | | | | |
| 15 | 921°396' | 1°54'790" | 2°76'118" | 3°68'158" | 1°59'197" | 5'51'237" | 0'43'277" | 7'35'316" | 8'27'356" | 9'18'396" | 921°397' | 42 | | | | | | | | | |
| 18 | 921°397' | 1°54'791" | 2°76'119" | 3°67'158" | 1°59'198" | 5'51'237" | 0'43'277" | 7'35'317" | 8'26'357" | 9'18'396" | 921°398' | 39 | | | | | | | | | |
| 21 | 921°398' | 1°54'793" | 2°75'119" | 3°67'159" | 1°59'198" | 5'51'238" | 0'43'277" | 7'34'317" | 8'26'357" | 9'18'397" | 921°399' | 36 | | | | | | | | | |
| 24 | 921°399' | 1°54'794" | 2°75'119" | 3°67'159" | 1°59'199" | 5'51'238" | 0'42'278" | 7'34'318" | 8'26'357" | 9'18'397" | 921°400' | 33 | | | | | | | | | |
| 27 | 921°400' | 1°54'796" | 2°75'119" | 3°67'159" | 1°59'199" | 5'50'239" | 0'42'279" | 7'34'319" | 8'25'358" | 9'17'399" | 921°401' | 30 | | | | | | | | | |
| 30 | 921°401' | 1°54'797" | 2°75'120" | 3°67'160" | 1°58'200" | 5'50'240" | 0'42'280" | 7'33'320" | 8'25'360" | 9'16'400" | 921°402' | 27 | | | | | | | | | |
| 33 | 921°402' | 1°54'798" | 2°75'120" | 3°67'160" | 1°58'200" | 5'50'240" | 0'41'280" | 7'33'320" | 8'25'360" | 9'16'400" | 921°403' | 24 | | | | | | | | | |
| 36 | 921°403' | 1°54'799" | 2°75'121" | 3°66'161" | 1°58'201" | 5'49'241" | 0'41'281" | 7'33'321" | 8'24'361" | 9'16'401" | 921°404' | 21 | | | | | | | | | |
| 39 | 921°404' | 1°54'800" | 2°75'121" | 3°66'161" | 1°58'201" | 5'49'241" | 0'41'281" | 7'33'322" | 8'24'362" | 9'16'402" | 921°405' | 18 | | | | | | | | | |
| 42 | 921°405' | 1°54'801" | 2°75'122" | 3°66'161" | 1°58'201" | 5'49'241" | 0'41'281" | 7'33'322" | 8'24'363" | 9'15'403" | 921°406' | 15 | | | | | | | | | |
| 45 | 921°406' | 1°54'802" | 2°75'122" | 3°66'161" | 1°58'201" | 5'49'242" | 0'41'282" | 7'32'322" | 8'23'364" | 9'15'404" | 921°407' | 12 | | | | | | | | | |
| 48 | 921°407' | 1°54'803" | 2°75'122" | 3°66'162" | 1°57'202" | 5'49'243" | 0'40'283" | 7'32'323" | 8'23'364" | 9'15'404" | 921°408' | 9 | | | | | | | | | |
| 51 | 921°408' | 1°54'804" | 2°75'122" | 3°66'162" | 1°57'202" | 5'49'243" | 0'40'283" | 7'32'323" | 8'23'364" | 9'15'404" | 921°409' | 6 | | | | | | | | | |
| 54 | 921°409' | 1°54'805" | 2°75'122" | 3°66'162" | 1°57'203" | 5'48'244" | 0'40'284" | 7'31'323" | 8'23'365" | 9'14'404" | 9 | | | | | | | | | | |

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| 24° | 914 407 | 189 813 | 274 122 | 365 168 | 457 203 | 548 244 | 640 285 | 731 326 | 822 366 | 914 407 | 66° |
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| 6 | 913 408 | 189 817 | 274 122 | 365 168 | 458 204 | 548 245 | 639 286 | 730 327 | 822 367 | 913 408 | 54 |
| 9 | 913 409 | 189 818 | 274 122 | 365 168 | 458 205 | 547 245 | 639 286 | 730 327 | 821 368 | 912 409 | 51 |
| 12 | 913 410 | 189 820 | 274 123 | 365 168 | 458 205 | 547 246 | 639 287 | 730 328 | 821 369 | 912 410 | 48 |
| 15 | 913 411 | 189 821 | 274 123 | 365 168 | 458 205 | 547 246 | 638 287 | 729 328 | 821 370 | 912 411 | 45 |
| 18 | 911 412 | 189 823 | 273 123 | 365 168 | 458 206 | 547 247 | 638 288 | 729 329 | 820 370 | 911 412 | 42 |
| 21 | 911 413 | 189 825 | 273 124 | 364 168 | 458 206 | 547 247 | 638 288 | 729 330 | 820 371 | 911 413 | 39 |
| 24 | 911 413 | 189 826 | 273 124 | 364 168 | 458 207 | 546 248 | 637 289 | 729 330 | 820 372 | 911 414 | 36 |
| 27 | 910 414 | 189 828 | 273 124 | 364 168 | 455 207 | 546 248 | 637 290 | 728 331 | 819 372 | 910 414 | 33 |
| 30 | 910 415 | 189 829 | 273 124 | 364 168 | 455 207 | 546 249 | 637 290 | 728 332 | 819 373 | 910 415 | 30 |
| 33 | 910 415 | 189 831 | 273 125 | 364 168 | 455 208 | 546 249 | 637 291 | 728 332 | 819 374 | 910 415 | 27 |
| 36 | 909 416 | 189 833 | 273 125 | 364 168 | 455 208 | 546 250 | 636 291 | 727 333 | 818 375 | 909 416 | 24 |
| 39 | 909 417 | 189 834 | 273 125 | 364 167 | 454 209 | 545 250 | 636 292 | 727 334 | 818 375 | 909 417 | 21 |
| 42 | 909 418 | 189 836 | 273 125 | 363 167 | 454 209 | 545 251 | 636 293 | 727 334 | 818 376 | 909 418 | 18 |
| 45 | 908 419 | 189 837 | 273 126 | 363 167 | 454 209 | 545 251 | 636 293 | 727 335 | 817 377 | 908 419 | 15 |
| 48 | 908 419 | 189 839 | 272 126 | 363 168 | 454 210 | 545 252 | 635 294 | 726 336 | 817 378 | 908 419 | 12 |
| 51 | 907 420 | 189 840 | 272 126 | 363 168 | 454 210 | 544 252 | 635 294 | 726 336 | 817 378 | 907 420 | 9 |
| 54 | 907 421 | 189 842 | 272 127 | 363 168 | 454 211 | 544 253 | 636 295 | 726 337 | 816 379 | 907 421 | 6 |
| 57 | 907 422 | 189 844 | 272 127 | 363 169 | 453 211 | 544 253 | 635 295 | 725 337 | 816 380 | 907 422 | 3 |
| 25° | 906 423 | 181 845 | 272 127 | 363 169 | 453 211 | 544 254 | 634 296 | 725 338 | 816 380 | 906 423 | 65° |
| 3 | 906 423 | 181 847 | 272 127 | 362 169 | 453 211 | 544 254 | 634 296 | 726 339 | 815 381 | 906 423 | 57 |
| 6 | 906 424 | 181 848 | 272 127 | 362 170 | 453 212 | 543 255 | 634 297 | 724 339 | 815 382 | 906 424 | 54 |
| 9 | 905 425 | 181 850 | 272 127 | 362 170 | 453 212 | 543 255 | 634 297 | 724 340 | 815 382 | 905 425 | 51 |
| 12 | 905 426 | 181 852 | 271 128 | 362 170 | 452 213 | 543 255 | 638 298 | 724 341 | 814 383 | 905 426 | 48 |
| 15 | 905 427 | 181 853 | 271 128 | 362 171 | 452 213 | 543 256 | 633 299 | 724 341 | 814 384 | 905 427 | 45 |
| 18 | 904 427 | 181 855 | 271 128 | 362 171 | 452 214 | 542 256 | 633 299 | 723 342 | 814 385 | 904 427 | 42 |
| 21 | 904 428 | 181 856 | 271 128 | 361 171 | 452 214 | 542 257 | 633 300 | 723 343 | 813 386 | 904 428 | 39 |
| 24 | 903 429 | 181 858 | 271 129 | 361 172 | 452 214 | 542 257 | 632 300 | 723 343 | 813 386 | 903 429 | 36 |
| 27 | 903 430 | 181 860 | 271 129 | 361 172 | 451 215 | 542 258 | 632 301 | 722 344 | 813 387 | 903 430 | 33 |
| 30 | 903 430 | 181 861 | 271 129 | 361 172 | 451 215 | 542 258 | 632 301 | 722 344 | 812 387 | 903 430 | 30 |
| 33 | 902 431 | 180 863 | 271 129 | 361 173 | 451 216 | 541 259 | 632 302 | 722 345 | 812 388 | 902 431 | 27 |
| 36 | 902 432 | 180 864 | 271 130 | 361 173 | 451 216 | 541 259 | 631 302 | 721 346 | 812 389 | 902 432 | 24 |
| 39 | 901 433 | 180 866 | 270 130 | 361 173 | 451 216 | 541 260 | 631 303 | 721 346 | 811 390 | 902 433 | 21 |
| 42 | 901 434 | 180 867 | 270 130 | 360 173 | 451 217 | 541 260 | 631 304 | 721 347 | 811 391 | 902 434 | 18 |
| 45 | 901 434 | 180 869 | 270 130 | 360 174 | 450 217 | 540 261 | 630 304 | 721 348 | 811 391 | 902 434 | 15 |
| 48 | 900 435 | 180 870 | 270 131 | 360 174 | 450 218 | 540 261 | 630 305 | 720 348 | 810 392 | 900 435 | 12 |
| 51 | 900 436 | 180 872 | 270 131 | 360 174 | 450 218 | 549 262 | 630 305 | 720 349 | 810 392 | 900 436 | 9 |
| 54 | 900 437 | 180 874 | 270 131 | 360 175 | 450 218 | 540 262 | 630 306 | 720 349 | 810 393 | 900 437 | 6 |
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| 3 | 898 439 | 180 878 | 270 132 | 359 176 | 449 219 | 539 263 | 629 307 | 719 351 | 809 395 | 898 439 | 57 |
| 6 | 898 440 | 180 880 | 269 132 | 359 176 | 449 220 | 539 264 | 629 308 | 718 352 | 808 396 | 898 440 | 54 |
| 9 | 898 441 | 180 881 | 269 132 | 358 176 | 449 220 | 539 264 | 628 308 | 718 353 | 808 397 | 898 441 | 51 |
| 12 | 897 441 | 179 883 | 269 132 | 359 177 | 449 221 | 538 265 | 628 309 | 718 353 | 808 397 | 897 441 | 48 |
| 15 | 897 442 | 179 885 | 269 133 | 359 177 | 448 221 | 538 265 | 628 310 | 717 354 | 808 398 | 897 442 | 45 |
| 18 | 896 443 | 179 886 | 269 133 | 359 177 | 448 221 | 538 266 | 628 310 | 717 354 | 807 399 | 896 443 | 42 |
| 21 | 896 444 | 179 888 | 269 133 | 358 178 | 448 222 | 538 266 | 621 311 | 717 355 | 806 399 | 896 444 | 39 |
| 24 | 896 447 | 179 889 | 269 133 | 358 178 | 448 222 | 537 267 | 627 311 | 717 356 | 806 400 | 896 446 | 36 |
| 27 | 895 445 | 179 891 | 269 134 | 358 178 | 448 223 | 537 267 | 627 312 | 716 356 | 806 401 | 895 445 | 33 |
| 30 | 895 446 | 179 892 | 268 134 | 358 178 | 447 223 | 537 268 | 626 312 | 716 357 | 805 402 | 895 446 | 30 |
| 33 | 895 447 | 179 894 | 268 134 | 358 179 | 447 223 | 537 268 | 626 313 | 716 358 | 805 402 | 895 447 | 27 |
| 36 | 894 448 | 179 896 | 268 134 | 358 179 | 447 224 | 536 269 | 620 313 | 715 358 | 805 403 | 894 448 | 24 |
| 39 | 894 449 | 179 897 | 268 135 | 358 179 | 447 224 | 536 269 | 626 314 | 715 359 | 804 404 | 894 449 | 21 |
| 42 | 894 449 | 179 899 | 268 135 | 357 180 | 447 225 | 536 270 | 625 315 | 715 359 | 804 404 | 894 449 | 18 |
| 45 | 893 450 | 179 900 | 268 135 | 357 180 | 446 225 | 535 270 | 625 315 | 714 360 | 803 405 | 893 450 | 16 |
| 48 | 893 451 | 179 902 | 268 135 | 357 180 | 446 225 | 536 271 | 625 316 | 714 361 | 803 406 | 893 451 | 12 |
| 51 | 892 452 | 178 903 | 268 135 | 357 181 | 446 226 | 535 271 | 625 316 | 714 361 | 803 406 | 892 452 | 9 |
| 54 | 892 453 | 178 905 | 268 136 | 357 181 | 446 226 | 535 271 | 624 317 | 714 362 | 803 407 | 892 453 | 6 |
| 57 | 891 453 | 178 906 | 268 136 | 357 181 | 446 227 | 535 272 | 624 317 | 713 363 | 802 408 | 891 453 | 3 |

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| 56 | 828-561 | 166 1-12 | 2-48 | 1-68 | 3-31 2-25 | 4-14 2-81 | 4-97 3-37 | 5-79 3-93 | 6-63 4-98 | 7-46 5-05 | 8-28 5-61 | 51 | | | | | | | | | |
| 57 | 827-562 | 165 1-12 | 2-48 | 1-69 | 3-31 2-25 | 4-14 2-81 | 4-96 3-37 | 5-79 3-93 | 6-62 4-99 | 7-45 5-06 | 8-27 5-62 | 49 | | | | | | | | | |
| 58 | 827-563 | 165 1-13 | 2-48 | 1-69 | 3-31 2-25 | 4-13 2-81 | 4-96 3-38 | 5-79 3-94 | 6-61 4-99 | 7-44 5-07 | 8-27 5-63 | 45 | | | | | | | | | |
| 59 | 826-564 | 165 1-13 | 2-48 | 1-69 | 3-30 2-25 | 4-13 2-82 | 4-96 3-38 | 5-78 3-94 | 6-61 4-99 | 7-43 5-07 | 8-26 5-64 | 43 | | | | | | | | | |
| 60 | 826-564 | 165 1-13 | 2-48 | 1-69 | 3-30 2-26 | 4-13 2-82 | 4-96 3-39 | 5-78 3-95 | 6-60 4-99 | 7-43 5-08 | 8-26 5-64 | 40 | | | | | | | | | |
| 61 | 825-565 | 165 1-13 | 2-48 | 1-69 | 3-30 2-26 | 4-13 2-83 | 4-95 3-39 | 5-78 3-95 | 6-60 4-99 | 7-43 5-08 | 8-26 5-65 | 38 | | | | | | | | | |
| 62 | 825-566 | 165 1-13 | 2-47 | 1-70 | 3-30 2-26 | 4-12 2-83 | 4-95 3-39 | 5-77 3-96 | 6-59 4-99 | 7-42 5-09 | 8-25 5-66 | 35 | | | | | | | | | |
| 63 | 824-565 | 165 1-13 | 2-47 | 1-70 | 3-30 2-27 | 4-12 2-83 | 4-94 3-40 | 5-77 3-96 | 6-59 4-99 | 7-41 5-10 | 8-24 5-67 | 33 | | | | | | | | | |
| 64 | 824-567 | 165 1-13 | 2-47 | 1-70 | 3-29 2-27 | 4-12 2-84 | 4-94 3-40 | 5-77 3-97 | 6-59 4-99 | 7-41 5-11 | 8-24 5-67 | 31 | | | | | | | | | |
| 65 | 823-568 | 165 1-14 | 2-47 | 1-70 | 3-29 2-27 | 4-12 2-84 | 4-94 3-41 | 5-76 3-97 | 6-59 4-99 | 7-41 5-11 | 8-23 5-68 | 24 | | | | | | | | | |
| 66 | 823-569 | 165 1-14 | 2-47 | 1-71 | 3-29 2-27 | 4-11 2-84 | 4-94 3-41 | 5-76 3-98 | 6-58 4-99 | 7-40 5-12 | 8-22 5-69 | 21 | | | | | | | | | |
| 67 | 822-569 | 164 1-14 | 2-47 | 1-71 | 3-29 2-28 | 4-11 2-85 | 4-95 3-39 | 5-75 3-98 | 6-58 4-99 | 7-40 5-12 | 8-22 5-69 | 18 | | | | | | | | | |
| 68 | 822-570 | 164 1-14 | 2-46 | 1-71 | 3-29 2-28 | 4-11 2-85 | 4-95 3-39 | 5-75 3-99 | 6-57 4-99 | 7-39 5-13 | 8-22 5-70 | 16 | | | | | | | | | |
| 69 | 821-571 | 164 1-14 | 2-46 | 1-71 | 3-28 2-28 | 4-11 2-85 | 4-95 3-42 | 5-75 3-99 | 6-57 4-99 | 7-39 5-14 | 8-21 5-71 | 14 | | | | | | | | | |
| 70 | 821-571 | 164 1-14 | 2-46 | 1-71 | 3-28 2-29 | 4-10 2-86 | 4-95 3-43 | 5-74 4-00 | 6-57 4-97 | 7-39 5-14 | 8-21 5-71 | 9 | | | | | | | | | |
| 71 | 820-572 | 164 1-14 | 2-46 | 1-72 | 3-28 2-29 | 4-10 2-86 | 4-95 3-43 | 5-74 4-01 | 6-56 4-98 | 7-38 5-15 | 8-20 5-72 | 6 | | | | | | | | | |
| 72 | 820-573 | 164 1-15 | 2-46 | 1-72 | 3-28 2-29 | 4-10 2-86 | 4-95 3-44 | 5-74 4-01 | 6-56 4-98 | 7-38 5-16 | 8-20 5-73 | 3 | | | | | | | | | |
| 73 | 819-574 | 164 1-15 | 2-46 | 1-72 | 3-28 2-29 | 4-16 2-87 | 4-91 3-44 | 5-73 4-02 | 6-55 4-99 | 7-37 5-16 | 8-19 5-74 | 55 | | | | | | | | | |
| 74 | 819-574 | 164 1-15 | 2-46 | 1-72 | 3-27 2-30 | 4-09 2-87 | 4-91 3-45 | 5-73 4-03 | 6-55 4-99 | 7-37 5-17 | 8-19 5-74 | 57 | | | | | | | | | |
| 75 | 818-575 | 164 1-15 | 2-45 | 1-73 | 3-27 2-30 | 4-09 2-88 | 4-91 3-45 | 5-73 4-03 | 6-54 4-98 | 7-36 5-18 | 8-18 5-75 | 54 | | | | | | | | | |
| 76 | 818-576 | 164 1-15 | 2-45 | 1-73 | 3-27 2-30 | 4-09 2-88 | 4-91 3-45 | 5-72 4-03 | 6-54 4-98 | 7-36 5-18 | 8-18 5-76 | 51 | | | | | | | | | |
| 77 | 817-576 | 163 1-15 | 2-45 | 1-73 | 3-27 2-31 | 4-09 2-88 | 4-90 3-46 | 5-72 4-04 | 6-54 4-98 | 7-35 5-19 | 8-17 5-77 | 45 | | | | | | | | | |
| 78 | 817-577 | 163 1-15 | 2-45 | 1-73 | 3-27 2-31 | 4-08 2-89 | 4-90 3-46 | 5-72 4-04 | 6-53 4-98 | 7-35 5-19 | 8-17 5-77 | 45 | | | | | | | | | |
| 79 | 816-578 | 163 1-16 | 2-45 | 1-73 | 3-26 2-31 | 4-08 2-89 | 4-90 3-47 | 5-71 4-04 | 6-53 4-98 | 7-35 5-20 | 8-16 5-78 | 43 | | | | | | | | | |
| 80 | 816-579 | 163 1-16 | 2-45 | 1-74 | 3-26 2-31 | 4-08 2-89 | 4-89 3-47 | 5-71 4-05 | 6-53 4-98 | 7-34 5-21 | 8-16 5-79 | 39 | | | | | | | | | |
| 81 | 815-579 | 163 1-16 | 2-45 | 1-74 | 3-26 2-32 | 4-08 2-90 | 4-89 3-48 | 5-70 4-05 | 6-52 4-98 | 7-34 5-21 | 8-16 5-79 | 36 | | | | | | | | | |
| 82 | 815-580 | 163 1-16 | 2-44 | 1-74 | 3-26 2-32 | 4-07 2-90 | 4-89 3-48 | 5-70 4-06 | 6-52 4-98 | 7-33 5-22 | 8-15 5-80 | 33 | | | | | | | | | |
| 83 | 814-581 | 163 1-16 | 2-44 | 1-74 | 3-25 2-33 | 4-07 2-91 | 4-88 3-49 | 5-70 4-07 | 6-51 4-95 | 7-33 5-23 | 8-14 5-81 | 32 | | | | | | | | | |
| 84 | 814-581 | 163 1-16 | 2-44 | 1-74 | 3-25 2-33 | 4-07 2-91 | 4-88 3-49 | 5-69 4-07 | 6-50 4-96 | 7-32 5-24 | 8-13 5-82 | 24 | | | | | | | | | |
| 85 | 813-583 | 163 1-17 | 2-44 | 1-75 | 3-25 2-33 | 4-06 2-91 | 4-88 3-50 | 5-69 4-08 | 6-50 4-96 | 7-31 5-25 | 8-13 5-83 | 21 | | | | | | | | | |
| 86 | 813-583 | 163 1-17 | 2-44 | 1-75 | 3-25 2-33 | 4-06 2-92 | 4-87 3-50 | 5-68 4-08 | 6-50 4-97 | 7-31 5-25 | 8-13 5-83 | 21 | | | | | | | | | |
| 87 | 812-584 | 163 1-17 | 2-44 | 1-75 | 3-25 2-34 | 4-06 2-92 | 4-87 3-51 | 5-68 4-09 | 6-49 4-97 | 7-30 5-26 | 8-12 5-84 | 18 | | | | | | | | | |
| 88 | 812-584 | 163 1-17 | 2-43 | 1-75 | 3-24 2-34 | 4-06 2-92 | 4-87 3-51 | 5-68 4-09 | 6-49 4-97 | 7-30 5-26 | 8-11 5-85 | 18 | | | | | | | | | |
| 89 | 811-585 | 163 1-17 | 2-43 | 1-75 | 3-24 2-34 | 4-06 2-92 | 4-87 3-51 | 5-67 4-10 | 6-48 4-98 | 7-29 5-27 | 8-11 5-86 | 9 | | | | | | | | | |
| 90 | 811-585 | 163 1-17 | 2-43 | 1-76 | 3-24 2-34 | 4-06 2-93 | 4-86 3-51 | 5-67 4-10 | 6-48 4-98 | 7-29 5-28 | 8-10 5-86 | 8 | | | | | | | | | |
| 91 | 810-586 | 163 1-17 | 2-43 | 1-76 | 3-24 2-35 | 4-06 2-93 | 4-86 3-52 | 5-67 4-11 | 6-48 4-98 | 7-29 5-28 | 8-10 5-87 | 8 | | | | | | | | | |
| 92 | 810-586 | 163 1-17 | 2-43 | 1-76 | 3-24 2-35 | 4-06 2-94 | 4-86 3-52 | 5-67 4-11 | 6-48 4-98 | 7-29 5-28 | | | | | | | | | | | |

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|-----|----------|------------|------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-----|
| | Lat. | Dep. | Lat. | Dep. | Lat. | Dep. | Lat. | Dep. | Lat. | Dep. | |
| 36 | 308° 588 | 189° 1'18" | 243° 1'16" | 3'24° 2'35" | 405° 2'34" | 4'53° 3'53" | 5'66° 4'11" | 6'47° 4'70" | 7'28° 5'32" | 8'09° 5'58" | 52° |
| 3 | 309° 586 | 189° 1'18" | 243° 1'17" | 3'23° 2'35" | 404° 2'34" | 4'53° 3'53" | 5'66° 4'12" | 6'47° 4'71" | 7'28° 5'30" | 8'09° 5'58" | 57 |
| 6 | 308° 585 | 189° 1'18" | 242° 1'17" | 3'23° 2'35" | 404° 2'35" | 4'55° 3'54" | 5'66° 4'12" | 6'46° 4'71" | 7'27° 5'30" | 8'08° 5'58" | 64 |
| 9 | 307° 580 | 181° 1'18" | 242° 1'17" | 3'23° 2'36" | 404° 2'35" | 4'54° 3'54" | 5'63° 4'13" | 6'46° 4'72" | 7'27° 5'31" | 8'07° 5'56" | 61 |
| 12 | 307° 591 | 181° 1'18" | 242° 1'17" | 3'23° 2'35" | 403° 2'35" | 4'54° 3'54" | 5'65° 4'13" | 6'46° 4'72" | 7'26° 5'32" | 8'07° 5'51" | 65 |
| 15 | 306° 591 | 181° 1'18" | 242° 1'17" | 3'23° 2'37" | 403° 2'36" | 4'54° 3'55" | 5'65° 4'14" | 6'45° 4'73" | 7'26° 5'32" | 8'06° 5'51" | 65 |
| 18 | 306° 592 | 181° 1'18" | 242° 1'18" | 3'23° 2'37" | 403° 2'36" | 4'54° 3'55" | 5'64° 4'14" | 6'45° 4'74" | 7'25° 5'33" | 8'06° 5'52" | 49 |
| 21 | 305° 593 | 181° 1'19" | 242° 1'18" | 3'22° 2'37" | 403° 2'36" | 4'53° 3'56" | 5'64° 4'15" | 6'44° 4'74" | 7'25° 5'33" | 8'05° 5'53" | 49 |
| 24 | 305° 593 | 181° 1'19" | 241° 1'18" | 3'22° 2'37" | 402° 2'37" | 4'53° 3'56" | 5'63° 4'15" | 6'44° 4'75" | 7'25° 5'34" | 8'05° 5'53" | 36 |
| 27 | 304° 594 | 181° 1'19" | 241° 1'18" | 3'22° 2'38" | 402° 2'37" | 4'53° 3'56" | 5'63° 4'16" | 6'44° 4'75" | 7'24° 5'35" | 8'04° 5'54" | 33 |
| 30 | 304° 595 | 181° 1'19" | 241° 1'18" | 3'22° 2'38" | 402° 2'37" | 4'52° 3'57" | 5'63° 4'16" | 6'43° 4'76" | 7'23° 5'35" | 8'04° 5'56" | 30 |
| 33 | 303° 596 | 181° 1'19" | 241° 1'19" | 3'21° 2'38" | 402° 2'38" | 4'52° 3'57" | 5'62° 4'17" | 6'43° 4'76" | 7'23° 5'36" | 8'03° 5'56" | 27 |
| 36 | 303° 595 | 181° 1'19" | 241° 1'19" | 3'21° 2'38" | 401° 2'38" | 4'52° 3'58" | 5'62° 4'17" | 6'42° 4'77" | 7'23° 5'37" | 8'03° 5'56" | 24 |
| 39 | 302° 597 | 180° 1'19" | 241° 1'19" | 3'21° 2'38" | 401° 2'38" | 4'51° 3'58" | 5'62° 4'18" | 6'42° 4'78" | 7'22° 5'37" | 8'02° 5'57" | 21 |
| 42 | 302° 598 | 180° 1'20" | 241° 1'19" | 3'21° 2'39" | 401° 2'39" | 4'51° 3'59" | 5'61° 4'18" | 6'41° 4'78" | 7'22° 5'38" | 8'02° 5'58" | 18 |
| 45 | 301° 598 | 180° 1'20" | 240° 1'19" | 3'21° 2'39" | 401° 2'39" | 4'51° 3'59" | 5'61° 4'19" | 6'41° 4'79" | 7'21° 5'38" | 8'01° 5'58" | 15 |
| 48 | 301° 599 | 180° 1'20" | 240° 1'19" | 3'20° 2'40" | 400° 3'00" | 4'50° 3'59" | 5'61° 4'19" | 6'41° 4'79" | 7'21° 5'39" | 8'01° 5'59" | 12 |
| 51 | 300° 600 | 180° 1'20" | 240° 1'19" | 3'20° 2'40" | 400° 3'00" | 4'50° 3'60" | 5'60° 4'20" | 6'40° 4'80" | 7'20° 5'40" | 8'00° 6'00" | 9 |
| 54 | 300° 600 | 180° 1'20" | 240° 1'19" | 3'20° 2'40" | 400° 3'00" | 4'50° 3'60" | 5'60° 4'20" | 6'40° 4'80" | 7'20° 5'40" | 8'00° 6'00" | 6 |
| 57 | 299° 601 | 180° 1'20" | 240° 1'20" | 3'20° 2'40" | 400° 3'01" | 4'79° 3'61" | 5'59° 4'21" | 6'39° 4'81" | 7'19° 5'41" | 7'99° 6'01" | 3 |
| 27 | 799° 642 | 150° 1'22" | 240° 1'21" | 3'19° 2'41" | 3'99° 3'01" | 4'79° 3'61" | 5'59° 4'21" | 6'39° 4'81" | 7'19° 5'42" | 7'99° 6'02" | 53 |
| 31 | 798° 603 | 150° 1'22" | 238° 1'21" | 3'19° 2'41" | 3'99° 3'01" | 4'79° 3'62" | 5'59° 4'22" | 6'38° 4'82" | 7'18° 5'42" | 7'98° 6'03" | 57 |
| 6 | 798° 603 | 150° 1'22" | 239° 1'21" | 3'19° 2'41" | 3'99° 3'02" | 4'79° 3'62" | 5'58° 4'23" | 6'38° 4'83" | 7'18° 5'43" | 7'98° 6'03" | 54 |
| 9 | 797° 604 | 139° 1'22" | 239° 1'21" | 3'19° 2'42" | 3'99° 3'02" | 4'78° 3'62" | 5'58° 4'23" | 6'38° 4'83" | 7'17° 5'44" | 7'97° 6'04" | 51 |
| 12 | 797° 605 | 139° 1'22" | 239° 1'22" | 3'19° 2'42" | 3'98° 3'02" | 4'78° 3'63" | 5'58° 4'23" | 6'37° 4'84" | 7'17° 5'44" | 7'97° 6'05" | 48 |
| 15 | 796° 605 | 138° 1'22" | 239° 1'22" | 3'19° 2'42" | 3'98° 3'03" | 4'78° 3'63" | 5'57° 4'24" | 6'37° 4'84" | 7'16° 5'45" | 7'96° 6'05" | 45 |
| 18 | 795° 606 | 138° 1'22" | 239° 1'22" | 3'19° 2'42" | 3'98° 3'03" | 4'77° 3'64" | 5'57° 4'24" | 6'36° 4'85" | 7'16° 5'45" | 7'95° 6'06" | 42 |
| 21 | 795° 607 | 138° 1'22" | 238° 1'22" | 3'19° 2'43" | 3'97° 3'03" | 4'77° 3'64" | 5'56° 4'25" | 6'36° 4'85" | 7'15° 5'46" | 7'95° 6'07" | 39 |
| 24 | 794° 607 | 138° 1'22" | 238° 1'22" | 3'19° 2'43" | 3'97° 3'04" | 4'77° 3'64" | 5'56° 4'25" | 6'36° 4'86" | 7'15° 5'47" | 7'94° 6'07" | 36 |
| 27 | 794° 608 | 138° 1'22" | 238° 1'22" | 3'19° 2'43" | 3'97° 3'04" | 4'76° 3'65" | 5'56° 4'26" | 6'35° 4'86" | 7'14° 5'47" | 7'94° 6'08" | 33 |
| 30 | 793° 609 | 139° 1'22" | 238° 1'22" | 3'19° 2'44" | 3'97° 3'04" | 4'76° 3'65" | 5'55° 4'26" | 6'35° 4'87" | 7'14° 5'48" | 7'93° 6'09" | 30 |
| 33 | 793° 609 | 139° 1'22" | 238° 1'23" | 3'19° 2'44" | 3'96° 3'03" | 4'76° 3'65" | 5'55° 4'27" | 6'34° 4'88" | 7'14° 5'49" | 7'93° 6'10" | 27 |
| 36 | 792° 610 | 138° 1'23" | 238° 1'23" | 3'19° 2'44" | 3'96° 3'03" | 4'75° 3'66" | 5'55° 4'27" | 6'34° 4'88" | 7'13° 5'49" | 7'92° 6'11" | 24 |
| 39 | 792° 611 | 138° 1'23" | 238° 1'23" | 3'19° 2'44" | 3'96° 3'05" | 4'75° 3'67" | 5'54° 4'28" | 6'33° 4'89" | 7'13° 5'50" | 7'92° 6'11" | 21 |
| 42 | 791° 612 | 139° 1'23" | 238° 1'23" | 3'19° 2'45" | 3'96° 3'06" | 4'75° 3'67" | 5'54° 4'28" | 6'33° 4'89" | 7'12° 5'50" | 7'92° 6'12" | 18 |
| 45 | 791° 612 | 138° 1'23" | 237° 1'23" | 3'19° 2'45" | 3'95° 3'06" | 4'74° 3'67" | 5'53° 4'29" | 6'33° 4'90" | 7'12° 5'51" | 7'92° 6'12" | 15 |
| 48 | 790° 613 | 138° 1'23" | 237° 1'23" | 3'19° 2'45" | 3'95° 3'05" | 4'74° 3'68" | 5'53° 4'29" | 6'32° 4'90" | 7'11° 5'52" | 7'90° 6'13" | 12 |
| 51 | 790° 614 | 138° 1'23" | 237° 1'23" | 3'19° 2'45" | 3'95° 3'07" | 4'74° 3'68" | 5'53° 4'30" | 6'32° 4'91" | 7'11° 5'52" | 7'90° 6'14" | 9 |
| 54 | 789° 614 | 138° 1'23" | 237° 1'23" | 3'19° 2'46" | 3'95° 3'07" | 4'73° 3'69" | 5'52° 4'30" | 6'31° 4'91" | 7'10° 5'53" | 7'89° 6'14" | 6 |
| 57 | 789° 615 | 138° 1'23" | 237° 1'24" | 3'19° 2'46" | 3'94° 3'07" | 4'73° 3'69" | 5'52° 4'30" | 6'31° 4'92" | 7'10° 5'53" | 7'89° 6'15" | 3 |
| 38° | 788° 616 | 156° 1'23" | 236° 1'24" | 3'15° 2'46" | 3'94° 3'08" | 4'73° 3'69" | 5'52° 4'31" | 6'30° 4'93" | 7'09° 5'34" | 7'88° 6'16" | 52 |
| 1 | 787° 616 | 157° 1'23" | 236° 1'24" | 3'15° 2'47" | 3'94° 3'08" | 4'72° 3'70" | 5'51° 4'31" | 6'30° 4'93" | 7'09° 5'35" | 7'87° 6'16" | 57 |
| 6 | 787° 617 | 157° 1'23" | 236° 1'25" | 3'15° 2'47" | 3'93° 3'08" | 4'72° 3'70" | 5'51° 4'32" | 6'30° 4'94" | 7'08° 5'36" | 7'87° 6'17" | 54 |
| 9 | 786° 618 | 157° 1'24" | 236° 1'24" | 3'15° 2'47" | 3'93° 3'09" | 4'72° 3'71" | 5'50° 4'33" | 6'29° 4'95" | 7'07° 5'37" | 7'86° 6'18" | 51 |
| 12 | 786° 618 | 157° 1'24" | 235° 1'24" | 3'14° 2'47" | 3'93° 3'09" | 4'72° 3'71" | 5'50° 4'33" | 6'29° 4'95" | 7'07° 5'37" | 7'86° 6'18" | 48 |
| 15 | 785° 619 | 157° 1'24" | 236° 1'24" | 3'14° 2'48" | 3'93° 3'10" | 4'71° 3'71" | 5'50° 4'33" | 6'28° 4'95" | 7'07° 5'37" | 7'86° 6'19" | 45 |
| 18 | 785° 620 | 157° 1'24" | 235° 1'24" | 3'14° 2'48" | 3'93° 3'10" | 4'71° 3'72" | 5'49° 4'34" | 6'27° 4'96" | 7'06° 5'38" | 7'86° 6'20" | 42 |
| 21 | 784° 620 | 157° 1'24" | 235° 1'24" | 3'14° 2'48" | 3'93° 3'10" | 4'71° 3'73" | 5'49° 4'34" | 6'27° 4'96" | 7'06° 5'38" | 7'86° 6'20" | 39 |
| 24 | 784° 621 | 157° 1'24" | 235° 1'24" | 3'13° 2'48" | 3'92° 3'11" | 4'70° 3'73" | 5'48° 4'35" | 6'27° 4'97" | 7'05° 5'39" | 7'86° 6'21" | 36 |
| 27 | 783° 622 | 157° 1'24" | 235° 1'25" | 3'13° 2'49" | 3'91° 3'11" | 4'70° 3'74" | 5'48° 4'36" | 6'26° 4'98" | 7'04° 5'60" | 7'86° 6'23" | 30 |
| 30 | 783° 622 | 157° 1'25" | 235° 1'25" | 3'13° 2'49" | 3'90° 3'11" | 4'70° 3'74" | 5'48° 4'36" | 6'26° 4'99" | 7'04° 5'61" | 7'86° 6'23" | 27 |
| 33 | 782° 623 | 156° 1'25" | 235° 1'25" | 3'13° 2'49" | 3'89° 3'12" | 4'69° 3'74" | 5'47° 4'37" | 6'25° 4'99" | 7'03° 5'61" | 7'86° 6'24" | 24 |
| 36 | 782° 624 | 156° 1'25" | 234° 1'25" | 3'13° 2'50" | 3'89° 3'12" | 4'69° 3'74" | 5'47° 4'37" | 6'25° 4'99" | 7'03° 5'62" | 7'86° 6'25" | 21 |
| 39 | 781° 625 | 156° 1'25" | 234° 1'25" | 3'12° 2'50" | 3'89° 3'13" | 4'68° 3'75" | 5'46° 4'38" | 6'24° 5'00" | 7'02° 5'63" | 7'86° 6'25" | 18 |
| 42 | 780° 626 | 156° 1'25" | 234° 1'25" | 3'12° 2'50" | 3'89° 3'13" | 4'68° 3'76" | 5'46° 4'38" | 6'24° 5'01" | 7'01° 5'64" | 7'86° 6'27" | 15 |
| 45 | 779° 627 | 156° 1'26" | 234° 1'25" | 3'12° 2'51" | 3'89° 3'13" | 4'68° 3'76" | 5'46° 4'39" | 6'23° 5'01" | 7'01° 5'65" | 7'86° 6'27" | 12 |
| 48 | 779° 627 | 156° 1'26" | 234° 1'25" | 3'12° 2'51" | 3'89° 3'14" | 4'67° 3'76" | 5'45° 4'39" | 6'23° 5'02" | 7'01° 5'65" | 7'86° 6'27" | 9 |
| 51 | 779° 627 | 156° 1'26" | 234° 1'25" | 3'12° 2'51" | 3'89° 3'14" | 4'67° 3'77" | 5'45° 4'40" | 6'23° 5'02" | 7'00° 5'65" | 7'86° 6'28" | 6 |
| 54 | 778° 628 | 156° 1'26" | 234° 1'26" | 3'11° 2'51" | 3'89° 3'14" | 4'67° 3'77" | 5'44° 4'40" | 6'22° 5'03" | 7'00° 5'66" | 7'86° 6'28" | 3 |
| 57 | 778° 629 | 156° 1'26" | 234° 1'26" | 3'11° 2'51" | 3'89° 3'14" | 4'67° 3'77" | 5'44° 4'40" | 6'22° 5'03" | 7'00° 5'66" | 7'86° 6'28" | 3 |

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| 39° | 777 029 | 1°55 1°26 | 2°33 1°89 | 3°11 2°52 | 3°89 3°15 | 4°66 3°78 | 5°44 4°41 | 6°22 5°03 | 6°99 5°66 | 7°77 6°29 | 51° |
| 3 | 777 030 | 1°55 1°26 | 2°33 1°89 | 3°11 2°52 | 3°88 3°15 | 4°66 3°78 | 5°44 4°41 | 6°21 5°04 | 6°99 5°67 | 7°77 6°30 | 57 |
| 6 | 776 681 | 1°55 1°26 | 2°33 1°89 | 3°10 2°52 | 3°88 3°15 | 4°66 3°78 | 5°43 4°41 | 6°21 5°05 | 6°98 5°68 | 7°76 6°31 | 54 |
| 9 | 775 681 | 1°55 1°26 | 2°33 1°89 | 3°02 2°53 | 3°88 3°16 | 4°65 3°79 | 5°43 4°42 | 6°20 5°05 | 6°98 5°68 | 7°75 6°31 | 51 |
| 12 | 775 682 | 1°55 1°26 | 2°32 1°90 | 3°10 2°53 | 3°87 3°16 | 4°65 3°79 | 5°42 4°42 | 6°20 5°06 | 6°97 5°69 | 7°75 6°32 | 48 |
| 15 | 774 683 | 1°55 1°27 | 2°32 1°90 | 3°10 2°53 | 3°87 3°16 | 4°65 3°80 | 5°42 4°43 | 6°20 5°06 | 6°97 5°69 | 7°74 6°33 | 45 |
| 18 | 774 683 | 1°55 1°27 | 2°32 1°90 | 3°10 2°53 | 3°87 3°17 | 4°64 3°80 | 5°42 4°43 | 6°20 5°07 | 6°96 5°70 | 7°74 6°33 | 42 |
| 21 | 773 683 | 1°55 1°27 | 2°32 1°90 | 3°03 2°54 | 3°87 3°17 | 4°64 3°80 | 5°41 4°44 | 6°19 5°07 | 6°96 5°71 | 7°73 6°34 | 39 |
| 24 | 773 685 | 1°55 1°27 | 2°32 1°90 | 3°03 2°54 | 3°89 3°17 | 4°64 3°81 | 5°41 4°44 | 6°18 5°08 | 6°95 5°71 | 7°73 6°35 | 36 |
| 27 | 772 685 | 1°54 1°27 | 2°32 1°91 | 3°00 2°54 | 3°86 3°18 | 4°63 3°81 | 5°41 4°45 | 6°18 5°08 | 6°95 5°72 | 7°72 6°35 | 33 |
| 30 | 772 686 | 1°54 1°27 | 2°31 1°91 | 3°00 2°54 | 3°86 3°18 | 4°63 3°82 | 5°40 4°45 | 6°17 5°05 | 6°94 5°72 | 7°72 6°36 | 30 |
| 33 | 771 687 | 1°54 1°27 | 2°31 1°91 | 3°08 2°55 | 3°86 3°18 | 4°63 3°82 | 5°40 4°46 | 6°17 5°09 | 6°94 5°73 | 7°71 6°37 | 27 |
| 36 | 771 687 | 1°54 1°27 | 2°31 1°91 | 3°08 2°55 | 3°85 3°19 | 4°62 3°82 | 5°39 4°46 | 6°16 5°10 | 6°93 5°74 | 7°71 6°37 | 24 |
| 39 | 770 688 | 1°54 1°28 | 2°31 1°91 | 3°08 2°55 | 3°85 3°19 | 4°62 3°82 | 5°39 4°47 | 6°16 5°10 | 6°93 5°74 | 7°70 6°38 | 21 |
| 42 | 769 689 | 1°54 1°28 | 2°31 1°92 | 3°08 2°56 | 3°85 3°19 | 4°62 3°83 | 5°39 4°47 | 6°16 5°11 | 6°92 5°75 | 7°69 6°39 | 18 |
| 45 | 759 689 | 1°54 1°28 | 2°31 1°92 | 3°05 2°56 | 3°84 3°20 | 4°61 3°84 | 5°38 4°48 | 6°15 5°12 | 6°92 5°75 | 7°69 6°39 | 15 |
| 48 | 768 640 | 1°51 1°28 | 2°30 1°92 | 3°07 2°56 | 3°84 3°20 | 4°61 3°84 | 5°38 4°48 | 6°15 5°12 | 6°91 5°76 | 7°68 6°40 | 12 |
| 51 | 768 641 | 1°54 1°28 | 2°30 1°92 | 3°07 2°56 | 3°84 3°20 | 4°61 3°84 | 5°37 4°49 | 6°14 5°13 | 6°91 5°77 | 7°68 6°41 | 9 |
| 54 | 767 641 | 1°53 1°28 | 2°30 1°92 | 3°07 2°57 | 3°84 3°21 | 4°60 3°85 | 5°38 4°49 | 6°14 5°13 | 6°90 5°77 | 7°67 6°41 | 6 |
| 57 | 767 642 | 1°53 1°28 | 2°30 1°93 | 3°07 2°57 | 3°83 3°21 | 4°60 3°85 | 5°37 4°49 | 6°13 5°14 | 6°90 5°78 | 7°67 6°42 | 3 |
| 40° | 766 643 | 1°53 1°29 | 2°30 1°93 | 3°06 2°57 | 3°83 3°21 | 4°60 3°86 | 5°36 4°50 | 6°13 5°14 | 6°89 5°79 | 7°66 6°43 | 50° |
| 3 | 765 643 | 1°53 1°29 | 2°30 1°93 | 3°06 2°57 | 3°83 3°22 | 4°59 3°86 | 5°36 4°50 | 6°12 5°15 | 6°89 5°79 | 7°65 6°43 | 57 |
| 6 | 765 644 | 1°53 1°29 | 2°29 1°93 | 3°06 2°58 | 3°82 3°22 | 4°59 3°86 | 5°35 4°51 | 6°12 5°15 | 6°88 5°80 | 7°65 6°44 | 54 |
| 9 | 764 645 | 1°53 1°29 | 2°29 1°93 | 3°06 2°58 | 3°82 3°22 | 4°59 3°87 | 5°35 4°51 | 6°11 5°16 | 6°88 5°80 | 7°64 6°45 | 51 |
| 12 | 764 645 | 1°53 1°29 | 2°29 1°94 | 3°06 2°58 | 3°82 3°23 | 4°58 3°87 | 5°35 4°52 | 6°11 5°16 | 6°87 5°81 | 7°64 6°46 | 48 |
| 15 | 763 646 | 1°53 1°29 | 2°29 1°94 | 3°05 2°58 | 3°82 3°23 | 4°58 3°88 | 5°34 4°52 | 6°11 5°17 | 6°87 5°82 | 7°63 6°46 | 45 |
| 18 | 763 647 | 1°53 1°29 | 2°29 1°94 | 3°05 2°59 | 3°81 3°23 | 4°58 3°88 | 5°34 4°53 | 6°10 5°17 | 6°86 5°82 | 7°63 6°47 | 42 |
| 21 | 762 647 | 1°52 1°29 | 2°29 1°94 | 3°05 2°59 | 3°81 3°21 | 4°57 3°88 | 5°34 4°53 | 6°10 5°18 | 6°86 5°83 | 7°62 6°47 | 39 |
| 24 | 762 648 | 1°52 1°30 | 2°28 1°94 | 3°05 2°59 | 3°81 3°21 | 4°57 3°89 | 5°33 4°54 | 6°09 5°18 | 6°85 5°83 | 7°62 6°49 | 36 |
| 27 | 761 649 | 1°52 1°30 | 2°28 1°95 | 3°04 2°00 | 3°80 3°24 | 4°57 3°89 | 5°33 4°54 | 6°09 5°19 | 6°85 5°84 | 7°61 6°49 | 33 |
| 30 | 760 649 | 1°52 1°30 | 2°28 1°95 | 3°04 2°00 | 3°80 3°25 | 4°56 3°86 | 5°32 4°55 | 6°08 5°20 | 6°84 5°85 | 7°60 6°50 | 30 |
| 33 | 760 650 | 1°52 1°30 | 2°28 1°95 | 3°04 2°00 | 3°80 3°25 | 4°56 3°89 | 5°32 4°55 | 6°08 5°20 | 6°84 5°85 | 7°60 6°50 | 27 |
| 36 | 759 651 | 1°52 1°30 | 2°28 1°95 | 3°04 2°00 | 3°80 3°25 | 4°56 3°90 | 5°31 4°56 | 6°07 5°20 | 6°83 5°86 | 7°59 6°51 | 24 |
| 39 | 759 651 | 1°52 1°30 | 2°28 1°95 | 3°03 2°01 | 3°79 3°26 | 4°55 3°91 | 5°31 4°56 | 6°07 5°21 | 6°83 5°86 | 7°59 6°51 | 21 |
| 42 | 758 652 | 1°52 1°30 | 2°28 1°96 | 3°02 2°01 | 3°79 3°26 | 4°55 3°91 | 5°31 4°57 | 6°07 5°22 | 6°82 5°87 | 7°58 6°52 | 18 |
| 45 | 758 653 | 1°52 1°31 | 2°27 1°96 | 3°03 2°01 | 3°79 3°26 | 4°55 3°92 | 5°30 4°57 | 6°06 5°22 | 6°82 5°87 | 7°58 6°53 | 15 |
| 48 | 757 653 | 1°51 1°31 | 2°27 1°96 | 3°03 2°01 | 3°78 3°27 | 4°54 3°92 | 5°30 4°57 | 6°06 5°23 | 6°81 5°88 | 7°57 6°53 | 12 |
| 51 | 756 654 | 1°51 1°31 | 2°27 1°96 | 3°03 2°02 | 3°78 3°27 | 4°54 3°92 | 5°29 4°58 | 6°05 5°23 | 6°81 5°89 | 7°56 6°54 | 9 |
| 54 | 756 655 | 1°51 1°31 | 2°27 1°96 | 3°02 2°02 | 3°78 3°27 | 4°54 3°93 | 5°29 4°58 | 6°05 5°24 | 6°80 5°89 | 7°56 6°55 | 6 |
| 57 | 755 655 | 1°51 1°31 | 2°27 1°97 | 3°02 2°02 | 3°75 3°28 | 4°53 3°93 | 5°29 4°59 | 6°04 5°24 | 6°80 5°90 | 7°55 6°55 | 3 |
| 41° | 755 656 | 1°51 1°31 | 2°26 1°97 | 3°02 2°02 | 3°77 3°28 | 4°53 3°94 | 5°28 4°59 | 6°04 5°25 | 6°79 5°90 | 7°55 6°56 | 49° |
| 3 | 754 657 | 1°51 1°31 | 2°26 1°97 | 3°02 2°03 | 3°77 3°28 | 4°52 3°94 | 5°28 4°60 | 6°03 5°25 | 6°79 5°91 | 7°54 6°57 | 57 |
| 6 | 754 657 | 1°51 1°31 | 2°26 1°97 | 3°01 2°03 | 3°77 3°29 | 4°52 3°94 | 5°27 4°60 | 6°03 5°26 | 6°78 5°92 | 7°54 6°57 | 54 |
| 9 | 753 658 | 1°51 1°32 | 2°26 1°97 | 3°01 2°03 | 3°76 3°29 | 4°52 3°95 | 5°27 4°61 | 6°02 5°26 | 6°78 5°92 | 7°53 6°58 | 51 |
| 12 | 752 659 | 1°50 1°32 | 2°26 1°98 | 3°01 2°03 | 3°76 3°29 | 4°51 3°95 | 5°27 4°61 | 6°02 5°27 | 6°77 5°93 | 7°52 6°59 | 48 |
| 15 | 752 659 | 1°50 1°32 | 2°26 1°98 | 3°01 2°04 | 3°76 3°30 | 4°51 3°96 | 5°26 4°62 | 6°01 5°27 | 6°77 5°93 | 7°52 6°59 | 45 |
| 18 | 751 660 | 1°50 1°32 | 2°25 1°98 | 3°01 2°04 | 3°76 3°30 | 4°51 3°96 | 5°26 4°62 | 6°01 5°28 | 6°76 5°94 | 7°51 6°60 | 42 |
| 21 | 751 661 | 1°50 1°32 | 2°25 1°98 | 3°00 2°04 | 3°75 3°30 | 4°50 3°96 | 5°25 4°62 | 6°01 5°29 | 6°76 5°95 | 7°51 6°61 | 39 |
| 24 | 750 661 | 1°50 1°32 | 2°25 1°98 | 3°00 2°05 | 3°75 3°31 | 4°50 3°97 | 5°25 4°63 | 6°00 5°29 | 6°75 5°95 | 7°50 6°61 | 36 |
| 27 | 750 662 | 1°50 1°32 | 2°25 1°98 | 3°00 2°05 | 3°75 3°31 | 4°50 3°97 | 5°25 4°63 | 6°00 5°30 | 6°75 5°96 | 7°50 6°62 | 33 |
| 30 | 749 663 | 1°50 1°33 | 2°25 1°99 | 3°00 2°05 | 3°74 3°31 | 4°49 3°98 | 5°24 4°64 | 5°99 5°30 | 6°74 5°96 | 7°49 6°63 | 30 |
| 33 | 748 663 | 1°50 1°33 | 2°25 1°99 | 2°99 2°05 | 3°71 3°32 | 4°49 3°98 | 5°24 4°64 | 5°99 5°31 | 6°74 5°97 | 7°48 6°63 | 27 |
| 36 | 748 664 | 1°50 1°33 | 2°24 1°99 | 2°99 2°06 | 3°74 3°32 | 4°49 3°98 | 5°24 4°65 | 5°98 5°31 | 6°73 5°98 | 7°48 6°64 | 24 |
| 39 | 747 665 | 1°49 1°33 | 2°24 1°99 | 2°99 2°06 | 3°74 3°32 | 4°48 3°99 | 5°23 4°65 | 5°98 5°32 | 6°73 5°98 | 7°47 6°65 | 21 |
| 42 | 747 665 | 1°49 1°33 | 2°24 2°00 | 2°99 2°06 | 3°73 3°33 | 4°46 3°99 | 5°23 4°66 | 5°97 5°32 | 6°72 5°99 | 7°47 6°65 | 18 |
| 45 | 746 666 | 1°49 1°33 | 2°24 2°00 | 2°98 2°06 | 3°73 3°33 | 4°46 3°99 | 5°22 4°66 | 5°97 5°33 | 6°71 5°99 | 7°46 6°66 | 15 |
| 48 | 745 667 | 1°49 1°33 | 2°24 2°00 | 2°98 2°07 | 3°73 3°33 | 4°47 3°99 | 5°22 4°67 | 5°97 5°33 | 6°71 6°00 | 7°45 6°67 | 12 |
| 51 | 745 667 | 1°49 1°33 | 2°23 2°00 | 2°98 2°07 | 3°72 3°34 | 4°47 3°99 | 5°21 4°67 | 5°96 5°34 | 6°70 6°00 | 7°45 6°67 | 9 |
| 54 | 744 668 | 1°49 1°34 | 2°23 2°00 | 2°98 2°07 | 3°72 3°34 | 4°47 4°00 | 5°21 4°67 | 5°95 5°34 | 6°70 6°01 | 7°44 6°68 | 6 |
| 57 | 744 668 | 1°49 1°34 | 2°23 2°01 | 2°97 2°07 | 3°72 3°34 | 4°46 4°01 | 5°21 4°68 | 5°95 5°35 | 6°69 6°02 | 7°44 6°68 | 3 |

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| 48° | 743°669 | 1°49'1"34" | 223°20' | 2°97'2"68 | 3°72'3"35 | 4°46'1"01 | 5°52'1"68 | 5°95'5"35 | 6°69'6"02 | 7°43'6"69 | 42° |
| 3 | 743°670 | 1°49'1"34" | 223°20' | 2°97'2"68 | 3°71'3"35 | 4°46'1"02 | 5°52'1"69 | 5°94'5"36 | 6°68'6"03 | 7°43'6"70 | 57 |
| 6 | 742°670 | 1°48'1"34" | 223°20' | 2°97'2"68 | 3°71'3"35 | 4°45'1"02 | 5°51'1"69 | 5°94'5"36 | 6°68'6"03 | 7°42'6"54 | |
| 9 | 741°671 | 1°48'1"34" | 222°20' | 2°97'2"68 | 3°71'3"36 | 4°45'1"03 | 5°51'1"70 | 5°93'5"37 | 6°67'6"04 | 7°41'6"71 | 51 |
| 12 | 741°672 | 1°48'1"34" | 222°20' | 2°96'2"69 | 3°70'3"36 | 4°44'1"03 | 5°51'1"70 | 5°93'5"37 | 6°67'6"05 | 7°41'6"72 | 48 |
| 15 | 740°672 | 1°48'1"34" | 222°20' | 2°96'2"69 | 3°70'3"37 | 4°44'1"04 | 5°51'1"71 | 5°92'5"38 | 6°66'6"05 | 7°40'6"72 | 45 |
| 18 | 740°673 | 1°48'1"35" | 222°20' | 2°96'2"69 | 3°70'3"37 | 4°44'1"04 | 5°51'1"71 | 5°92'5"38 | 6°66'6"06 | 7°40'6"73 | 42 |
| 21 | 739°674 | 1°48'1"35" | 222°20' | 2°96'2"69 | 3°70'3"37 | 4°43'1"04 | 5°51'1"72 | 5°91'5"39 | 6°65'6"06 | 7°39'6"74 | 39 |
| 24 | 738°674 | 1°48'1"35" | 222°20' | 2°95'2"70 | 3°69'3"37 | 4°43'1"05 | 5°51'1"72 | 5°91'5"39 | 6°65'6"07 | 7°38'6"74 | 36 |
| 27 | 738°675 | 1°48'1"35" | 221°20' | 2°95'2"70 | 3°69'3"38 | 4°42'1"05 | 5°51'5"40 | 5°89'5"40 | 6°64'6"07 | 7°38'6"75 | 33 |
| 30 | 737°676 | 1°47'1"35 | 221°20' | 2°95'2"70 | 3°68'3"38 | 4°42'1"06 | 5°51'5"41 | 5°89'5"41 | 6°63'6"09 | 7°37'6"76 | 30 |
| 33 | 737°676 | 1°47'1"35 | 221°20' | 2°94'2"70 | 3°68'3"38 | 4°42'1"06 | 5°51'5"41 | 5°89'5"42 | 6°62'6"09 | 7°36'6"77 | 27 |
| 36 | 736°677 | 1°47'1"35" | 221°20' | 2°94'2"70 | 3°68'3"39 | 4°41'1"07 | 5°51'5"42 | 5°88'5"42 | 6°62'6"10 | 7°36'6"78 | 24 |
| 39 | 736°678 | 1°47'1"36" | 221°20' | 2°94'2"71 | 3°68'3"39 | 4°41'1"07 | 5°51'5"42 | 5°88'5"43 | 6°61'6"10 | 7°35'6"78 | 21 |
| 42 | 735°678 | 1°47'1"36" | 220°20' | 2°94'2"71 | 3°67'3"39 | 4°41'1"07 | 5°51'4"75 | 5°88'5"43 | 6°61'6"10 | 7°35'6"78 | 18 |
| 45 | 734°679 | 1°47'1"36" | 220°20' | 2°94'2"71 | 3°67'3"39 | 4°41'1"07 | 5°51'4"75 | 5°87'5"43 | 6°61'6"11 | 7°34'6"79 | 15 |
| 48 | 734°679 | 1°47'1"36" | 220°20' | 2°93'2"72 | 3°67'3"40 | 4°40'1"08 | 5°51'4"76 | 5°87'5"44 | 6°60'6"11 | 7°34'6"79 | 12 |
| 51 | 733°680 | 1°47'1"36" | 220°20' | 2°93'2"72 | 3°67'3"40 | 4°40'1"08 | 5°51'4"76 | 5°87'5"44 | 6°60'6"12 | 7°33'6"80 | 9 |
| 54 | 733°681 | 1°47'1"36" | 220°20' | 2°93'2"72 | 3°66'3"40 | 4°40'1"08 | 5°51'4"77 | 5°86'5"45 | 6°59'6"13 | 7°33'6"81 | 6 |
| 57 | 732°681 | 1°46'1"36" | 220°20' | 2°93'2"73 | 3°66'3"41 | 4°39'4"09 | 5°52'4"77 | 5°86'5"45 | 7°32'6"81 | 3 | |
| 47° | 731°682 | 1°46'1"36" | 219°20' | 2°93'2"73 | 3°66'3"41 | 4°33'4"09 | 5°12'4"77 | 5°85'5"46 | 6°58'6"14 | 7°31'6"82 | 43° |
| 3 | 731°683 | 1°46'1"37" | 219°20' | 2°93'2"73 | 3°65'3"41 | 4°38'1"10 | 5°12'4"78 | 5°85'5"46 | 6°58'6"14 | 7°31'6"83 | 57 |
| 6 | 730°683 | 1°46'1"37" | 219°20' | 2°92'2"73 | 3°64'3"42 | 4°38'1"10 | 5°11'4"78 | 5°84'5"47 | 6°57'6"15 | 7°30'6"83 | 54 |
| 9 | 730°684 | 1°46'1"37" | 219°20' | 2°92'2"74 | 3°64'3"43 | 4°38'1"10 | 5°11'4"79 | 5°84'5"47 | 6°57'6"16 | 7°30'6"84 | 51 |
| 12 | 729°685 | 1°46'1"37" | 219°20' | 2°92'2"74 | 3°64'3"43 | 4°37'4"71 | 5°10'4"79 | 5°83'5"48 | 6°56'6"16 | 7°29'6"85 | 49 |
| 15 | 728°685 | 1°46'1"37" | 219°20' | 2°91'2"74 | 3°64'3"43 | 4°37'4"71 | 5°10'4"80 | 5°83'5"48 | 6°56'6"17 | 7°28'6"85 | 45 |
| 18 | 728°686 | 1°46'1"37" | 218°20' | 2°91'2"74 | 3°64'3"43 | 4°37'4"71 | 5°10'4"80 | 5°82'5"49 | 6°55'6"17 | 7°28'6"86 | 42 |
| 21 | 727°686 | 1°46'1"37" | 218°20' | 2°91'2"75 | 3°64'3"43 | 4°36'1"12 | 5°09'4"81 | 5°82'5"49 | 6°54'6"18 | 7°27'6"86 | 39 |
| 24 | 727°687 | 1°46'1"37" | 218°20' | 2°90'2"75 | 3°63'3"44 | 4°36'1"12 | 5°09'4"81 | 5°81'5"50 | 6°54'6"18 | 7°27'6"87 | 36 |
| 27 | 726°688 | 1°46'1"38" | 218°20' | 2°90'2"75 | 3°63'3"44 | 4°36'1"13 | 5°08'4"82 | 5°80'5"51 | 6°53'6"20 | 7°25'6"88 | 33 |
| 30 | 725°688 | 1°45'1"38" | 218°20' | 2°90'2"75 | 3°63'3"44 | 4°35'4"13 | 5°08'4"82 | 5°80'5"51 | 6°53'6"20 | 7°25'6"89 | 30 |
| 33 | 725°689 | 1°45'1"38" | 217°20' | 2°90'2"76 | 3°62'3"44 | 4°35'4"13 | 5°07'4"82 | 5°80'5"51 | 6°52'6"20 | 7'25'6"89 | 27 |
| 36 | 724°690 | 1°45'1"38" | 217°20' | 2°90'2"76 | 3°62'3"45 | 4°35'4"14 | 5°07'4"83 | 5°79'5"52 | 6°52'6"21 | 7'24'6"90 | 24 |
| 39 | 724°690 | 1°45'1"38" | 217°20' | 2°89'2"76 | 3°62'3"45 | 4°34'1"14 | 5°07'4"83 | 5°79'5"52 | 6'51'6"21 | 7'24'6"90 | 21 |
| 42 | 723°691 | 1°45'1"38" | 217°20' | 2°89'2"76 | 3°61'3"45 | 4°34'1"15 | 5'06'4"84 | 5'76'5"53 | 6'51'6"22 | 7'23'6"91 | 19 |
| 45 | 722°692 | 1°44'1"38" | 217°20' | 2°89'2"77 | 3°61'3"46 | 4'33'1"15 | 5'06'4"84 | 5'78'5"53 | 6'50'6"22 | 7'22'6"92 | 15 |
| 48 | 722°692 | 1°44'1"38" | 217°20' | 2°89'2"77 | 3'61'3"46 | 4'33'1"15 | 5'05'4"85 | 5'77'5"53 | 6'50'6"23 | 7'22'6"92 | 12 |
| 51 | 721°693 | 1°44'1"39" | 216°20' | 2°88'2"77 | 3'61'3"46 | 4'33'1"16 | 5'05'4"85 | 5'77'5"54 | 6'49'6"23 | 7'21'6"93 | 9 |
| 54 | 721°693 | 1°44'1"39" | 216°20' | 2°88'2"77 | 3'60'3"47 | 4'32'4"16 | 5'04'4"85 | 5'76'5"55 | 6'48'6"24 | 7'21'6"93 | 6 |
| 57 | 720°694 | 1°44'1"39" | 216°20' | 2°88'2"78 | 3'60'3"47 | 4'32'4"16 | 5'04'4"86 | 5'76'5"55 | 6'46'6"25 | 7'20'6"94 | 3 |
| 46° | 719°695 | 1°44'1"39" | 216°20' | 2°88'2"78 | 3'60'3"47 | 4'32'4"17 | 5'04'4"86 | 5'75'5"56 | 6'47'6"25 | 7'19'6"95 | 44° |
| 3 | 719°695 | 1°44'1"39" | 216°20' | 2°87'2"78 | 3'59'3"48 | 4'31'1"17 | 5'03'4"87 | 5'75'5"56 | 6'47'6"26 | 7'19'6"95 | 57 |
| 6 | 718°696 | 1°44'1"39" | 215°20' | 2°87'2"78 | 3'59'3"48 | 4'31'1"18 | 5'03'4"87 | 5'75'5"57 | 6'46'6"26 | 7'18'6"96 | 54 |
| 9 | 718°697 | 1°44'1"39" | 215°20' | 2°87'2"79 | 3'59'3"48 | 4'31'1"18 | 5'02'4"88 | 5'74'5"57 | 6'46'6"27 | 7'18'6"97 | 51 |
| 12 | 717°697 | 1°43'1"39" | 215°20' | 2°87'2"79 | 3'58'3"49 | 4'30'1"18 | 5'02'4"88 | 5'74'5"58 | 6'45'6"27 | 7'17'6"97 | 48 |
| 15 | 716°696 | 1°43'1"40" | 215°20' | 2°87'2"79 | 3'58'3"49 | 4'30'1"19 | 5'01'4"88 | 5'73'5"58 | 6'45'6"28 | 7'16'6"98 | 45 |
| 18 | 716°698 | 1°43'1"40" | 215°20' | 2°86'2"79 | 3'58'3"49 | 4'29'4"19 | 5'01'4"89 | 5'73'5"59 | 6'44'6"29 | 7'16'6"98 | 42 |
| 21 | 715°699 | 1°43'1"40" | 215°20' | 2°86'2"80 | 3'58'3"50 | 4'29'4"19 | 5'01'4"89 | 5'72'5"59 | 6'44'6"29 | 7'15'6"99 | 39 |
| 24 | 714°700 | 1°43'1"40" | 214°20' | 2°86'2"80 | 3'57'3"50 | 4'29'4"20 | 5'00'4"90 | 5'72'5"60 | 6'43'6"30 | 7'14'7'00 | 36 |
| 27 | 714°700 | 1°43'1"40" | 214°20' | 2°86'2"80 | 3'57'3"50 | 4'28'4"20 | 5'00'4"90 | 5'71'5"60 | 6'42'6"30 | 7'14'7'00 | 33 |
| 30 | 713°701 | 1°43'1"40" | 214°20' | 2°85'2"80 | 3'57'3"50 | 4'28'4"21 | 4'99'4"91 | 5'71'5"61 | 6'42'6"31 | 7'13'7'01 | 30 |
| 33 | 713°702 | 1°43'1"40" | 214°20' | 2°85'2"81 | 3'56'3"51 | 4'28'4"21 | 4'99'4"91 | 5'70'5"61 | 6'41'6"31 | 7'13'7'02 | 27 |
| 36 | 712°702 | 1°42'1"40" | 214°21' | 2°85'2"81 | 3'56'3"51 | 4'27'4"21 | 4'98'4"92 | 5'70'5"62 | 6'41'6"32 | 7'12'7'02 | 24 |
| 39 | 711°703 | 1°42'1"41" | 213°21' | 2°85'2"81 | 3'56'3"51 | 4'27'4"22 | 4'98'4"92 | 5'69'5"62 | 6'40'6"32 | 7'11'7'03 | 21 |
| 42 | 711°703 | 1°42'1"41" | 213°21' | 2°84'2"81 | 3'55'3"52 | 4'26'4"22 | 4'98'4"92 | 5'69'5"63 | 6'40'6"33 | 7'11'7'03 | 18 |
| 45 | 710°704 | 1°42'1"41" | 213°21' | 2°84'2"82 | 3'55'3"52 | 4'26'4"22 | 4'97'4"93 | 5'68'5"63 | 6'39'6"34 | 7'10'7'04 | 15 |
| 48 | 710°705 | 1°42'1"41" | 213°21' | 2°84'2"82 | 3'55'3"52 | 4'26'4"23 | 4'97'4"93 | 5'68'5"64 | 6'39'6"34 | 7'10'7'05 | 12 |
| 51 | 709°705 | 1°42'1"41" | 213°21' | 2°83'2"82 | 3'54'3"53 | 4'25'4"24 | 4'96'4"94 | 5'67'5"64 | 6'38'6"35 | 7'09'7'05 | 9 |
| 54 | 708°706 | 1°42'1"41" | 213°21' | 2°83'2"83 | 3'54'3"53 | 4'25'4"24 | 4'95'4"95 | 5'66'5"65 | 6'37'6"36 | 7'08'7'06 | 6 |
| 57 | 707°707 | 1°41'1"41" | 212°21' | 2°83'2"83 | 3'54'3"54 | 4'24'4"24 | 4'95'4"95 | 5'66'5"66 | 6'36'6"36 | 7'07'7'07 | 3 |
| 45° | 707°707 | 1°41'1"41" | 212°21' | 2°83'2"83 | 3'54'3"54 | 4'24'4"24 | — | — | — | — | 45° |

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